Planning Board Meeting February 12, 2019 – 7:00 PM Town Office Conference Room



Meeting Materials

Planning Board Tuesday, February 12, 2019 7:00 PM – Town Office Conference Room

CALL TO ORDER

PUBLIC HEARING

CLUC Amendments 2019

MINUTES

January 22, 2019

COMMUNICATIONS

OLD BUSINESS

NEW BUSINESS

- Minor Subdivision Application Amendment Holly Russell Off Cleve Tripp Road Map 5 Lot 24 Sublot 3
- Formal Site Plan Review Poland Corner Road Water Main Extension Stream Crossing and Water Tank Storage Poland Corner Road Map 11 Lots 6B and 5
- Formal Site Plan Review MB Investment Properties, LLC 100 Bark Mulch Drive Map 4 Lot 15 Sublot 2 and Map 4 Lot 17
- Findings of Fact and Conclusions of Law for:
 - Formal Shoreland Zoning Application Thomas and Abbie Mannett 41 Black Duck Lane Map 36 Lot 7 Sublot 8
 - Formal Shoreland Zoning Application Leonard and Patsy Adams 283 Jordan Shore Drive Map 29 Lot 3
 - Sketch Plan Review Dawn Miller 1199 Maine Street Map 12 Lot 44
 - Formal Shoreland Zoning Application Arthur and Rebecca Weissman 37 Mountain View Drive Map 49 Lot 35
 - Minor Subdivision Application Megquire Hill Holdings, LLC 260 Megquire Hill Road Map 14 Lots 25 and 25B
 - Minor Subdivision Application Greg Washburn Off Maine Street Map 15 Lot 18B

Vote of CLUC Amendments 2019

Planning Board Tuesday, February 12, 2019 7:00 PM – Town Office Conference Room

ANY OTHER BUSINESS

ADJOURNMENT

POLAND PLANNING BOARD MINUTES OF MEETING

January 22, 2019 Approved on _____, 2019

<u>CALL TO ORDER</u> – Vice Chairperson Porter called the meeting to order at 7:00pm with Members Mark Weinberg, George Greenwood, Stephanie Floyd, CEO Scott Neal, and Sarah Merrill present.

<u>MINUTES</u> – January 8, 2019 – Member Greenwood moved to approve the minutes with a change. Member Floyd seconded the motion. Discussion: None Vote: 4-yes 0-no

COMMUNICATIONS – None

<u>OLD BUSINESS</u> – Formal Site Plan Review – 1484 Maine Street – Mark Lopez – Map 15 Lot 18B (Additional Information)

Jeff Amos is representing Mark Lopez and presented the project to the Board. The current plan calls for rotating the buildings ninety degrees (90°) and putting in a six foot (6') tall stockade fence.

The Board feels that the issues have been addressed and Mr. Lopez has tried to do everything the Board has requested. The Board will conditionally approve the project pending third party review and the review from the Fire Chief.

Member Greenwood moved to approve the checklist as complete. Member Floyd seconded the motion. Discussion: None Vote: 4-yes 0-no

Member Greenwood moved to conditionally accept the application pending third party review and review from the Fire Chief and with the following conditions: the public hearing and site walk are waived. Member Weinberg seconded the motion. Discussion: None Vote: 4-yes 0-no

NEW BUSINESS – Formal Shoreland Zoning Application – Waterhouse Brook Dam on Poland Corner Road – Town of Poland Conservation Committee – Map 10 Lot 28 Sublot 1

Stewart Davis of Davis Land Surveying presented the Conservation Commission project to the Board. The Poland Conservation Commission would like to put a pedestrian bridge over the Waterhouse Brook Dam to connect two sides of a trail and to give people a safe place to cross away from the bridge.

Town Manager Matt Garside spoke on the design changes that need to happen and other changes the Conservation Commission wants to see. Don Stover also spoke about Waterhouse Brook Park.

There is a correction from the plan submitted. The steps in the plan are labeled as 4' x 8' however, they will be 4' x 4'. Once there is a final design for the bridge the DEP will

POLAND PLANNING BOARD MINUTES OF MEETING

January 22, 2019 Approved on _____, 2019

give their final say and the Conservation Commission will come back to the planning board.

The Conservation Commission would like to put some parking spaces in but realize this could be an issue. Mr. Davis will meet with CEO Neal to discuss this issue and to look at the zoning for the fire/rescue station.

Member Greenwood moved to table the application until the Conservation Commission has more information. Member Weinberg seconded the motion.

Discussion: None Vote: 4-yes 0-no

<u>Formal Shoreland Zoning Application – 283 Jordan Shore Drive – Leonard and Patsy Adams – Map 29 Lot 3</u>

Stewart Davis of Davis Land Surveying presented the Adams project to the Board. The Adams would like to build a small addition and rebuild the steps on the camp. The Adams have also replaced an old shed with a larger shed which they did not get a building permit for. Mr. Davis said that the Adams, who live out of state, will come to get the appropriate permits for the shed when they are back in Maine.

The Adams will need to get a Flood Hazard Development Application permit from the state as part of their building application with the code enforcement office.

Member Greenwood moved to approve the application checklist. Member Floyd seconded the motion. Discussion: None Vote: 4-yes 0-no

Member Greenwood moved to approve as complete the Formal Shoreland Zoning Application with the following conditions: the public hearing is waived, the site walk is waived, and the mylars will show the percentage of expansion on them and be recorded at the registry of deeds.

ANY OTHER BUSINESS – Marijuana Committee Update

Mark Weinberg gave an update on the progress being made on the marijuana CLUC changes. The Committee is finished meeting and the final CLUC changes are being written up and will be part of the public hearing on all of the CLUC changes.

<u>ADJOURN</u> – Member Floyd moved to adjourn the meeting at 8:35 pm. Member Weinberg seconded the motion. Discussion: None Vote: 4-yes 0-no

Recorded by: Sarah Merrill

POLAND PLANNING BOARD MINUTES OF MEETING January 22, 2019

Approved on ______, 2019

Planning I	Board
, Chairperson	James Porter, Vice - Chairperson
George Greenwood, Secretary	Mark Weinberg, Member
Stephane Floyd, Member	, Alternate

<u>Article?</u> To see if the Town will vote to adopt the 2019 Amendment to the <u>Poland</u>
<u>Comprehensive Land Use Code</u> – Chapter 14 Definitions "Repair"? (A copy of the proposed ordinance amendment is available for inspection in the Clerk's office, as well as on the Town's web site at polandtownoffice.org, and will also be available at Town Meeting.)

Chapter 14 Definitions:

Repair: To take necessary action to fix normal damage or storm damage. To restore by fixing normal damage or storm damage i.e. 49% or less is being changed.

<u>Article?</u> To see if the Town will vote to adopt the 2019 Amendment to the <u>Poland</u>
<u>Comprehensive Land Use Code</u> – Chapter 14 Definitions "Replace"? (A copy of the proposed ordinance amendment is available for inspection in the Clerk's office, as well as on the Town's web site at polandtownoffice.org, and will also be available at Town Meeting.)

Chapter 14 Definitions:

Replace: to put something new in place of i.e. 50% or more is being changed.

<u>Article?</u> To see if the Town will vote to adopt the 2019 Amendment to the <u>Poland</u>
<u>Comprehensive Land Use Code</u> – section 508.18. L.3.? (A copy of the proposed ordinance amendment is available for inspection in the Clerk's office, as well as on the Town's web site at polandtownoffice.org, and will also be available at Town Meeting.)

L.3. Signs in Street Right-of-Ways - No sign except traffic, and similar public safety signs, erected in accordance with this Section, Official Business Directional Signs erected in accordance with Section 508.18.K, and publicly erected information kiosks or sign boards, and temporary noncommercial signs of any type meeting the requirements of 23 M.R.S.A. §1913 – A(1)(L) shall be located in the public right-of-way of any street or highway.

<u>Article?</u> To see if the Town will vote to adopt the 2019 Amendment to the <u>Poland</u>
<u>Comprehensive Land Use Code</u> – Chapter 14 Definitions "Agriculture"? (A copy of the proposed ordinance amendment is available for inspection in the Clerk's office, as well as on the Town's web site at polandtownoffice.org, and will also be available at Town Meeting.)

Chapter 14 Definitions:

Agriculture: the production, keeping, or maintenance, for sale or lease, of plants and/or animals, including but not limited to: forages and sod crops; grains and seed crops; dairy animals and dairy products; poultry and poultry products; livestock; fruits and vegetables; and ornamental and green house products. Agriculture does not include forest management, timber harvesting activities, or confined feeding operations, or marijuana stores, marijuana production manufacturing, marijuana cultivation, marijuana processing, or marijuana testing, etc.

Article? To see if the Town will vote to adopt the 2019 Amendment to the Poland Comprehensive Land Use Code. Section 1100 "Amendments to Building Standards"? (A copy of the proposed ordinance amendment is available for inspection in the Clerk's office, as well as on the Town's web site at polandtownoffice.org, and will also be available at Town Meeting.)

CHAPTER 11 - BUILDING STANDARDS 109

1100 SHORT TITLE. This Chapter shall be known and may be cited as the "Building Code Standards for the Town of Poland, Maine" in conjunction with the Building Code Standards and Adoption of the 2015 International Building Code (IBC), 2015 International Residential Code (IRC), 2015 International Existing Building Code (IEBC), 2015 2009 International Energy Conservation Code (IECC) and 2009 International Mechanical Code (IMC), all of which are hereby incorporated by Reference. The following standards have also been adopted E-1465-2006 2008, (Standard Practice for Radon Control Options for the Design and Construction of New Low-Rise Residential Buildings), ASHRAE 62.1 – 2007 2013 (Ventilation for Acceptable Indoor Air Quality) ASHRAE 62.2 – 2007 2013 (Ventilation and Acceptable Indoor Air Quality in Low-Rise Residential Buildings) ASHRAE 90.1 – 2007 2013 (Energy Standard for Buildings except Low-Rise Residential Buildings) editions without addenda. 1

<u>Article?</u> To see if the Town will vote to adopt the 2019 Amendment to the <u>Poland</u> <u>Comprehensive Land Use Code</u>. Section 304.3 "Appeal Procedure"? (A copy of the proposed ordinance amendment is available for inspection in the Clerk's office, as well as on the Town's web site at polandtownoffice.org, and will also be available at Town Meeting.)

304.3 Appeal Procedure

- A. In all cases a person aggrieved by a decision of the Code Enforcement Officer or an appealable decision of the Planning Board shall commence an appeal within forty-five (45) business days of the decision being appealed. The appeal shall be filed with the Board of Appeals on forms approved for that purpose, and the aggrieved person shall specifically set forth on the form the grounds for the appeal.
- B. Following the filing of an appeal, and before taking action on the appeal, the Board of Appeals shall hold a Public Hearing on the appeal within thirty (30) <u>business</u> days of receipt of the appeal. The Board of Appeals shall notify the Code Enforcement Officer and Planning Board at least ten (10) <u>business</u> days in advance of the time and place of the hearing, and shall publish notice of the hearing at least five (5) <u>business</u> days in advance of the hearing in a newspaper of general circulation in the area.
- C. The Board of Appeals shall notify by Certified Mail the appellant and the owners of property within five hundred (500) feet of the property of which the appeal is taken, at least ten (10) business days in advance of the hearing. For the purpose of this Section, abutting property owners shall include properties directly across a street from the property for which the appeal is made.
 - 1. The owners of property shall be considered to be those against whom taxes are assessed. Failure of any property owner to receive notice of the Public Hearing shall not necessitate another hearing or invalidate any action by the Board of Appeals. ¹
 - 2. It is the property owners' responsibility to keep the town Tax Assessor's Office informed of their current mailing address for the purpose of administering any portion of this Code. Failure on the part of the landowner to inform the town of such shall not delay or vacate any board or town officer action or decision.
- D. At any hearing, a party may be represented by an agent or attorney. Hearings shall not be continued to other times except for good cause.
- E. The Code Enforcement Officer or designated assistant shall attend all hearings and may present to the Board of Appeals all plans, photographs, or other material deemed appropriate for an understanding of the appeal.
- F. Within twenty (20) <u>business</u> days of the Public Hearing, the Board of Appeals shall reach a decision on an appeal and shall inform, in writing, the appellant, the Code Enforcement Officer, the Planning Board and the Municipal Officers of it decision and its reasons therefore.

David 2 of 45

<u>Article?</u> To see if the Town will vote to adopt the 2019 Amendment to the <u>Poland</u> <u>Comprehensive Land Use Code</u>. Section 304.4 "Reconsideration"? (A copy of the proposed ordinance amendment is available for inspection in the Clerk's office, as well as on the Town's web site at polandtownoffice.org, and will also be available at Town Meeting.)

304.4 Reconsideration - The Board of Appeals may reconsider any decision reached within forty-five (45) <u>business</u> days of its prior decision. A request to the Board to reconsider a decision made by anyone other than a member of the Board must be filed within 10 <u>business</u> days of the decision that is to be reconsidered. A member of the Board may make a request for reconsideration at the next meeting of the Board, subject to the time limitations set forth in this section. In the event that the Board votes to reconsider a decision, all action on the reconsideration must be completed within forty-five (45) <u>business</u> days of the original decision. The Board may conduct additional hearings and receive additional evidence and testimony. Notwithstanding the provisions of Section 304.5.A, an appeal of a reconsidered decision must be made within fifteen (15) <u>business</u> days after the decision on reconsideration.

<u>Article?</u> To see if the Town will vote to adopt the 2019 Amendment to the <u>Poland</u>
<u>Comprehensive Land Use Code</u> – Chapter 14 Definitions "Business Day"? (A copy of the proposed ordinance amendment is available for inspection in the Clerk's office, as well as on the Town's web site at polandtownoffice.org, and will also be available at Town Meeting.)

Chapter 14 Definitions:

Business Day: Business days are Monday through Friday, excluding holidays.

Article?. Shall an ordinance entitled "2019 Amendments to the Poland Comprehensive Land Use Code to Authorize and Regulate Medical Marijuana Registered Caregiver Retail Stores" be enacted? (A copy of the proposed ordinance is available for review and inspection at the Town Clerk's Office; it will also be available at Town Meeting.)

1507 MEDICAL MARIJUANA REGISTERED CAREGIVER RETAIL STORES

1507.1 Allowed Locations

Medical marijuana registered caregiver retail stores shall be allowed in the following locations, subject to the requirements of this Section:

- A. Village 4 Zone.
- B. General Purpose 1 Zone.
- C. Each zone, Village 4 and General Purpose 1, may have a combined maximum of five medical marijuana registered caregiver retail stores and/or adult use retail stores in each district. A site may contain one or more stores provided they are all within the same building.
- D. This is a permitted use requiring Planning Board approval and licensing from the Town to operate

1507.2 Signs

A. Signage for medical marijuana registered caregiver retail stores will comply with Chapter 5, Section 508.18 Signs, of this Code, as well as with any applicable requirements of State law or regulation.

1507.3 Security

Medical marijuana registered caregiver retail stores shall comply with the following requirements:

- A. Shall have lockable doors and windows and shall be served by an alarm system.
- B. Shall have video surveillance capable of covering the exterior and interior of the facility. The video surveillance system shall be operated with continuous recording twenty-four hours per day, seven days per week and video shall be retained for a minimum duration of thirty (30) days. Such records shall be made available to law enforcement agencies when investigating a criminal complaint.

1507.4 Ventilation

Each medical marijuana registered caregiver retail store must be in an enclosed structure and shall have an odor mitigation system installed and operational that will provide odor control sufficient to ensure that no odors are perceptible off the premises or to neighboring tenants in the building.

Article?. Shall an ordinance entitled "2019 Amendments to the Poland Comprehensive Land Use Code to Authorize and Regulate Medical Marijuana Testing Facilities" be enacted? (A copy of the proposed ordinance is available for review and inspection at the Town Clerk's Office; it will also be available at Town Meeting.)

1506 MEDICAL MARIJUANA TESTING FACILITIES

1506.1 Allowed Locations

Medical marijuana testing facilities shall be allowed in the following locations, subject to the requirements of this Section:

- A. All Zones
- B. Must be a registered caregiver and follow the requirements of home occupation as defined in the CLUC.
- C. Requires licensing by the Town to operate

1506.2 Security

Medical marijuana testing facilities must meet the following security requirements:

- A. Shall have lockable doors and windows and shall be served by an alarm system.
- B. Shall have video surveillance capable of covering the exterior and interior of the facility. The video surveillance system shall be operated with continuous recording twenty-four hours per day, seven days per week and video shall be retained for a minimum duration of thirty (30) days. Such records shall be made available to law enforcement agencies when investigating a criminal complaint.

1506.3 Ventilation

Medical marijuana testing facilities must be in an enclosed structure and shall have an odor mitigation system installed and operational that will provide odor control sufficient to ensure that no odors are perceptible off the premises or to neighboring tenants.

Article?. Shall an ordinance entitled "2019 Amendments to the Poland Comprehensive Land Use Code to Authorize and Regulate Medical Marijuana Manufacturing Facilities" be enacted? (A copy of the proposed ordinance is available for review and inspection at the Town Clerk's Office; it will also be available at Town Meeting.)

1505 MEDICAL MARIJUANA MANUFACTURING FACILITIES

1505.1 Allowed Locations

Medical marijuana manufacturing facilities shall be allowed in the following locations, subject to the requirements of this Section:

- A. All Zones
- B. Must be a registered caregiver and follow the requirements of home occupation as defined in the CLUC.
- C. Requires licensing by the Town to operate. (Same comment as 1506.1)

1505.2 Security

Medical marijuana manufacturing facilities must meet the following security requirements:

- A. Shall have lockable doors and windows and shall be served by an alarm system.
- B. Shall have video surveillance capable of covering the exterior and interior of the facility. The video surveillance system shall be operated with continuous recording twenty-four hours per day, seven days per week and video shall be retained for a minimum duration of thirty (30) days. Such records shall be made available to law enforcement agencies when investigating a criminal complaint.

1505.3 Ventilation

Medical marijuana manufacturing facilities must be in an enclosed structure and shall have an odor mitigation system installed and operational that will provide odor control sufficient to ensure that no odors are perceptible off the premises or to neighboring tenants.

<u>Article ?.</u> Shall an ordinance entitled "2019 Amendments to the Poland Comprehensive Land Use Code to Authorize and Regulate Adult Use Marijuana Cultivation Facilities" be enacted? (A copy of the proposed ordinance is available for review and inspection at the Town Clerk's Office; it will also be available at Town Meeting.)

1510 ADULT USE MARIJUANA CULTIVATION FACILITIES

1510.1 Allowed Locations

Adult use marijuana cultivation facilities shall be allowed in the following locations, subject to the requirements of this Section:

- A. Tier 1, maximum 500 square feet of canopy.
 - 1. All Zones.
 - 2. Tier 1 Adult Use Marijuana Cultivation must follow the requirements of home occupation as defined in the CLUC.
- B. Tier 2, maximum 2000 square feet of canopy.
 - 1. Village 4 Zone and General Purpose 1 Zone. Must be combined with a Medical or Adult Use retail storefront. Maximum of five cultivation sites in each zone.
 - 2. Farm and Forest Zone. Maximum of five cultivation sites.
 - 3. Total number of Tier 2 cultivation sites among the three zones cannot exceed a maximum of 10.
- C. Tier 3, maximum 7000 square feet of canopy.
 - 1. Farm and Forest Zone. Maximum of five cultivation sites.
- D. Tier 4, 20,000 square feet of canopy. Not permitted.
- E. Tier 2, and 3 are permitted uses requiring Planning Board approval and licensing from the Town to operate.

1510.2 Security

Adult use marijuana cultivation facilities must meet the following security requirements:

- A. Shall have lockable doors and windows and shall be served by an alarm system.
- B. Shall have video surveillance capable of covering the exterior and interior of the facility. The video surveillance system shall be operated with continuous recording twenty-four hours per day, seven days per week and video shall be retained for a minimum duration of thirty (30) days. Such records shall be made available to law enforcement agencies when investigating a criminal complaint.

1510.3 Ventilation

Adult use marijuana cultivation facilities must be in an enclosed structure and shall have an odor mitigation system installed and operational that will provide odor control sufficient to ensure that no odors are perceptible off the premises or to neighboring tenants.

Article?. Shall an ordinance entitled "2019 Amendments to the Poland Comprehensive Land Use Code to Authorize and Regulate Adult Use Marijuana Products Manufacturing Facilities" be enacted? (A copy of the proposed ordinance is available for review and inspection at the Town Clerk's Office; it will also be available at Town Meeting.)

1511 ADULT USE MARIJUANA PRODUCTS MANUFACTURING FACILITIES

1511.1 Allowed Locations

Adult use marijuana products manufacturing facilities shall be allowed in the following locations, subject to the requirements of this Section:

- A. Allowed in all locations where adult use cultivation is allowed and must be co-located with a cultivation facility.
- B. This is a permitted use requiring Planning Board approval and licensing by the Town to operate.

1511.2 Security

Adult use marijuana products manufacturing facilities must meet the following security requirements:

- A. Shall have lockable doors and windows and shall be served by an alarm system.
- B. Shall have video surveillance capable of covering the exterior and interior of the facility. The video surveillance system shall be operated with continuous recording twenty-four hours per day, seven days per week and video shall be retained for a minimum duration of thirty (30) days. Such records shall be made available to law enforcement agencies when investigating a criminal complaint.

1511.3 Ventilation

Adult use marijuana products manufacturing facilities must be in an enclosed structure and shall have an odor mitigation system installed and operational that will provide odor control sufficient to ensure that no odors are perceptible off the premises or to neighboring tenants.

Article?. Shall an ordinance entitled "2019 Amendments to the Poland Comprehensive Land Use Code to Authorize and Regulate Adult Use Marijuana Testing Facilities" be enacted? (A copy of the proposed ordinance is available for review and inspection at the Town Clerk's Office; it will also be available at Town Meeting.)

1512 ADULT USE MARIJUANA TESTING FACILITIES

1512.1 Allowed Locations

Adult use marijuana testing facilities shall be allowed in the following locations, subject to the requirements of this Section:

- A. Allowed in all zones.
- B. This is a permitted use requiring Planning Board approval and licensing by the Town to operate.

1512.2 Security

Adult use marijuana testing facilities must meet the following security requirements:

- A. Shall have lockable doors and windows and shall be served by an alarm system.
- B. Shall have video surveillance capable of covering the exterior and interior of the facility. The video surveillance system shall be operated with continuous recording twenty-four hours per day, seven days per week and video shall be retained for a minimum duration of thirty (30) days. Such records shall be made available to law enforcement agencies when investigating a criminal complaint.

1512.3 Ventilation

Adult use marijuana testing facilities must be in an enclosed structure and shall have an odor mitigation system installed and operational that will provide odor control sufficient to ensure that no odors are perceptible off the premises or to neighboring tenants.

Article?. Shall an ordinance entitled "2019 Amendments to the Poland Comprehensive Land Use Code to Authorize and Regulate Adult Use Marijuana Stores" be enacted? (A copy of the proposed ordinance is available for review and inspection at the Town Clerk's Office; it will also be available at Town Meeting.)

1513 ADULT USE MARIJUANA STORES

1513.1 Allowed Locations

Adult use marijuana stores shall be allowed in the following locations, subject to the requirements of this Section:

- A. Village 4 Zone.
- B. General Purpose 1 Zone.
- C. Each zone, Village 4 and General Purpose 1, may have a combined maximum of five medical and/or adult use storefront sites. A site may contain one or more storefronts provided they are all within the same building.
- D. This is a permitted use requiring Planning Board approval and licensing by the Town to operate.

1513.2 Signs

A. Signage for adult use marijuana stores will comply with Chapter 5, Section 508.18 Signs, of this Code, as well as with any applicable requirements of State law or regulation.

1513.3 Security

Adult use marijuana stores must meet the following security requirements:

- A. Shall have lockable doors and windows and shall be served by an alarm system.
- B. Shall have video surveillance capable of covering the exterior and interior of the facility. The video surveillance system shall be operated with continuous recording twenty-four hours per day, seven days per week and video shall be retained for a minimum duration of thirty (30) days. Such records shall be made available to law enforcement agencies when investigating a criminal complaint.

1513.4 Ventilation

Adult use marijuana store must be in an enclosed structure and shall have an odor mitigation system installed and operational that will provide odor control sufficient to ensure that no odors are perceptible off the premises or to neighboring tenants.

Article?. Shall an ordinance entitled "2019 Amendments to the Poland Comprehensive Land Use Code to Add Definitions to Section 1402 Related to Medical Marijuana and Adult Use Marijuana Uses" be enacted? (A copy of the proposed ordinance is available for review and inspection at the Town Clerk's Office; it will also be available at Town Meeting.)

Adult use marijuana: "Adult use marijuana" means marijuana cultivated, manufactured, distributed or sold by a marijuana establishment.

Adult Use Marijuana establishment: "Marijuana establishment" means a cultivation facility, a products manufacturing facility, a testing facility or a marijuana store licensed under state law.

Adult use marijuana product: "Adult use marijuana product" means a marijuana product that is manufactured, distributed or sold by a marijuana establishment.

Adult use marijuana products manufacturing facility: "Marijuana products manufacturing facility" means a facility licensed under state law to purchase adult use marijuana from a cultivation facility or another products manufacturing facility; to manufacture, label and package adult use marijuana and adult use marijuana products; and to sell adult use marijuana and adult use marijuana products to marijuana stores and to other products manufacturing facilities.

Adult use marijuana store: "Adult Use Marijuana Store" means a facility licensed under state law to purchase adult use marijuana, immature marijuana plants and seedlings from a cultivation facility, to purchase adult use marijuana and adult use marijuana products from a products manufacturing facility and to sell adult use marijuana, adult use marijuana products, immature marijuana plants and seedlings to consumers.

Adult use marijuana testing facility: "Adult use marijuana testing facility" means a facility licensed under state law to develop, research and test adult use marijuana, marijuana products and other substances.

Disqualifying drug offense: "Disqualifying drug offense" means a conviction for a violation of a state or federal controlled substance law that is a crime punishable by imprisonment for one year or more, but does not include (1) An offense for which the sentence, including any term of probation, incarceration or supervised release, was completed 10 or more years earlier; or (2) An offense that consisted of conduct that would have been permitted under the Maine Adult Use Marijuana Act.

Home cultivation of marijuana: "Home cultivation of marijuana" means cultivation for personal adult use by a person 21 years of age or older is limited to cultivation of three (3) mature marijuana plants, twelve (12) immature marijuana plants, and an unlimited number of seedlings by each person 21 years of age or older who is domiciled on a parcel or tract of land.

Marijuana cultivation: "Marijuana cultivation" means the planting, propagation, growing, harvesting, drying, curing, grading, trimming or other processing of marijuana for use or sale. "Cultivation" or "cultivate" does not include manufacturing, testing or marijuana extraction.

Marijuana cultivation facility: "Marijuana Cultivation facility" means a facility licensed under state law to purchase marijuana plants and seeds from other cultivation facilities; to cultivate,

prepare and package adult use marijuana; to sell adult use marijuana to products manufacturing facilities, to marijuana stores and to other cultivation facilities; and to sell marijuana plants and seeds to other cultivation facilities and immature marijuana plants and seedlings to marijuana stores. Cultivation facilities are categorized into tiers based on the number of adult plants and usable square footage for cultivation.

Marijuana manufacturing or manufacture: "Manufacturing" or "manufacture" of marijuana means the production, blending, infusing, compounding or other preparation of marijuana and marijuana products, including, but not limited to, marijuana extraction or preparation by means of chemical synthesis. "Manufacturing" or "manufacture" does not include cultivation or testing.

Marijuana odor mitigation: All marijuana establishments, cultivation operation, manufacturing facility, testing, and retail storefronts must be in an enclosed structure and shall have an odor mitigation system installed and operational that will provide odor control sufficient to ensure that no odors are perceptible off the premises or to neighboring tenants.

Marijuana product: "Marijuana product" means a product composed of marijuana or marijuana concentrate and other ingredients that is intended for use or consumption. "Marijuana product" includes, but is not limited to, an edible marijuana product, a marijuana ointment and a marijuana tincture. "Marijuana product" does not include marijuana concentrate.

Medical marijuana establishment: "Medical marijuana establishment" means a medical marijuana registered caregiver retail store, registered dispensary, medical marijuana testing facility, or medical marijuana manufacturing facility.

Medical marijuana manufacturing facility: "Medical marijuana manufacturing facility" means a manufacturing facility authorized under state law to manufacture marijuana products for medical use or to engage in marijuana extraction for medical use.

Medical Marijuana registered caregiver retail store: "Medical Marijuana registered caregiver retail store" means a registered caregiver authorized under state law to cultivate medical marijuana for qualifying patients that operates a retail store to sell medical marijuana to qualifying patients.

Medical marijuana testing facility: "Medical marijuana testing facility" means a public or private laboratory authorized under state law to test medical marijuana for contamination, potency or cannabinoid profile.

Registered dispensary: "Registered dispensary" means a dispensary authorized under state law to cultivate and dispense medical marijuana to qualifying patients and caregivers.

State registration authority: "State registration authority" means the authority created or designated by the state for the purpose of regulating and controlling registration for medical marijuana establishments.

Article ?. Shall an ordinance entitled "2019 Amendments to the Poland Comprehensive Land Use Code to adopt Marijuana Standards" be enacted? (A copy of the proposed ordinance is available for review and inspection at the Town Clerk's Office; it will also be available at Town Meeting.)

CHAPTER 15 – MARIJUANA STANDARDS

<u>1501 SHORT TITLE</u> – This Chapter shall be known as the "Marijuana Standards for the Town of Poland, Maine."

1502 PURPOSE

1502.1 Purposes – The purposes of this Chapter are:

- A. To provide guidance to those wishing to establish any type of marijuana establishment in Poland;
- B. To clarify the approval criteria for each type of marijuana establishment; and
- C. To promote uniform standards.

1503 ADMINISTRATION

- A. Any commercial operation must submit site plans to the Planning Board and is subject to the administration of the Planning Board.
- B. The Code Enforcement Officer is responsible for the final sign off to the State regarding whether or not an operation meets the applicable requirements of the Comprehensive Land Use Code.

1504 MEDICAL MARIJUANA CAREGIVER CULTIVATION

1504.1 Allowed Locations

Medical marijuana caregiver cultivation sites shall be allowed in the following locations, subject to the requirements of this Section:

- A. All Zones
- B. Must follow the requirements of home occupation as defined in the CLUC.

1504.2 Security

Medical marijuana caregiver cultivation sites must meet the following security requirements:

- A. Shall have lockable doors and windows and shall be served by an alarm system.
- B. Shall have video surveillance capable of covering the exterior and interior of the facility. The video surveillance system shall be operated with continuous recording twenty-four hours per day, seven days per week and video shall be retained for a minimum duration of thirty (30) days. Such records shall be made available to law enforcement agencies when investigating a criminal complaint.

1504.3 Ventilation

Medical marijuana caregiver cultivation sites must be in an enclosed structure and shall have an odor mitigation system installed and operational that will provide odor control sufficient to ensure that no odors are perceptible off the premises or to neighboring tenants.



Town of Poland, Maine Planning Board

Minor Subdivision Application Instructions

Instructions:

- 1. Read every part of this document. Failure to follow requirements can and will delay the Planning Board's decisions.
- 2. Fill out the forms And Obtain copies of information as required by the application on these pages.
 - A) Obtain names and addresses of "Abutters" (within 500 feet of your lot) from the Assessor's Office.
 - i) You will need to deliver this list to the Planning Board Office. It is needed for the official abutters' notification to be sent by this office of your request.
 - ii) An "Agenda Request form should be filed at the same time so that a meeting date with proper notification to the abutters can be set.
- Use the "Submission Checklist" to make sure submission requirements are met.
 - A) The checklist is a summary of the standard requirements in Section 606.3 of the Comprehensive Land Use Code.
 - i) The actual Code wording may be found on-line at www.polandtownoffice.org. Go to the "Code Enforcement" page and then select "Comprehensive Land Use Code". Hardcopies are available for purchase at the town office.
 - B) Make sure all waiver requests have a written statement for each request. Check with the Code Enforcement Office to make sure items stated as "On File" are indeed in the town office.
 - Some requirements may need only a one paragraph or one sentence statement. Make sure all requests are answered.
- Make the necessary copies of <u>all</u> information requested.
- 5. NUMBER OF COPIES OF THE APPLICATION AND DUE DATES
 - A) A total of 14 copies of the plans are needed.
 - i) Four (4) of the copies are required for Department Heads and should be attached to the department head notice. (The Recreation Dept. need to be notified but usually does not need a copy of the application)
 - ii) The applicant is responsible for the delivery of the Road Commissioner's, the Fire/Rescue Chief's, the Police Dept, and the School Departments application copies.
 - B) Department heads require at least fourteen days to review the applications. If insufficient time is given to the department heads for review, their response to the Planning Board will delay the Planning Board's decision.
 - C) The Code Enforcement Office must receive an original application (An application with original signatures) and an additional 13 copies for the Board members along with the appropriate fees by 1:00 p.m. eleven (11) days before the stated meeting to remain on the upcoming agenda.
 - D) If review for completeness of information by the Code Enforcement Officer is desired, a copy must be submitted to the CEO at least 14 days prior to the meeting.
 - E) The application will be put on display for public review 7 days prior to the meeting.
 - F) Applications received after the Agenda is posted may not be reviewed by the Board for your scheduled meeting date.

PROJECT NAME:
Date of Planning Board Review: / / / Application #
LOT INFORMATION:
LOT INFORMATION: Tax Assessor's Map # 0005-0024-0003 Lot # 3 Sub-lot #
Lake Watershed: Loper Kange.
Road Location: Cleve IVIDD Road
Lot Size 105,098 Sq. Ft. Water frontageFt. Road Frontage: 27,002 Ft.
Year lot created: (If unknown, give best estimate with "est." after date)
Zoning District(s): Rural Residential 2 Flood Zone:Aquifer Overlay:
Current use of lot: Vacant
LAND OWNER(s); Submit copy of deed and copy of Tax Assessor's information card
Name(s):
Company
Mail Address: 5 IVELL FUELLE Phone: (207) 140-4082
Mechanic Fails, ME 04256
Town/State/Zip Mechanic Falls, ME 04256

	DI 1						
10/11/11		CANT or CONTACT PERSON					
	olica		downer	_Contrac		22	Buyer
If la	ndowi	er, write "Same" below and continue to	next block below. If not th	e landowne	er, submit a letter of permission to	construct on or use	the land, or copy of a
		buy from the landowner, along with the	e following information:				
	me(s						
	mpar						
Ma	il Ad	dress:			Phone:	# 19 TH	
	100000	eran same					
To	wn/S	ate/Zip					
				=			
TH	IS A	PPLICATION IS FOR: (Che	eck all that apply)	100 to 10			
		Commercial	on an inat appiny	100	New Development		
		Industrial		ā	Change in Use		
_		Institutional		_	Expansion of Use		
		Governmental		<u> </u>	Expansion of Structure(s)		
	,	Residential			Resumption of Use		
_					resumption of ose		
		IG LOT CONDITIONS					
1.	Ge	<u>neral</u>	" I I (0.7)			V	/ 1
		i) Does this lot have any exist	ting development? (If	No, go to	next page)	Yes	No
	b.	Is there an existing Well				Yes	No
	C.	Is there an existing Septic Syste				Yes	No
		i) If yes, submit a copy of a se		ng(s) sho	wing size & location.	1	
	d.	Is there an existing Road Entrar				Yes	No
		 i) If yes, will there be any cha 				Yes	No
		ii) (If no, submit copy of appro	opriate road entry app	lication if	entrance is onto a state or	town road.)	,
	e.	Any structures to be removed				Yes	No
		i) If yes, submit information a	bout the structure to I	be remov	ed and how any debris will	be disposed of.	
2.	Exi	sting Land Development & Imp	provements NOT Inc	luding B	uildings		
	a.	Size of lawns					Sq. Ft. or Acres
	b.	Size of fields					Sq. Ft. or Acres
	C.	Size of driveways/roads				×-140,-241	Sq. Ft
	d.	Size of paths & other non-veget	tated areas				Sq. Ft.
	e.	Wetlands already filled				(A	Sq. Ft.
3.	133-3-3	sting Main Structure					
•	a.	Ground Footprint					Sq. Ft.
	b.	- 1회장과 이번 이번 전체적으로 100mm 이번 중요한다. 100mm	erior dimensions of all	(floors)			Sq. Ft.
	C.	Road Frontage Setback	onor amnonolono or an	110010)		\$ - n	Ft.
	d.	Side Setback					t.: Ft.
	e.	Rear Setback					—· `. Ft.
	f.	Distance to Great Pond		Not ann	licable (over 250')	-	 Ft.
		Distance to Stream			licable (over 250')		r Ft.
	g. h.	Distance to Stream Distance to Wetlands			licable (over 250')		r.c. Ft.
				_ivot app	ilicable (over 250)		
	i.	Foundation	Full Decement		Frost Walls	Slab	Piers
	-	-4' A(-)	_Full Basement	-			
4.		sting Accessory Structure(s)					
	a.	Total Number of Structures					— _{Ca Et}
	b.	Total Ground Footprint					Sq. Ft.
	C.	Total Floor Space					Sq. Ft.
	d.	Closest Road Setback				-	Ft.
	e.	Closest Side Setback				4	Ft.
	f.	Closest Rear Setback					Ft.
	g.	Distance to Great Pond	30		olicable (over 250')		Ft.
	h.	Distance to Streams			olicable (over 250')	_	Ft.
	i	Distance to Wetlands		Not and	licable (over 250')		Ft.

PROPOSED Development

SUBMISSION REQUIREMENTS:

- 1. Standard submissions requirements shall follow Section 606.2 Comprehensive Land Use Code.
- Additional information requested by the Planning Board at the Pre-application Sketch Plan and/or the Site Inspection meetings shall be added to the standard submission requirements.
- 3. Information shall be submitted in the order shown in the check list.
- Submit information on status of any necessary state and/or federal permit.

DISCLOSURE: (READ BEFORE SIGNING)

- 1. I hereby acknowledge that I have read this application and pertinent sections of the ordinances, and state that the information in this document is to the best of my knowledge true and accurate. I agree to comply with all the Town of Poland's ordinances and the State of Maine's statutes regulating the activities sought in this application as well as any permit(s) approved for this application.
- I understand that all construction of structures shall conform to the Maine Uniform Building and Energy Code and the NFPA-101 Life Safety Code, 2006
- 3. I understand that any approval is valid for only the use(s) as specified in this application. The permitting authority must approve any change(s) made to the use(s) sought in the application. Any approval issued for this application is approved on the basis of truthful information provided by the applicant(s), and as allowed by the ordinances of the town.
- 4. I understand that it is my responsibility to assure that the lot description herein accurately describes its ownership, its boundary lines, and the setback measurements from the legal boundary lines.
- 5. I understand that I have the burden of proof as to the legal right to use the property, and that approval of this application in no way relieves me of this burden. Any approval issued does not constitute a resolution in favor of me or the landowner in any matters regarding the property boundaries, ownership, or similar titles.
- 6. I understand that all necessary **Building and Use Permits** shall be secured from the Code Enforcement Office after the Planning Board grants approval of this application.
- I understand that a Certificate of Occupancy shall be required prior to the start of any use or occupancy associated with this application unless
 a signed written waiver is issued with the permit. Fines and penalties may be issued if use or occupancy is started prior to the issuance of the
 certificate.
- 8. I understand that the **approval becomes invalid if** construction or use has not commenced within twelve (12) months of the approval date, construction is suspended for more than six (6) months and no notice for just cause is submitted prior to the end of the six (6) months, or it is found that false statements have been furnished in this application.
- 9. I understand that if I fail to comply with the aforementioned statements, a "STOP WORK" order may be issued for which I will immediately halt any construction and/or use(s) that are approved for this application. This failure may also require that I return the property to its natural state or as closely thereto before the use(s) was/were approved.
- 10. I understand that failure to follow these requirements will lead to Violation Notices and Citations that have fines and penalties. This in turn can lead to civil proceedings in District Court.
- 11. I understand that all state and federal permits are my responsibility as the applicant and/or owner.
- 12. Anything agreed to verbally or in writing during the application process must be adhered to and will be enforced.

Applicant's Signature

1/29/19 Date

Submissions CHECKLIST:

Plan Name:

The following list is the information required by Chapter 606.3 of the Comprehensive Land Use Code for the Town of Poland, Maine for a Minor Subdivision Plan Application. Please check in the columns on the left if the information has been provided, a request of a waiver, or you believe the information is not applicable to your application. If a waiver is requested, or the information is not applicable, a written explanation is required.

For Applicant Use		ant Use		For Planning Board Use			rd Use
Provided	Waiver Request	Not Applicable		Received	On File	Waived	Not Applicable
			Section 606.3 Submission requirements				
1			A. Application form				
		/	B. Location Map				
/		V	B.1. Existing subdivision In proximity				
V			B.2. Location & names of streets				
V		/	B.3. Zoning Districts				
/			B.4. Outline of subdivision & holdings				
//	H.		C. Final Plan				
1/			* 2 Originals plans				
/		/	* 12 copies of application	economic —			
		/	* Board Endorsement area				
3/			* Digital form of plan				
			D.1. Proposed name				
/		VIO	D.2. Names & address of all involved				
V			D.3. Date plan prepared	Y			
,		1/	D.4. Existing physical features	ξ.			
/			* Number of acres				
./			* Property lines				
ν			* Existing buildings				
		1	* Vegetative cover type				
		1/	* Trees >24" DBH				
/		1	* Clearing area for lawns & structures				
/			* Restrictions on clearings				
1		1	D.5. Location of water bodies				
		1	D.6. Contours at requested intervals				
/		V	D.7. Zoning Districts			Viewer-	
1			D.8 Location, names, widths of:				
./			* Roads				
V		/	* Easements				
		/	* Buildings				
		1/	* Parks	ii.			
102	-	1/	* Open Spaces				
1			D.9. Title, rights, & interests				
1			D.10. Standard boundary survey				
1/			D.11. Copy of most recent deed on parcel				
		1	D.12. Intended deed restrictions				
		1/	D.13. Type of sewage disposal				
1/		MA	D.13.a. SSWS pit locations & analyses			4.	
V			D.14.a. Public water supply approval				
		1	D.14.b. Private wells adequate supply				
/			D.14.c. Adequate central supply				
V		,	D.15. Wetland identified				
v			D.16. Phosphorous analysis				
			D.17. Location of sewers, water mains, culverts, & drainage				
			ways				

TOTIGOG	Request	Not Applicable		Received	On File	Waived	Not Applicabl
		V	D.19. Dedicated public use land				
		V	* Offers to town				
		V	* Selectmen look favorable on offer				
		V	D.20. Flood prone areas				
		1	D.21. Hydrogeologic assessment				
- Somi		1/	D.21.a. Sand & gravel aquifers				
		/	D.21.b. Average dwelling density				
		-/	* Potential for adverse impact				
		//	D.22. Storm water management plan				
		V	D.23. Erosion & sedimentation plan				
		1/	D.23.a Permit from DEP			77 - 7 - A	
		V	D.24 Areas of wildlife habitat				
			D.25. Areas on NRHP				
		1//	D.26. Disposal of debris				
			* DEP permit				
		//	D.27. Scenic sites				
		V	D.28. Agricultural areas				
			D.29. Archeological resources		1		
		V	D.30. Technical & financial ability				
			Supplemental Information				
/			Notification of fire, rescue, road, & school departments				
			Status of State and/or Federal permits				
			Condition A.				
447 ta			Condition B.				
			Condition C.				
			Condition D.				
			Condition E.				
			Condition F.				
view y vote yes,	process. e of the Bo an onsite in	ard this application	on requires an on-site inspection: duled for/ at:	YesA		No PM	0.000,000,000,000,000,000
						,	
n, sit		g Board Chair				/ Date	<i>I</i> .
n-site	<i>Plannin</i> e Inspectio	on	Requirements Met	t Deficient	Waive		/ Not Applicab
	e Inspectio	on ITEN		t Deficient			/ Not Applicab
. Les	e Inspections than 6 in	on ITEN ches of snow on	the ground	t Deficient			/ Not Applicab
. Les	e Inspections than 6 in auctures, roa	on ITEN ches of snow on ads, parking, etc.	the ground	t Deficient			/ Not Applicat
. Les . Stru	e Inspections than 6 in actures, roatice of inspections	on ITEN ches of snow on ads, parking, etc. ection posted	the ground	t Deficient			/ Not Applicab

For Planning Board Use

For Applicant Use

F. PB set contour intervals for formal application

Town of Poland

Planning Board DEPARTMENTAL REVIEW OF PROPOSED SITE APPLICATION

Date://	
To:	In accordance with Chapter 606, Site Review, of the Comprehensive Land Use Code for the Town of Poland, an applicant for development approval is required to ask that Municipal Departments to comment on their capacity of capital facilities to serve a proposed development. Therefore, the Planning Board, by way of the applicant, is notifying you of the following proposed project and requests your comments
Applicant: Holly L Russell Address: 15 Tirrell flyenue Mechanic Falls, ME	04256
	# Sublot #
Road Location: Leve Tripp Read	
Project overview:	
Applicants: 1. Should attach all relevant sections of their plans to prevent	(4)
Mail this form letter along with a copy of the application s to the scheduled meeting. (See reverse for list of Departn	o that each department head <u>receives</u> it at least fourteen days prior nent Heads)
	vered their response to the Planning Board Office in time for the
For the D	epartment Head
I have reviewed this application and provide the following: The project has no impact on the Department. The Department has adequate existing capital facil	
Signed:	Date://
Head of Department	
RETURN THIS FORM TO:	
Please return by: Date://	Planning Board Office Town of Poland 1231 Maine Street Poland, Maine 04274-7328

Town of Poland Department Heads

FIRE/ RESCUE CHIEF MARK BOSSE

1231 Maine Street Poland, ME 04274 998-2361 (Dispatch)

ROAD COMMISSIONER BYRON A. STROUT

1231 Maine Street Poland, ME 04274

998-4601

SUPERINTENDENT OF SCHOOLS KENNETH HEALEY

1146 Maine Street Poland, ME 04274

998-2727

POLICE DEPARTMENT CHIEF DEPUTY WILLIAM GAGNE

Androscoggin Sheriff's Office

2 Turner Street Auburn, ME 04210 998-2361 (Dispatch) Planning Board Office Town of Poland 1231 Maine Street Poland, ME 04274

To Whom It May Concern:

I purchased a piece of property located on the Cleve Tripp Road in Poland, Maine on October 1st, 2018, more specifically described as Map #: 0005-0024-0003, Lot #: 3. This lot had stipulations that I have to "share" a driveway with the lot to the left of me. I do not want to have a shared driveway with anyone and therefore I have contacted the Town of Poland and on October 16th, 2018 I met with Adam who works for the Town of Poland road department and he is in agreement that my driveway does not need to be shared and the visibility would be better suited for a different location.

At this time, I am asking the Town of Poland to allow me to change the location of the current "shared driveway" to the new location on Cleve Tripp Road.

Thank you

15 Tirrell Avenue

Mechanic Falls, ME 04256



Subject Property:

Parcel Number:

0005-0024-0003

CAMA Number:

0005-0024-0003

Property Address: CLEVE TRIPP RD

Mailing Address: COFFIN, AUBREY L.

128 RANGE HILL RD.

POLAND, ME 04274

Abutters:

Parcel Number: CAMA Number: 0005-0022

0005-0022

Property Address: CLEVE TRIPP RD.

Parcel Number: CAMA Number:

0005-0022C 0005-0022C

Property Address: 59 CLEVE TRIPP RD.

Parcel Number:

0005-0023

CAMA Number:

0005-0023

Property Address: 47 CLEVE TRIPP RD.

Parcel Number: CAMA Number:

0005-0023A 0005-0023A

Property Address:

37 CLEVE TRIPP RD.

Parcel Number: CAMA Number:

0005-0024 0005-0024

Property Address: CLEVE TRIPP RD.

Parcel Number: CAMA Number:

0005-0024-0001 0005-0024-0001

Property Address: 42 CLEVE TRIPP RD.

Parcel Number:

0005-0024-0002 0005-0024-0002

CAMA Number: Property Address:

CLEVE TRIPP RD

Parcel Number:

0005-0024-0004

CAMA Number: Property Address: CLEVE TRIPP RD.

0005-0024-0004

Parcel Number:

0005-0024-0005 0005-0024-0005

CAMA Number:

Property Address: CLEVE TRIPP RD.

Parcel Number:

0005-0024-0006 0005-0024-0006

CAMA Number:

Property Address: 10 BLACK DUCK LANE

Mailing Address: FOSTER, GLENN

P. O. BOX 157

GRAY, ME 04039

Mailing Address: LEMMINGS, LACEY M.

108 BLUEBERRY LN

GRAY, ME 04039

Mailing Address: LAJOIE, ADAM M.

47 CLEVE TRIPP RD. POLAND, ME 04274

Mailing Address: HARVEY, VICTORIA Y. 37 CLEVE TRIPP RD.

POLAND, ME 04274

Mailing Address: COFFIN, AUBREY L.

128 RANGE HILL RD.

POLAND, ME 04274

Mailing Address: PARSHALL, JACK P.

42 CLEVE TRIPP RD. POLAND, ME 04274

Mailing Address: COFFIN, AUBREY L.

128 RANGE HILL RD.

POLAND, ME 04274

Mailing Address: BERGERON, RAYMOND

31 SANDERSON ROAD POLAND, ME 04274

Mailing Address: GRAVEL, ANN-MARIE

270 MARKED TREE RD. HOLLISTON, MA 01746

Mailing Address: GRAVEL, PAUL B.

10 BLACK DUCK LANE POLAND, ME 04274



Parcel Number: CAMA Number: 0005-0024D

0005-0024D

Property Address: 34 ECHO COVE LANE

Mailing Address: MONTESI, ANTHONY R.

69 GARNET RD

ROXBURY, CT 06783

Parcel Number:

0007-0021

0007-0021

CAMA Number:

Property Address: BAILEY HILL RD.

Mailing Address: HUNTRESS, FRED A., JR.

67 STROUT RD. POLAND, ME 04274

Parcel Number: CAMA Number: 0007-0021

Property Address: BAILEY HILL RD.

0007-0021-0006

Mailing Address: HUNTRESS, FRED A., JR.

67 STROUT RD. POLAND, ME 04274

Parcel Number: CAMA Number:

0036-0001A

0036-0001A

Property Address: 61 ECHO COVE LANE

Mailing Address: WELCH, BENJAMIN L.

8 MEDINAH CIRCLE FALMOUTH, ME 04105

Parcel Number: **CAMA Number:** 0036-0002 0036-0002

Mailing Address: COFFIN, AUBREY L.

128 RANGE HILL RD. POLAND, ME 04274

Property Address: 47 ECHO COVE LANE

Mailing Address: COLETTI, MICHAEL P.

111 CROSS LANE BEVERLY, MA 01915

Parcel Number: CAMA Number: 0036-0003 0036-0003

Property Address: 67 BLACK DUCK LANE

Mailing Address: STICKNEY, MICHAEL R.

61 WEYMOUTH ST. BRUNSWICK, ME 04011

Parcel Number: CAMA Number: 0036-0004 0036-0004

Property Address: 63 BLACK DUCK LANE

Mailing Address: DRISCOLL, THOMAS

Parcel Number: CAMA Number: 0036-0005 0036-0005

84 OAKRIDGE ST.

Property Address: 53 BLACK DUCK LANE CHICOPEE, MA 01020

Parcel Number: CAMA Number: 0036-0006

0036-0006

Property Address: 45 BLACK DUCK LANE

Mailing Address: MANNETT, THOMAS W., III

1 AMBIANA AVE. RAYMOND, ME 04071

Parcel Number: CAMA Number:

0036-0007-0008 0036-0007-0008

Mailing Address: MANNETT, THOMAS W., III

1 AMBIANA AVE. RAYMOND, ME 04071

Property Address: 41 BLACK DUCK LANE

0036-0009

Mailing Address: JOPE, ROGER

Parcel Number: CAMA Number:

0036-0009

Property Address: 39 BLACK DUCK LANE

39 BLACK DUCK LANE POLAND, ME 04274

Parcel Number: CAMA Number: 0036-0010 0036-0010

Property Address: 37 BLACK DUCK LANE

Mailing Address: STONE, CHARLES H., JR.

170 PINE ST.

NORTON, MA 02766





Parcel Number: CAMA Number: 0036-0011

0036-0011

Property Address: 31 BLACK DUCK LANE

NORFOLK, MA 02056

100 ROCKWOOD RD.

Parcel Number: CAMA Number: 0036-0011A 0036-0011A

Property Address: BLACK DUCK LANE

Mailing Address: WATSON, PATRICIA L.

100 ROCKWOOD RD. NORFOLK, MA 02056

Parcel Number: CAMA Number: 0036-0012

0036-0012

Property Address: 29 BLACK DUCK LANE

Mailing Address:

LIMOGES, GLORIA B.& ROBERT D.

Trustees

Mailing Address: WATSON, PATRICIA L.

29 BLACK DUCK LANE POLAND, ME 04274



WARRANTY DEED

(Statutory Short Form)

Aubrey L. Coffin, of Poland, Androscoggin County, Maine, for consideration paid, grants to Holly Russell, of Mechanic Falls, Androscoggin County, Maine whose mailing address is 15 Tirrell Avenue, Mechanic Falls, ME 04256, with Warranty Covenants, a certain lot or parcel of land situated on the southerly side of the Cleve Tripp Road in Poland, Androscoggin County, Maine, more particularly described as follows:

See Schedule "A" attached hereto and made a part hereof.

Meaning and intending to convey a portion and a portion only of the premises conveyed to Aubrey L. Coffin and Dorothy C. Coffin by deed of Guy Chipman and Merle Chipman recorded in the Androscoggin County Registry of Deeds in Book 919, Page 102.

The grantor is the surviving joint tenant his late wife, Dorothy C. Coffin having died on June 23, 2017 a resident of Poland Spring, Androscoggin County, Maine. Reference is made to Androscoggin County Probate Court Docket #2017-362

Dated: October 2nd , 2018

Aubrey & Coffin

Aubrey L. Coffin

STATE OF MAINE Androscoggin, ss.

October <u>2nd</u>, 2018

Personally appeared, before me, the above-named Aubrey L. Coffin and acknowledged the foregoing instrument to be his free act and deed.

Notary Public/Attorney

MARY Elen TAYLOR

Print Name

CoffinA\34.01\WDLot#3WD

SCHEDULE "A"

A certain lot or parcel of land located on the Southerly side of the Cleve Tripp Road in Poland, Androscoggin County, Maine, more particularly described as follows:

Lot 3 as depicted on the recorded plan of Echo Cove Subdivision recorded at the Androscoggin County Registry of Deeds in Plan Book 40, Page 150.

Lot 3 is conveyed subject to all easements, terms, buffers, restrictions and conditions shown on said Plan including the four Deed Restrictions & Requirements and the thirteen Notes as stated on said Plan.

Being a portion and a portion only of the premises conveyed by Guy Chipman and Merle Chipman to Aubrey L. Coffin and Dorothy C. Coffin recorded at the Androscoggin County Registry of Deeds in Book 919, Page 102.



Pine Tree Engineering, Inc.

53 Front Street Bath, Maine 04530 (207) 443-1508 Fax: (207) 442-7029 E-mail: pte@pte-maine.com

#18001

January 30, 2019

Town of Poland Attn: Scott Neal, CEO 1231 Maine Street Poland, Maine 04274

Subject: Poland Corner Road Water Main Extension

Stream Crossing and Water Storage Tank

Poland, Maine

Dear Scott:

On behalf of the Mechanic Falls Water Department, enclosed are one original, one electronic disk, and nine copies of the Formal Site Plan Review Application for the Poland Corner Road Water Main Extension project. The water main will be extended in order to provide public water to additional neighborhoods in Poland.

The purpose of the water storage tank is to increase the volume of water available at this end of town to improve fire protection services and water supply for any high use situations. An easement will be granted to the Mechanic Falls Water Department for the installation of the storage tank. The disturbed land around the storage will be loamed and seeded and maintained as lawn.

The following items are also included with this application:

- Site Location Map
- National Flood Hazard Map
- Abutter Information
- Proposed Legal Description and Easement for Water Storage Tank
- Construction Drawings (one set 24" x 36", nine sets 11" x 17")
- Natural Resources Protection Act Permit-by-Rule application

Highway Opening and Utility Location permits will be obtained from the Maine Department of Transportation since Poland Corner Road is a State Aid roadway.

Scott Neal, CEO January 30, 2019 Page 2

Please let me know if you have any questions or comments regarding this submittal.

Sincerely,

PINE TREE ENGINEERING, INC.

Robert L. Prue, P.E. Project Manager

RLP/szd Enclosures

c: Steve French, Superintendent, Mechanic Falls Water Department



Town of Poland, Maine Planning Board

Formal Site Plan Review

Instructions:

- 1. Read every part of this document. Failure to follow requirements can and will delay the Planning Board's decisions.
- 2. Fill out the forms on pages 1 through 6. Obtain or get copies of information as required by the application on these pages.
- 3. Use the "Submission Checklist" on pages 5 and 6 to make sure submission requirements are met.
 - a. The checklist is a summary of the standard requirements in Section 509.8 of the Comprehensive Land Use Code.
 - i. The actual Code wording may be found on-line at www.polandtownoffice.org. Go to the "Code Enforcement" page, select "Comprehensive Land Use Code" at that bottom of the page. Hardcopies are available for purchase at the town office.
 - b. Make sure all waiver requests have a written statement for each request. Check with the Code Enforcement Office to make sure items stated as "On File" are indeed in the town office.
 - c. Some requirements may need only a one paragraph or one sentence statement. Make sure all requests are answered.
- 4. NUMBER OF COPIES OF THE APPLICATION AND DUE DATE
 - a. A total of at least ten (10) copies of the plans and one PDF copy (on either cd or usb) are needed. Be sure to make a copy for yourself.
 - b. The Code Enforcement Office must receive the original application, an additional 9 copies, and a digital PDF copy (either cd or usb) with appropriate fees by 1:00 p.m. eleven (11) days before the stated meeting to be put on the upcoming agenda.
 - c. If review for missing information by the Code Enforcement Officer is desired, a copy must be submitted to the CEO at least 14 days prior to the meeting.
 - d. The application must be on file for public review for at least 10 days prior to the meeting. Applications received after the Agenda is posted may not be reviewed by the Board for your scheduled meeting date.
- 5. Check with this office to make sure that all departments have responded to your application prior to the meeting.

PROJECT NAME: Poland Corner Road Water Main Extension Stream Crossing and Water Storage Tank
Date of Planning Board Review: / / Application #
LOT INFORMATION:
Tax Assessor's Map # 11 (tank) Lot # Portion of #6B to be joined to #5 (tank)
Watershed: Range Brook (crossing)
Property's Road Location: Poland Corner Road
Lot Size: <u>.21 (tank)</u> Acres Road Frontage: <u>150</u> Ft.
Year lot created: 2019 (If unknown, give best estimate with "est." after date)
Zoning District(s): <u>Rural Residential III</u> Flood Zone: <u>Yes</u> Aquifer Overlay: <u>No</u>
Current use of lot: Vacant
LAND OWNER(s):
Name(s) Orrin Welch (tank) - easement to Water Department Town of Poland right-of-way (crossing)
Company Town of Mechanic Falls Water Department
Company 10wh of Mechanic Lans water Department
Mail Address: 62 Highland Ave. Main Phone 207-345-5351
wall Floric 20 F 3 to 555 1
Town/State/Zip Mechanic Falls, ME 04256 Alternate Phone: 207-212-7116
Alleriale Filoto. 207 212 7110

Applicar If lando permiss informa	nt is: wner, write sion to con	$\frac{\text{DNTACT PERSON:}}{X} \text{Landowner} \qquad \text{Contractor} \qquad \text{Renter} \qquad \text{Buyer}$ e "Same" below and continue to next block below. If not the landowner, submit a struct on or use the land, or copy of a contract to buy from the landowner, along $\frac{\text{Same}}{X} \text{Same}$	
Compar	_ าy		
Mail Add	dress: _	Main Phone:	
Town/Si	tate/Zip <u> </u>	Alternate Phone:	
Com Indu Insti	PPLICATIO nmercial istrial tutional ernmental in Space	New Development Change In Use Expansion of Use Expansion of Structure(s) Resumption of Use	
(This pa 1. Ger Doe X a.	nge is to des neral es this lot ha No Is there ar Ls there ar	DNDITIONS: Scribe what is on your lot currently) ave any development? (If No, go to "Proposed Development") n existing Well No n existing Septic System	Yes Yes Yes
c. d.	i) If yes Is there ar I) i) If yes i) (If no. Any struct	s, submit a copy of a septic permit, or drawing(s) showing size & location. n existing Road Entry No , will there be any changes/modifications? No , submit copy of appropriate road entry application if entrance is onto a state or town roures to be removed	Yes Yes pad.) Yes
2. Exi a. b. c. d. e.	i) If yes sting Land Size of lav or Acres Size of fie or Acres Size of dri Size of oth	s, submit information about the structure to be removed and how any debris will be dispositely be been applied to be removed and how any debris will be dispositely be been applied to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to be removed and how any debris will be dispositely be a submitted to	Sq. FtSq. FtSq. FtSq. FtSq. FtSq. Ft. Sq. Ft.
	sting Main Ground Fo Total Gros	Structure	Sq. Ft. Sq. Ft. St

	d.	Side Setback				Ft.
	e.	Rear Setback				Ft.
	f.	Distance to Great Pond		Not applicable (over 250')		Ft.
	g.	Distance to Stream		Not applicable (over 250')		Ft.
	ň.	Distance to Wetlands		Not applicable (over 250')		Ft.
1.	Fo	undation Type	Full Basement	Frost Walls	Slab	Piers
5.	Ex	isting Accessory Structure(s)	_			
	a.	Total Number of Structures				
	b.	Total Ground Footprint				Sq. Ft
	C.	Total Floor Space				Sq. Ft
	d.	Closest Road Setback				Ft.
	e.	Closest Side Setback				Ft.
	f.	Closest Rear Setback				Ft.
	g.	Distance to Great Pond		Not applicable (over 250')		Ft.
	h.	Distance to Streams		Not applicable (over 250')		Ft.
	i.	Distance to Wetlands		Not applicable (over 250')		Ft.
Ó.	To	tal Existing Impervious Surfaces				Sq. Ft
	a.	Add 2c +2d + 3a + 5b				
PR	OPC	SED DEVELOPMENT:				
۱.		etlands to be impacted			0	Sq. Ft
2.		w footprint(s) and developed area(s	s):		201	0 5
		Changes in building footprint(s)			301	Sq. Ft
	b.	Changes in driveway/roadway			0	Sq. Ft
	C. d.	Changes in patios, walkways, etc. TOTAL (2a+2b+2c)			301	Sq. Ft Sq. Ft
3.		rcentage of lot covered by impervio	us surfaces:		3.3%	
	a.	(Equals [areas on line 6 page 2 +		otal lot area measured in sq. ft.]		

SUBMISSIONS:

- 1. Attach drawings and/or statements describing the following items, if applicable:
 - a. Provide a copy of deed and Tax Assessor's information card.
 - b. Provide a map of the general area showing land features within at least ½ mile of this lot.
 - c. Provide site plan(s) of your lot with <u>existing</u> development and its dimensions shown.
 - d. Provide site plan(s) of your lot with proposed development and its dimensions shown.
 - . (May be combined on existing development drawing.)
 - e. Provide detailed plans of proposed structural development and changes.
 - f. Provide statements or drawings of methods of infrastructure:
 - i. Water supply
 - ii. Sewage disposal
 - iii. Fire protection
 - iv. Electricity
 - v. Solid waste disposal
 - g. Type, size, and location of signs.
 - h. Number of parking spaces.
 - i. Provide phosphorus loading calculation if in a great pond watershed area.
 - j. Anticipated date for start of construction.
 - k. Anticipated date for completion of construction.
 - I. Standard submissions requirements shall follow Section 509.8 of the Comprehensive Land Use Code. Copies of the Code are available for viewing at the Town Office and Library. Copies are available for purchase (\$25.00) in the Code Enforcement Office.
 - i. (Use checklist starting on page 6 for summary of usual requirements.)
 - m. Other requirements unique to your project added by the Planning Board.

2. List all state and federal approvals, permits, and licenses required, if any, for the project:

This includes but is not limited to the following:

- 1. State highway entrance permit.
- 2. Soil disturbances involving more than one acre.
- 3. Impact on more than 4,300 square feet of any type wetland.
- 4. Soil disturbances within 100 feet of lakes, rivers or streams.
- 5. Activity within 75 feet, over the water, or in the water of lakes, rivers, or streams.
- Timber harvesting.
- 7. Flood zones.
- 8. Discharges and emissions

DISCLOSURE: (READ BEFORE SIGNING)

- 1. I hereby acknowledge that I have read this application and pertinent sections of the ordinances, and state that the information in this document is to the best of my knowledge true and accurate. I agree to comply with all the Town of Poland's ordinances and the State of Maine's statutes regulating the activities sought in this application as well as any permit(s) approved for this application.
- 2. I understand that all construction of structures shall conform to or exceed the minimum requirements of the Maine Uniform Building and Energy Code, and the NFPA-101 Life Safety Code, 2009
- 3. I understand that any approval is valid for only the use(s) as specified in this application. The permitting authority must approve any change(s) made to the use(s) sought in the application. Any approval issued for this application is approved on the basis of truthful information provided by the applicant(s), and as allowed by the ordinances of the town.
- 4. I understand that it is my responsibility to assure that the lot description herein accurately describes its ownership, its boundary lines, and the setback measurements from the legal boundary lines.
- 5. I understand that I have the burden of proof as to the legal right to use the property, and that approval of this application in no way relieves me of this burden. Any approval issued does not constitute a resolution in favor of me or the landowner in any matters regarding the property boundaries, ownership, or similar titles.
- 6. I understand that all necessary **Building and Use Permits** shall be secured from the Code Enforcement Office after the Planning Board grants approval of this application.
- 7. I understand that a **Certificate of Occupancy or Compliance** shall be required prior to the start of any use or occupancy associated with this application unless a signed written waiver is issued with the permit. Fines and penalties may be issued if use or occupancy is started prior to the issuance of the certificate.
- 8. I understand that the **approval becomes invalid if** construction or use has not commenced within twelve (12) months of the Planning Board's approval date, construction is suspended for more than six (6) months and no notice for just cause is submitted prior to the end of the six (6) months, or it is found that false statements have been furnished in this application.
- 9. I understand that if I fail to comply with the aforementioned statements, a "STOP WORK" order may be issued for which I will immediately halt any construction and/or use(s) that are approved for this application. This failure may also require that I return the property to its natural state or as closely thereto before the use(s) was/were approved.
- 10. I understand that failure to follow these requirements will lead to Violation Notices and Citations that have fines and penalties. This in turn can lead to civil proceedings in District and/or Superior Court.
- 11. I understand that **all state and federal permits** are my responsibility as the applicant and/or owner and will secure the same prior to the start of the project.

Applicant's Signature(s)

Date

Submission CHECKLIST

The <u>following list is a short summary</u> of the information required in Chapter 509.8 of the Comprehensive Land Use Code for the Town of Poland, Maine. Please checkmark or place an "X" in the left-hand columns if the information has been provided, if you request a waiver from submitting the information, or you believe the information is not applicable to your application. If a waiver(s) is requested, or the information is not applicable, a written explanation is required. Columns on the right are for the Planning Board's use.

For	For Applicant Use			For Planning Board Use					
Provided	Waiver Request	Not Applicable	Section 509.8.A Submission requirements	Received	On File	Waived	Not Applicable		
X	,		1. Site Plan Drawings						
X			2. Signed copy of application						
X			3.a. Name & address of owner						
X			Name of development						
X			Name & address of abutters within 500' of lot for development						
X			Map of general location						
X			Show all contiguous properties						
X			Names, Map, & lot #'s on drawings						
X			Copy of deeds, agreements						
X			Engineer/ designer of plans						
			Existing Conditions (Site Plan)						
X			Zoning Districts on and/or abutting project's lot shown						
X			Bearings & Distances shown on drawings						
X			Location of utilities, culverts, drains						
X			Location, name of existing r/w						
		X	Location, dimensions of existing structures						
X			Location, dimensions of existing roads, walks, parking, loading, etc.						
X			Location of intersection within 200'						
X			Location of open drains, wetlands, wildlife areas, historic sites, etc.						
X			Direction of surface drainage						
X			100-yr. Floodplain						
		X	Signs						
X			Easement, covenants, restrictions						
			Proposed Development (Site Plan)						
X			Location & dimensions of all new structures. New development delineated from existing development						
X			Setback dimensions shown & met						
		X	Exterior lighting (Will meet full cutoff requirements)						
		X	Incineration devices						
		X	Noise of machinery and operations						
		X	Type of odors generated						
		X	Septic system and other soils reports						
X			Water supply						
		X	Raw & finished materials stored outside						
X			Contours shown at PB specified intervals						
		X	Curbs, sidewalks, drives, fences, retaining walls, parking, etc.						
		X	Landscaping plan						
X			Easements, r/w, legal restrictions						
X			Abutters' property lines, names						
			TRAFFIC DATA						

For Applicant Use				For Planning Board Use					
Provided	Waiver Request	Not Applicable	Section 509.8.A Submission requirements	Received	On File	Waived	Not Applicable		
		X	Peak hour traffic						
		X	Traffic counts						
		X	Traffic accident data						
		X	Road capacities						
		X	Traffic signs, signals						
			STORMWATER & EROSION						
X			Method for handling stormwater shown						
X			Flow direction						
		X	Catch basins, dry wells, ditches, etc.						
		X	Engineering Analysis of stormwater						
X			Erosion control measures						
		X	Hydrologist groundwater impact						
X			Utility plans for all utilities						
		X	Cross-section profile of roads, walks						
X			Construction drawings of roads, utilities						
		X	Cost analysis of project and financial capability demonstrated						
			Phosphorus control plan if in watershed of a great pond						
		X	Submission of waiver requests						
X			Copies of state, federal applications, permits, &/or licenses required for this project.						
			Condition A.						
			Condition B.						
			Condition C.						
			Condition D.						
			Condition E.						

This application was first looked at by the Planning of the review process.	Board on	<u>//</u> b	out does	not create v	ested rights i	n the initiation
By vote of the Board this application requires an on If yes, an onsite inspection is scheduled for	-site inspecti /	on: /	_at	Yes :	AM	No PM
By vote of the Board this application requires a publif yes, public hearing is scheduled for	lic hearing: /	1	at	Yes :	AM	No PM
Conditions of Approval for Formal Site Review:						
Planning Board Chair				D) Date	1

Site Review and Shoreland Zoning Review Fees:

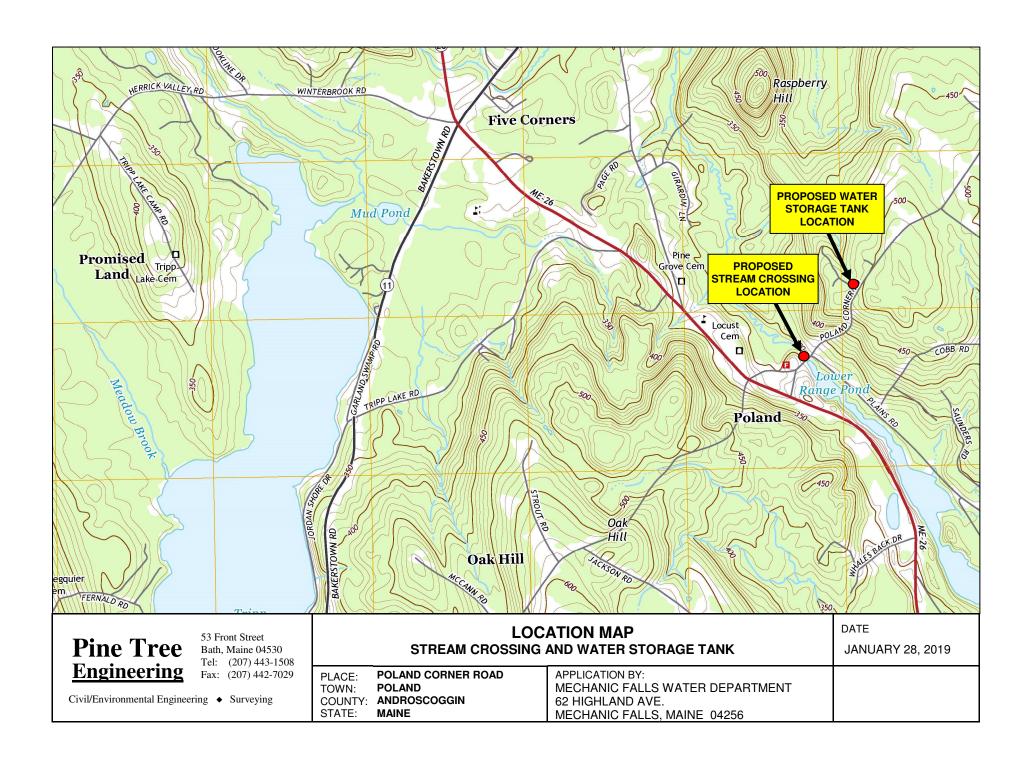
Type of fee	<u>Fee</u>	<u>Units or Comments</u>
Application – sketch plans, Rough design	\$75.00	Each application (no other fees)
Application – formal ³	\$150.00	Each application + fees below
Approval extension, Planning Board Approval only	\$50.00	One extension only (no other fees)
Escrow 4, minimum amount	\$700.00	When required by Planning Board
Extension of approval	\$100.00	Before approval expires
Auto graveyards, recycle business	\$5.00	Per vehicle storage slot (parking space)
Junkyard, Storage Lots	\$1.50	Per ft ² of outside storage
Residential Towers	\$20.00 + \$5.00	Based on Cost of Work
	per \$1,000.00	
Commercial Towers	\$20.00 + \$10.00	Based on Cost of Work
	per \$1,000.00	
Notifications	\$.75	Each Notification, First Class Mail sent by Town

- 1. Building and Structures may include up to five times the footprint area of the building for grounds improvements, exclusive of the building footprint, as part of the building review fee.
- 2. <u>Building and Grounds Improvement Fees</u>. The sum of these two fees may be limited to \$2,500.00 per application at the discretion of the Planning Board. (Junkyards, auto graveyards, recycling business, and towers excluded.
- 3. Reduced Fees: The Planning Board may, upon application therefore, allow a reduced total site review fees to \$50.00 in any case which it determines that the work for which the permit is sought will be performed within the Shoreland Zone. The project shall be intended solely for the purpose of protecting a Great Pond, Stream, River, or other Natural Resources through the implementation of Conservation, Best Management Practices, or other environmental safeguards. Also, the project shall not result in the enlargement of any building or structure or an intensification of the existing use of the property.
- 4. Review Escrow Funds may be used by the Town to pay for professional reviews an advice requested by the Planning Board or Code Enforcement Officer related to the applicant's proposed development. Review escrow funds deposited by the applicant not spent during the course of the Town's review shall be returned to the developer within sixty days after the Planning Board's decision on the application is final. If Professional review and advice fees exceed the amount deposited, the developer shall pay the amount outstanding before final approval or any permit is granted.

	Per vehicle storage slot (parking space)
J	Per ft ² of outside storage
	Based on Cost of Work
	Based on Cost of Work
	Each Notification, First Class Mail sent by Town
_	
-	

Town of Poland, Maine PLANNING BOARD AGENDA REQUEST

Date of meeting you are requesting to be scheduled for: 2 / 12 / 2019 Meetings are normally conducted from 7:00 to 10:00 PM in the Municipal Conference Room at the Town Office Map 11 (tank) Lot 6B Sub-lot
Applicant's Name: Mechanic Falls Water Department, Steve French, Superintendent Mailing Address: 62 Highland Avenue Town, State, Zip: Mechanic Falls, ME 04256
Home Phone: Hours: Work Phone: 345-5351 Hours: M-F 7:30 - 4:00
Type of application: Sketch Plan X Site Review X Shoreland Subdivision Informational Road location for project: Poland Corner Road
Zoning: Rural Residential III Lake Watershed: No Nature of business to be discussed (<i>Brief description</i>): Water main extension, including a stream crossing and installation of a 30,000 gallon water storage tank.
IMPORTANT - READ CAREFULLY: This Office must receive the original application, plus nine (9) copies, a digital PDF copy (on either cd or usb), and appropriate fees by Friday at 1:00 p.m., eleven (11) days before the stated meeting to be put on the upcoming agenda.
 New business is scheduled on the agenda in the order this office receives this form. If you want your application reviewed for contents prior to the meeting, it must be in this office 14 days before the meeting. Should the Board choose to adjourn before all business is addressed, all remaining business will be tabled until the next available meeting. Unfinished business is conducted before new business is addressed. Applicant's Signature: Date: Date:
OFFICE USE ONLY: Request Taken By: Date: Time::a.m. p.m.

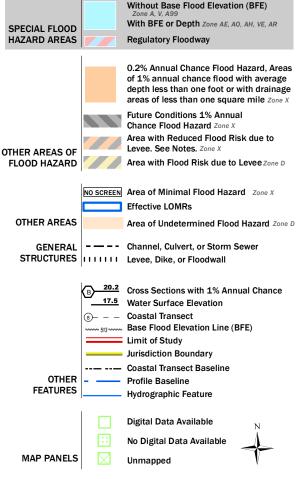


National Flood Hazard Layer FIRMette





SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT



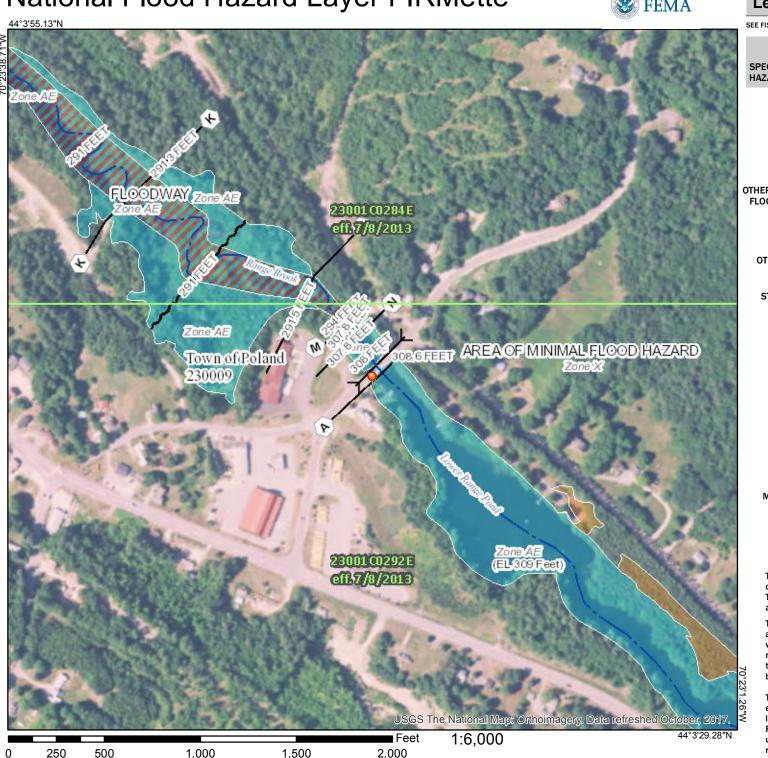


The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 1/25/2019 at 12:01:05 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.





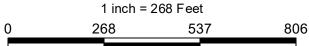


Stream Crossing

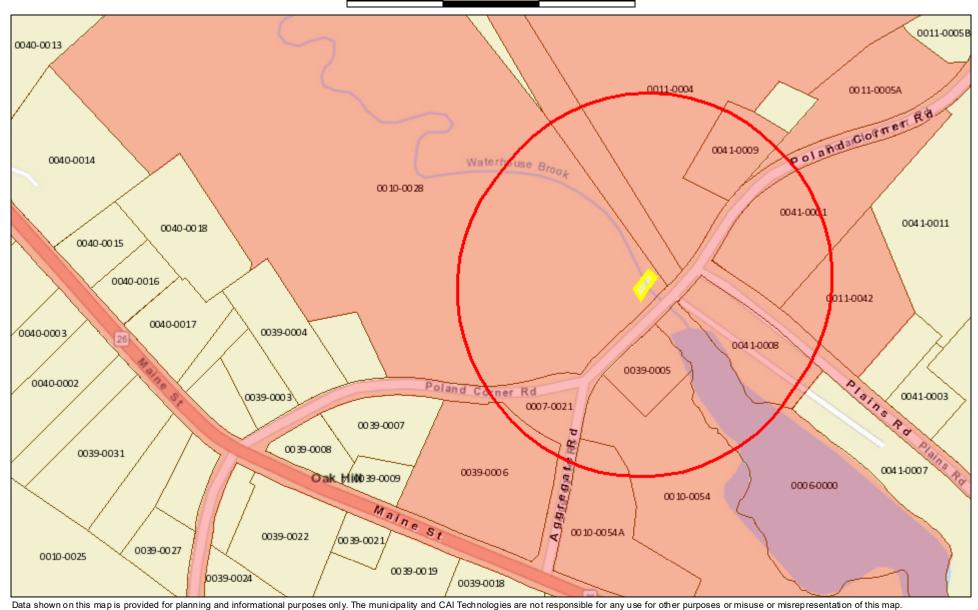
CAI Technologies
Precision Mapping, Geograpial Solutions.

January 25, 2019

Poland, ME



www.cai-tech.com



Abutters Poland Corner Road Water Main Extension Stream Crossing Poland, Maine

Parcel Number	GIS Number	Cama Number	Property Address	Owner Name	Co- Owner Name	Owner Address	Owner Address 2	Owner City	Owner State	Owner Zip
0010-0028	0010-0028	0010-0028	33 POLAND CORNER RD.	POLAND, TOWN OF		1231 MAINE ST.		POLAND	ME	04274
0010-0028	0010-0028	0010-0028- 0001	POLAND CORNER RD.	POLAND, TOWN OF		1231 MAINE STREET		POLAND	ME	04274
0010-0054	0010-0054	0010-0054	MAINE ST.	POLAND, TOWN OF		1231 MAINE ST.		POLAND	ME	04274
0010-0054A	0010-0054A	0010-0054A	3 AGGREGATE RD.	REGIONAL SCHOOL UNIT #16		3 AGGREGATE RD.		POLAND	ME	04274
0011-0004	0011-0004	0011-0004	65 POLAND CORNER RD.	MULLEN, ROBERT D.		25 OLD PLAINS RD.		POLAND	ME	04274
0011-0004-0001	0011-0004- 0001	0011-0004- 0001	POLAND CORNER RD.	POLAND, TOWN OF		1231 MAINE ST.		POLAND	ME	04274
0011-0005A	0011-0005A	0011-0005A	99 POLAND CORNER RD.	HAGGETT, MICHAEL A.		99 POLAND CORNER RD.		POLAND	ME	04274
0011-0042	0011-0042	0011-0042	644 PLAINS RD.	LIBBY, E. BEULAH		1642 WEST SHORES RD.		MELBOURNE	FL	32935
0039-0005	0039-0005	0039-0005	46 POLAND CORNER RD.	JEWELL, DALE K.		P. O. BOX 36		POLAND	ME	04274
0039-0006	0039-0006	0039-0006	30 POLAND CORNER RD.	POLAND, TOWN OF		1231 MAINE ST.		POLAND	ME	04274
0041-0001	0041-0001	0041-0001	70 POLAND CORNER RD.	JARVIS, CLARK J.		70 POLAND CORNER RD.		POLAND	ME	04274
0041-0008	0041-0008	0041-0008	645 PLAINS RD.	CROUCH, JOHN E.		P. O. BOX 268		POLAND	ME	04274
0041-0009	0041-0009	0041-0009	69 POLAND CORNER RD.	MARTIN, PATRICK M.		112 BIRCH DRIVE		POLAND	ME	04274
ADDITIONAL	. PROJECT	ROUTE A	BUTTERS:							
0041-0010	0041-0010	0041-0010	77 POLAND CORNER RD.	BOSTON, KENT R.		77 POLAND CORNER RD.		POLAND	ME	04274
0041-0011	0041-0011	0041-0011	POLAND CORNER ROAD	LIBBY, ERNEST G.		1642 WEST SHORES RD.		MELBOURNE	FL	32935





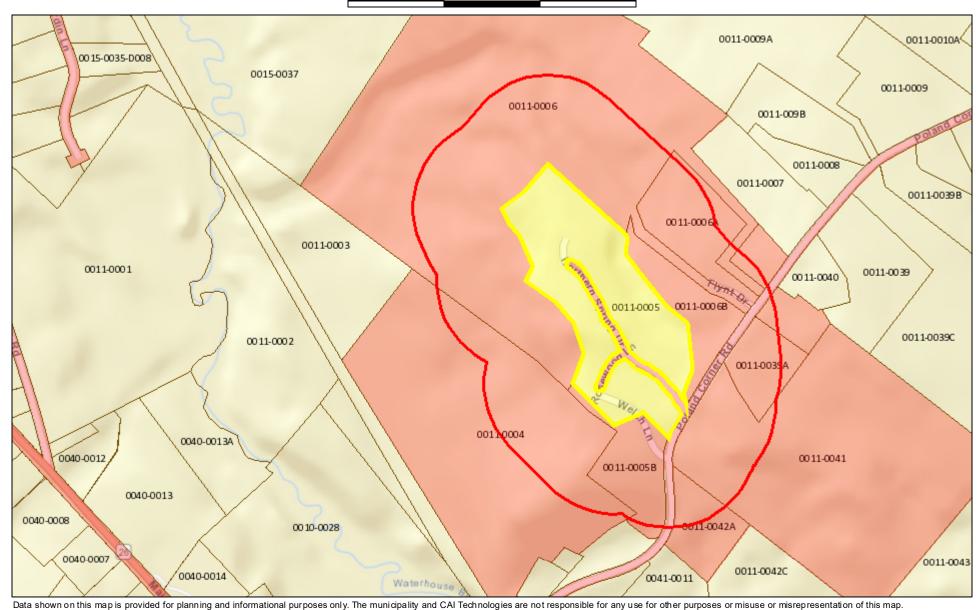
Water Storage Tank

Poland, ME



January 28, 2019

1 inch = 537 Feet www.cai-tech.com 0 537 1075 1613



Abutters Poland Corner Road Water Maine Extension Water Storage Tank Poland, Maine

Parcel Number	GIS Number	Cama Number	Property Address	Owner Name	Owner Address	Owner Address 2	Owner City	Owner State	Owner Zip
0011-0004	0011-0004	0011-0004	65 POLAND CORNER RD.	MULLEN, ROBERT D.	25 OLD PLAINS RD.		POLAND	ME	04274
0011- 0005B	0011-0005B	0011- 0005B	117 POLAND CORNER RD.	TIBBETTS, MICHAEL D.	117 POLAND CORNER RD.		POLAND	ME	04274
0011-0006	0011-0006	0011-0006	POLAND CORNER RD.	FLYNT, WILLIAM A. ET AL	%MARY PIGNATA	600 THACKERAY CT.	ROSEVILLE	CA	95747
0011- 0006A	0011-0006A	0011- 0006A	14 FLYNT DR.	FLYNT, WILLIAM A.	14 FLYNT DR.		POLAND	ME	04274
0011- 0006B	0011-0006B	0011- 0006B	7 FLYNT DR.	FLYNT, JOHN B.	7 FLYNT DR.		POLAND	ME	04274
0011- 0006C	0011-0006C	0011- 0006C	167 POLAND CORNER RD.	THE ISABELLE COTTAGE, LLC	167 POLAND CORNER RD.		POLAND	ME	04274
0011- 0039A	0011-0039A	0011- 0039A	POLAND CORNER RD.	FROST, GREGORY ALAN	55 LUNT LANE		POLAND	ME	04274
0011-0039- A001	0011-0039- A001	0011-0039- A001	164 POLAND CORNER RD.	COLE, SUSAN P.	164 POLAND CORNER RD.		POLAND	ME	04274
0011-0041	0011-0041	0011-0041	POLAND CORNER RD.	SMALL WOODLAND OWNERS	P. O. BOX 836		AUGUSTA	ME	04332
0011- 0042A	0011-0042A	0011- 0042A	112 POLAND CORNER RD.	PERRON, JACOB	112 POLAND CORNER RD.		POLAND	ME	04274

Proposed Legal Description for John B. Flynt & Patricia R. Flynt to Affordable Homes, Inc. Poland Corner Road, Poland, Maine

A certain lot or parcel of land with the improvements thereon, located on the northerly side of the Poland Corner Road, in the Town of Poland, County of Androscoggin, and State of Maine being more particularly bounded and described as follows:

Beginning at a capped rebar set on the northerly sideline of the Poland Corner Road, said point of beginning being the most southerly corner of John B. Flynt as described in a warranty deed from William F. Flynt dated December 03, 1987 and recorded in Book 2188, Page 301 A.C.R.D., Thence, from said point of beginning N 16°34'15" W a distance of 74.62' feet to a point;

Thence, N 45°52'10" E a distance of 114.89' feet to a capped rebar set and the northerly corner of the parcel herein described;

Thence, S 44°32'28" E along remaining land formerly of the said John B. Flynt a distance of 70.96' feet to a capped rebar set on the northerly sideline of the said road;

Thence, S 47°42'17" W along the northerly sideline of the said road a distance of 150.00' feet to a capped rebar set and the point of beginning;

CONTAINING 0.209 Acres.

ALL CAPPED REBAR SET are 5/8th inch diameter rebar with an orange plastic cap inscribed K.F. Farrar PLS # 2021.

BEARINGS are based upon a plat entitled "Boundary Retracement Survey of property located along Poland Corner Road, Poland, Maine, for Affordable Homes, Inc. "dated December 12, 2018 by Kenneth F. Farrar PLS # 2021, and refer to magnetic north;

Meaning and Intending to describe a portion of the premises as described in a Warranty Joint Tenancy Deed John B. Flynt to John B. Flynt and Patricia R. Flynt dated August 20, 2015 and recorded in Book 9211, Page 161, Androscoggin County Registry of Deeds.

prepared by:

Kenneth F. Farrar Maine P.L.S. License # 2021 P.O. Box 368 West Paris, Maine 04289 18034lgl

December 12, 2018



Proposed Legal Description for Water Storage Tank Easement Area Affordable Homes, Inc. to Town of Mechanic Falls Water Department Poland Corner Road, Poland, Maine

A certain lot or parcel of land with the improvements thereon, located on the northerly side of the Poland Corner Road, in the Town of Poland, County of Androscoggin, and State of Maine being more particularly bounded and described as follows:

Beginning at a capped rebar set on the northerly sideline of the Poland Corner Road, said point of beginning being the most southerly corner of John B. Flynt as described in a warranty deed from William F. Flynt dated December 03, 1987 and recorded in Book 2188, Page 301 A.C.R.D., Thence, from said point of beginning N 16°34'15" W a distance of 56.00' feet to a capped rebar set;

Thence, N 43°45'42" E a distance of 123.67' feet to a capped rebar set and the northerly corner of the parcel herein described;

Thence, S 53°57'38" E a distance of 59.00' feet to a capped rebar set on the northerly sideline of the said road;

Thence, S 47°42'17" W along the northerly sideline of the said road a distance of 150.00' feet to a capped rebar set and the point of beginning;

CONTAINING 0.171 Acres.

ALL CAPPED REBAR SET are 5/8th inch diameter rebar with an orange plastic cap inscribed K.F. Farrar PLS # 2021.

BEARINGS are based upon a plat entitled "Plan Showing a Proposed Water Storage Tank Easement Area of property located along Poland Corner Road, Poland, Maine, for the Mechanic Falls Water Department "dated December 12, 2018 by Kenneth F. Farrar PLS # 2021, and refer to magnetic north;

prepared by:

Kenneth F. Farrar Maine P.L.S. License # 2021 P.O. Box 368 West Paris, Maine 04289 18034agl

December 12, 2018



DEPARTMENT OF ENVIRONMENTAL PROTECTION PERMIT BY RULE NOTIFICATION FORM

(For use with DEP Regulation, Natural Resouces Protection Act- Permit by Rule Standards, Chapter 305)

PLEASE TYPE OR PRINT IN BLACK INK ONLY

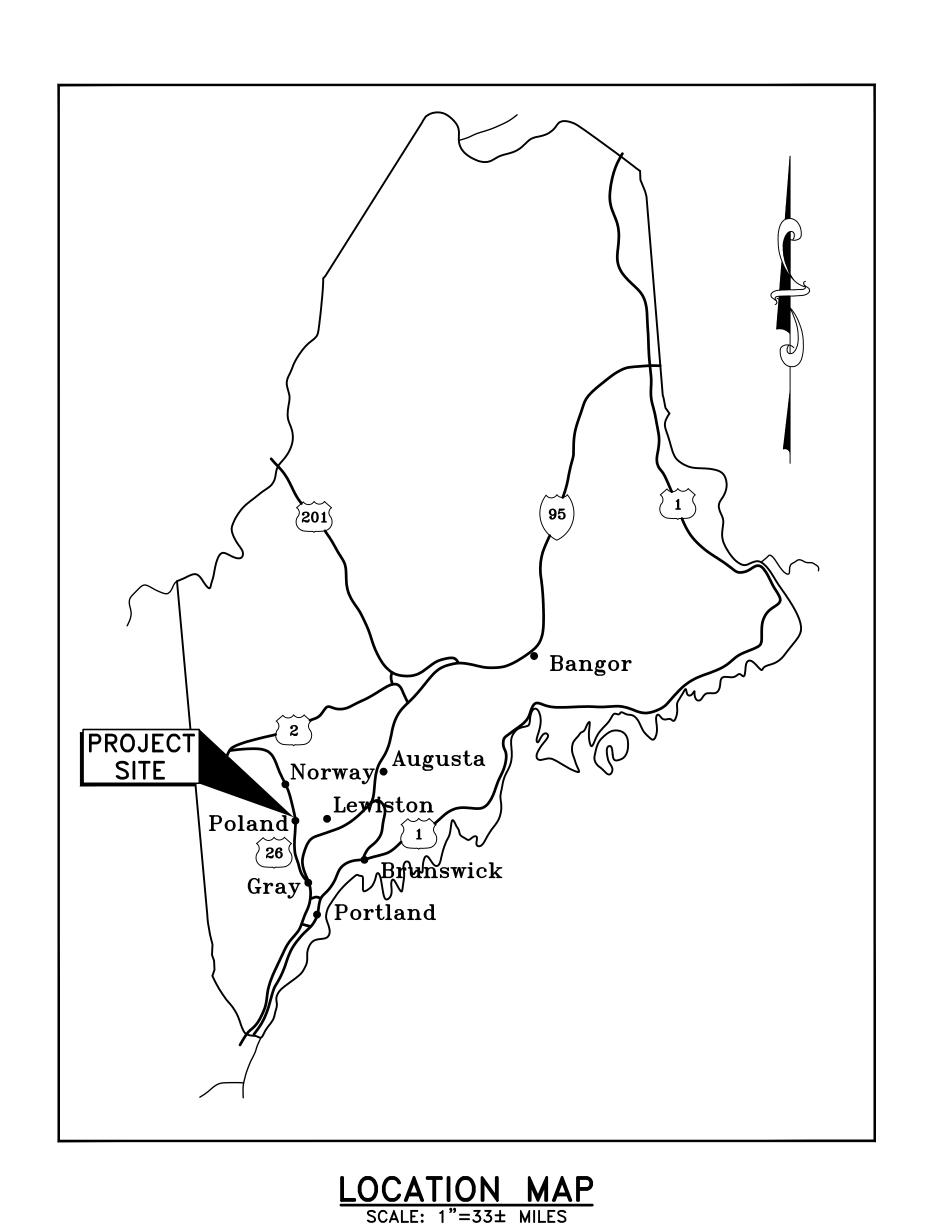
				AGENT INFORMATION (If Applying on Behalf of Owner)					
APPLICAN	TINFORMATION (OW	ner)		AGENT INFORMA	HON (If	Applying on Ber	nalf of Owner)		
Name:			Na	me:					
Mailing Address:			Ма	iling Address:					
Town:			То	wn:					
State and Zip Code:			Sta	ate and Zip Code:					
Daytime Phone #:			Da	ytime Phone #:					
Email Address:			Em	nail Address:					
		PRO	JECT INF	ORMATION					
Part of a larger	Yes After the Fact	? 🛛 Yes	Project i	nvolves work below	☐ Ye	Name of			
project? (check one):	□ No (check one):	□ No	mean lo	w water? (check one)	: 🛚 No	waterbody:			
Project Town:		Project (Addres	Location s):			Map & Lot Number:			
Brief Project									
Description:									
Brief Directions to Site:									
PERMIT BY RULE (PBF									
requirements for Permit I			hapter 30	5. I and my agents,	if any, <u>k</u>	<u>ave read</u> and wi	II comply with all		
of the standards in the	Sections checked be	low.							
Sec. (2) Act. Adj. to Pr	otected Natural Res.	☐ Sec.(1	0) Stream	Crossing	☐ Se	c. (17) Transfers/P	ermit Extension		
Sec. (3) Intake Pipes		☐ Sec. (11) State T	ransportation Facil.	☐ Se	c. (18) Maintenand	e Dredging		
Sec. (4) Replacement	of Structures	☐ Sec. (12) Restor	ation of Natural Areas	☐ Se	c. (19) Activities in	/on/over		
Sec. (5) REPEALED		☐ Sec. (13) F&W C	reation/Enhance/Water	r	significant vernal p	oool habitat		
Sec. (6) Movement of	Rocks or Vegetation	Qua	lity Improv	nprovement Sec. (20) Activities located in/on/ove					
Sec. (7) Outfall Pipes	J		14) REPE <i>A</i>						
Sec. (8) Shoreline state	hilization		•	ublic Boat Ramps waterfowl & wading bird habitat or					
Sec. (9) Utility Crossin			•	I Sand Dune Projects	_				
()									
NOTE: Municipal permit may be required for str Project Office for more in	eam crossings and fo								
	CATION FORMS CAN	NOT BE AC	CEPTED	WITHOUT THE NEC	CESSAR	Y ATTACHMENT	S		
☐ Attach all require									
	utlined in Chapter		` ,						
☐ Attach a check fo									
	can be found at the			-					
☐ <u>Attach</u> a location									
☐ Attach Proof of Le									
	's registration info						opy or		
	ge=x) Individuals a						identity		
I authorize staff of the									
	site for the purpose of					ic, and manne	10300100310		
I also understand that			•			Anartment unle	ee tha		
	res or denies the PB			days after receipt	Dy tile L	cpartificiti <i>arii</i> c	33 1110		
By signing this Notifica				ets all annlicability i	requiren	ents and standa	ards in the rule and		
that the applicant has s									
Signature of Agent or	l		o p. op	_	ate:	piaco:			
Applicant:					ato.				
Keep a copy as a record Environmental Protection									
of the DEP's receipt of ne									
years. Work carried ou									
AUGUSTA DEP		AND DEP	,	BANGOR DEP		PRESQUE ISLE D	EP		
17 STATE HOUSE ST		NCO ROAD	02	106 HOGAN ROAD	1	1235 CENTRAL DE			
AUGUSTA, ME 04333 (207)287-7688		.AND, ME 041 22-6300	us	BANGOR, ME 0440 ⁻ (207)941-4570	ı	PRESQUE ISLE, N (207)764-0477	VIE U4/09		
OFFICE USE ONLY	Ck.#	1		Staff	Sta				
		4_					1		
PBR #	FP	Date		Acc. Date	De ^s		After Photos		
	ì				, – u	-			

CONSTRUCTION DRAWINGS

POLAND CORNER ROAD WATER MAIN EXTENSION

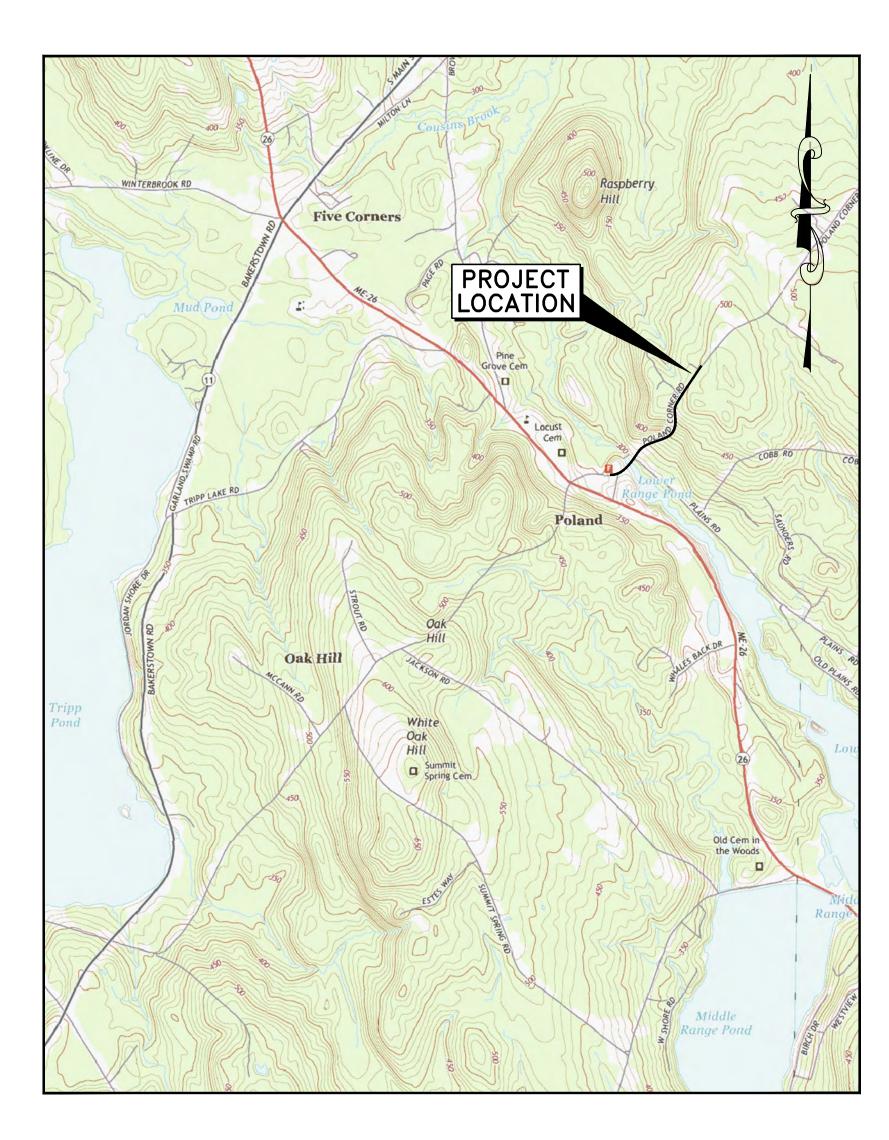
POLAND, MAINE

PINE TREE ENGINEERING, INC. 53 Front Street Bath, Maine 04530

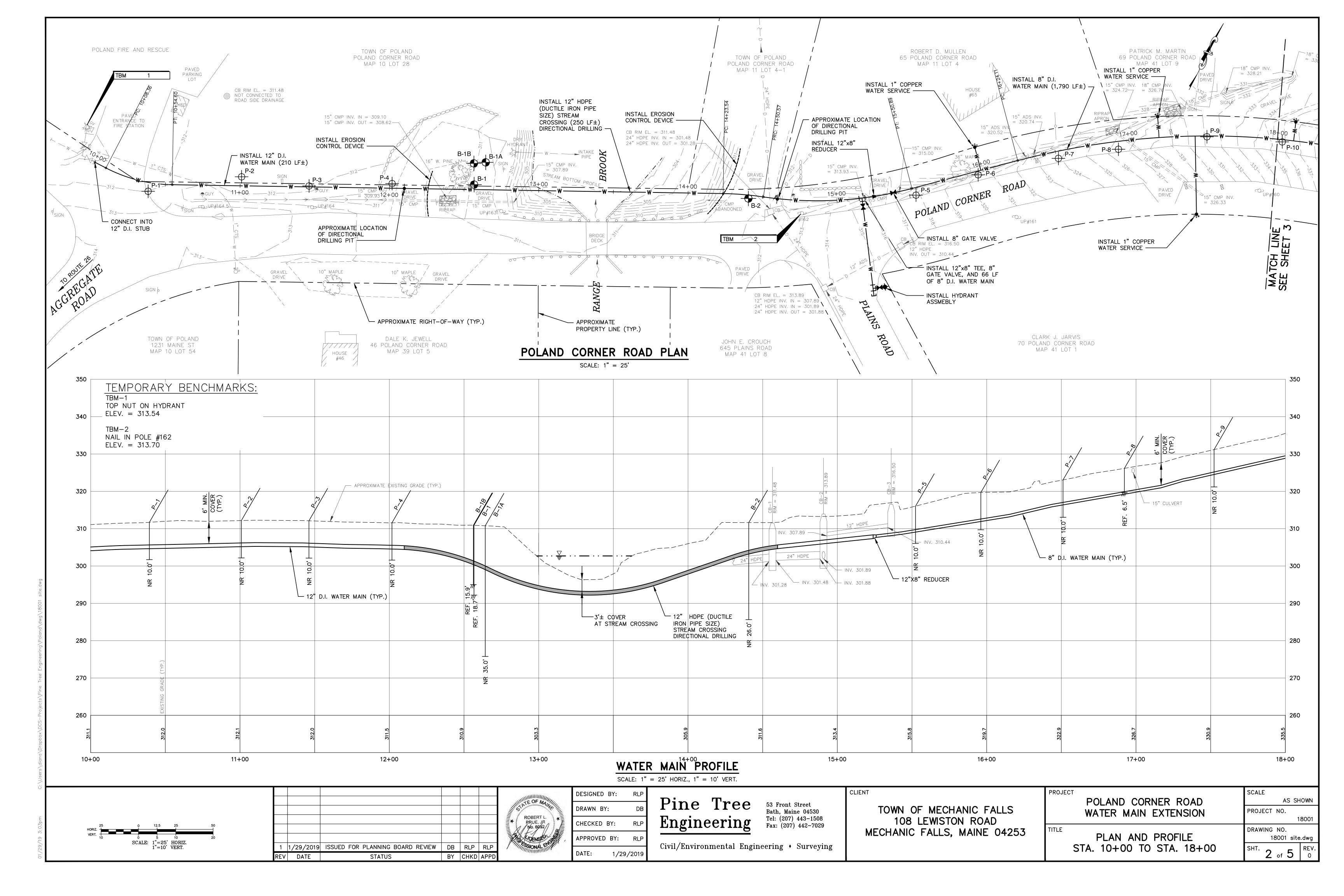


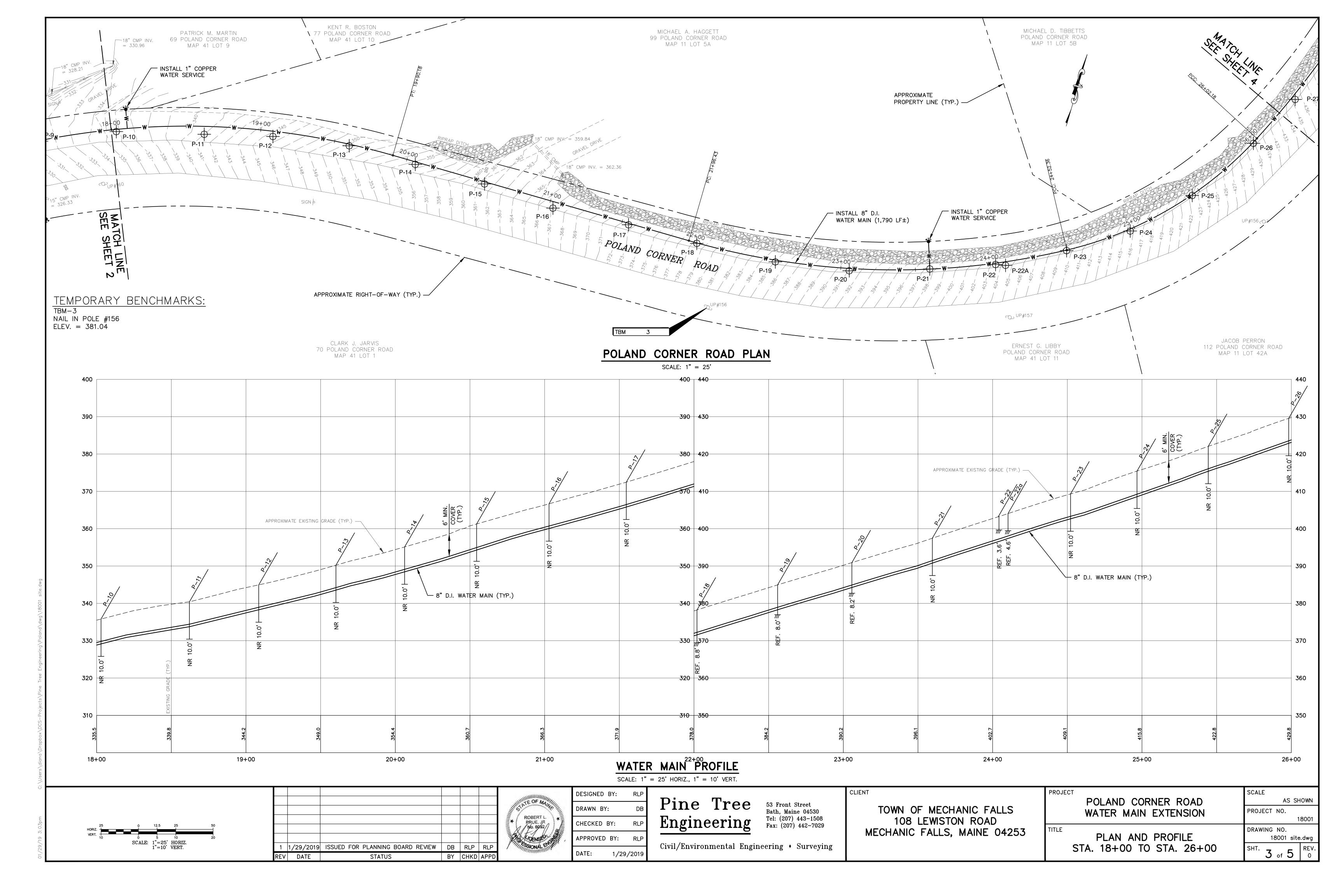
DRAWING INDEX

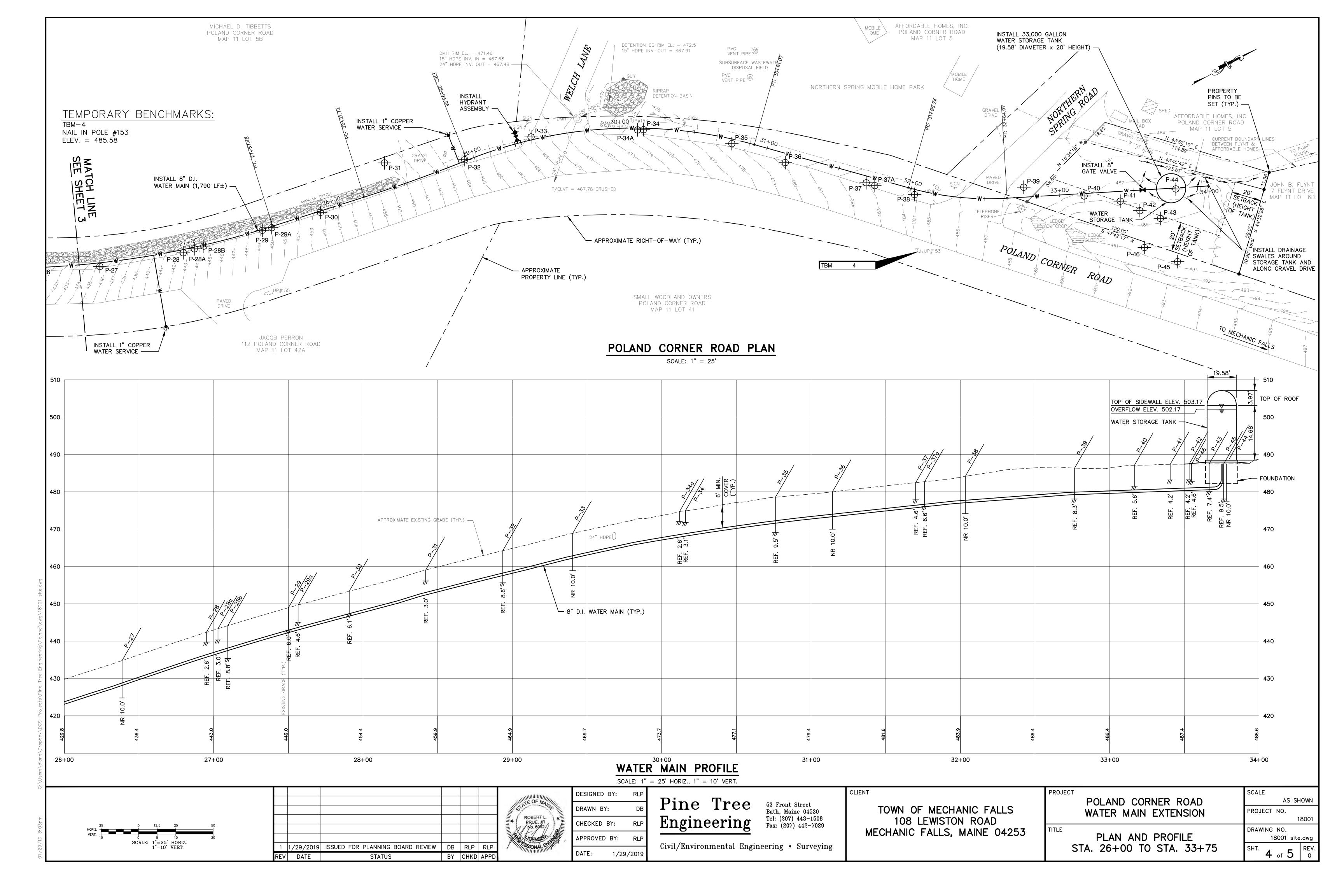
- 1 COVER
- 2 PLAN AND PRFOLE
- 3 PLAN AND PROFILE
- 4 PLAN AND PROFILE
- 5 DETAILS

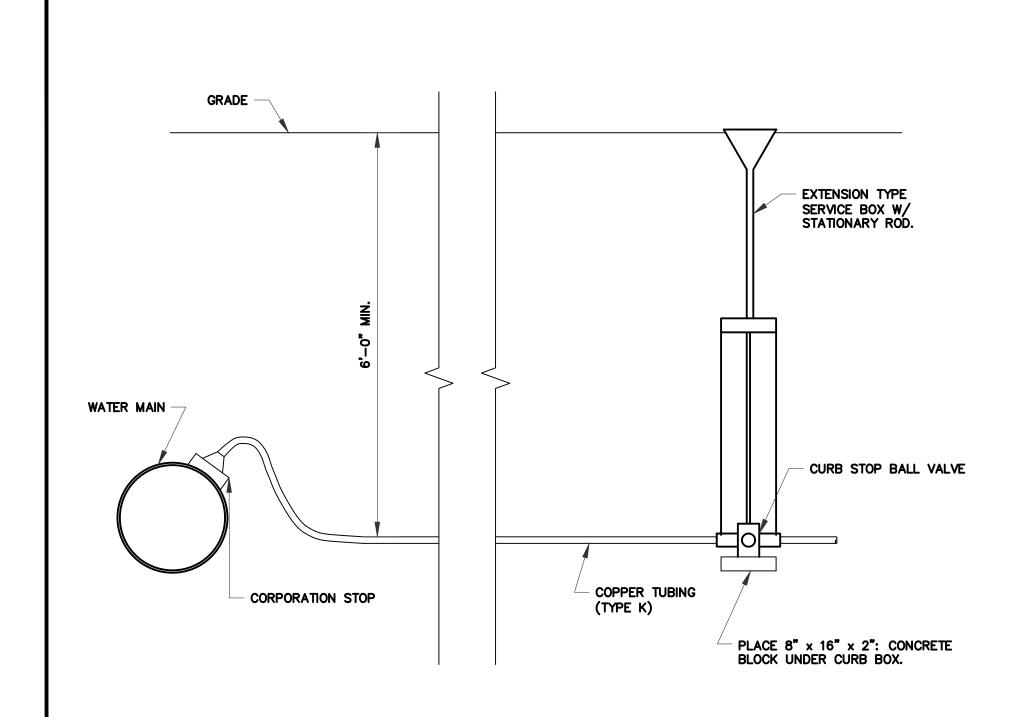


AREA MAP
SCALE: 1"=2000±



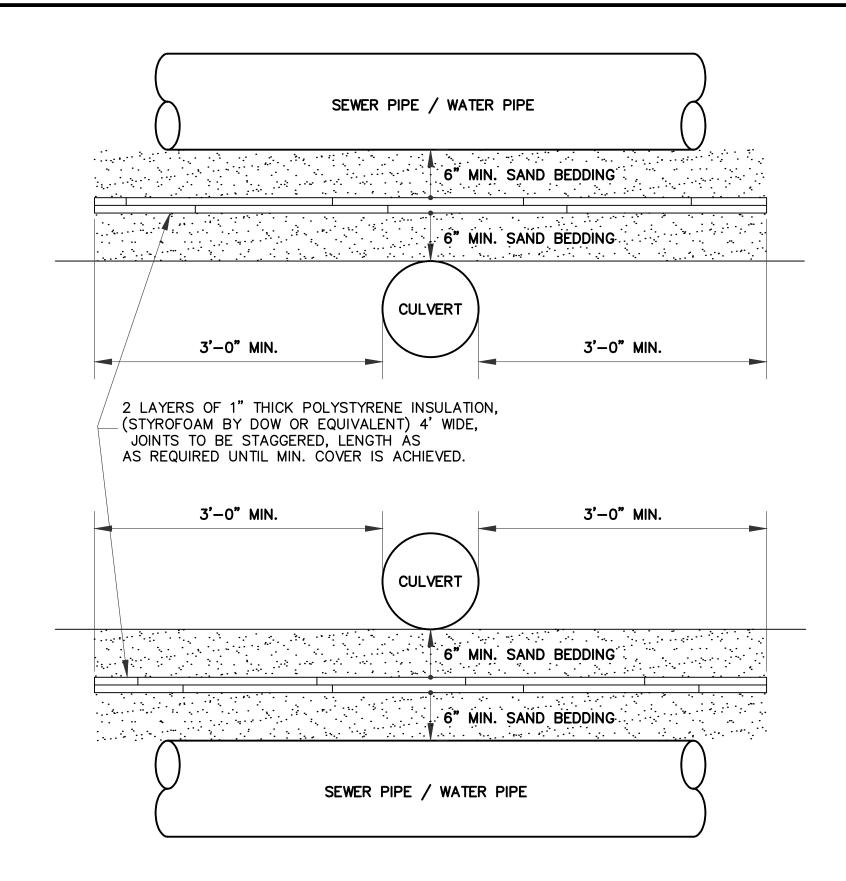


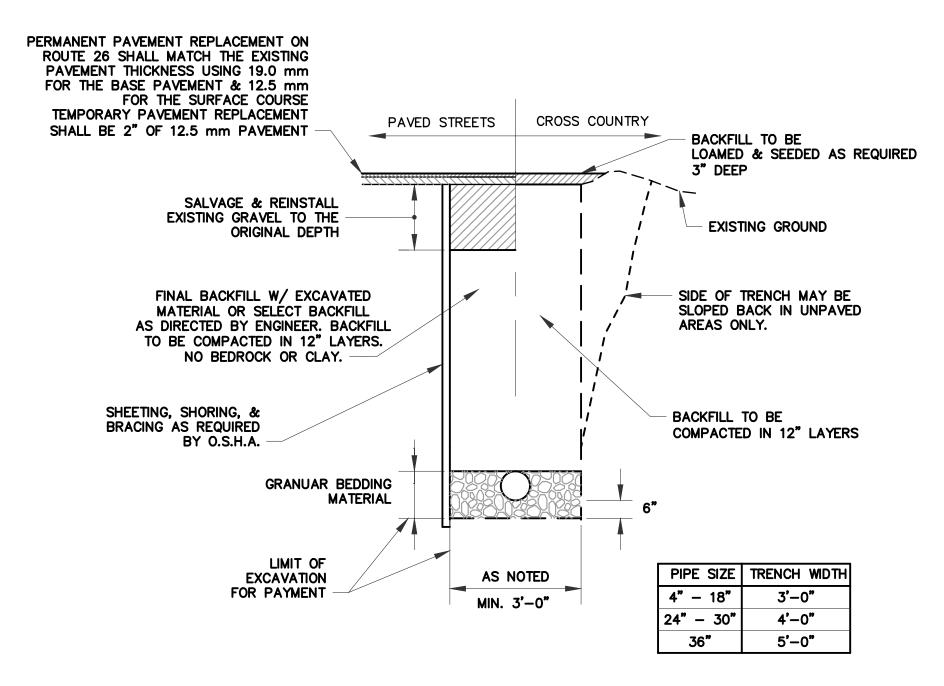




TYPICAL SERVICE CONNECTION

N.T.S

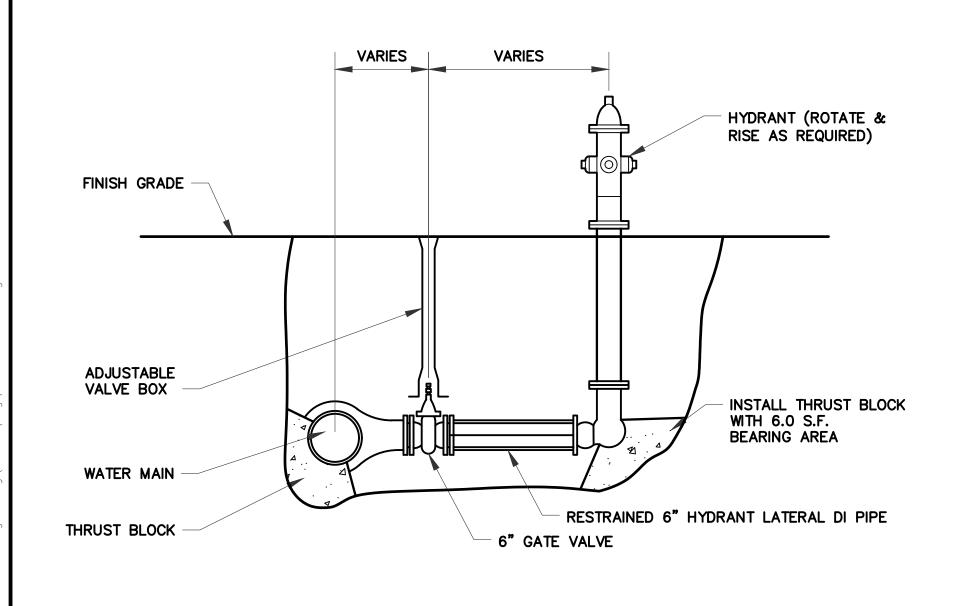


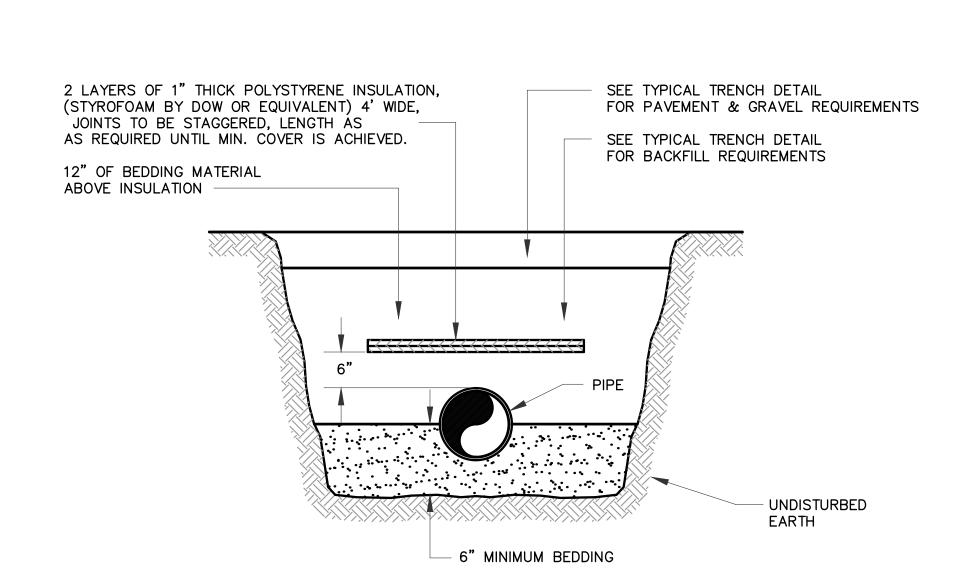


DUCTILE IRON PIPE TYPICAL TRENCH DETAIL N.T.S.

SEWER CULVERT CROSSING DETAIL

SCALE: 1" = 1'-0"





19'-7" TOP OF ROOF ELEV. 507.14 TOP OF SIDEWALL ELEV. 503.17 OVERFLOW ELEV. 502.17 12" FREEBOARD — PROPOSED GRADE ---**EXISTING** GROUND — _______ FOUNDATION REVEAL FOUNDATION -TANK DETAIL N.T.S.

TYPICAL HYDRANT ASSEMBLY DETAIL

SCALE: 1/2" = 1'-0"

TRENCH PIPE INSULATION DETAIL SCALE: 3/4" = 1'-0"

1 |1/29/2019 ISSUED FOR PLANNING BOARD REVIEW | DB | RLP | RLP REV DATE BY CHKD APPE STATUS

i.	HILL	ATEC	OF M	AINI	1/1.
WHITHIN	5	ROB PRI	ERT I JE, JF		11111111
		KIO	6092		
1	Vill	ESSIC	NAL	THINK	21

	DESIGNED BY:	RLP
	DRAWN BY:	DB
HIIIIII	CHECKED BY:	RLP
,	APPROVED BY:	RLP
	DATE: 1/2	29/2019

Pine Tree 53 Front Street Bath, Maine 04530

Tel: (207) 443-1508 Fax: (207) 442-7029 Civil/Environmental Engineering • Surveying

CLIENT

TOWN OF MECHANIC FALLS 108 LEWISTON ROAD MECHANIC FALLS, MAINE 04253

PROJECT	POLAND CORNER ROAD	SCALE AS SHOWN
	WATER MAIN EXTENSION	PROJECT NO. 18001
TITLE DETAILS		DRAWING NO. 18001 DETAILS.dwg
DE IAILS	SHT. 5 REV.	



February 1, 2019

Mr. Scott Neal Town of Poland 1231 Maine Street Poland, Maine 04274

Re: Application for Site Plan Review

MB Bark, LLC

100 Bark Mulch Drive, Auburn, Maine St.Germain Collins File No.: 2265-0002

Dear Mr. Neal:

MB Bark, LLC (MB Bark) is proposing to develop additional material handling areas at its facility in Poland, Maine with a street address of 100 Bark Mulch Drive, Auburn, Maine. The development will include the expansion of one gravel pad area and the construction of two additional gravel pads and two internal access drives connecting the new gravel pads. These pads will be used for the processing and storage of recyclable materials as allowed under the applicant's solid waste processing license from the Maine Department of Environmental Protection (MEDEP). No buildings are proposed in this application.

MB Bark has received approval from the MEDEP for the new handling areas, and to accept and recycle asphalt (roofing) shingles in accordance with its solid waste processing license.

Enclosed please find 10 copies of the completed application form and supporting documents. Checks are enclosed for the review and notification fees (\$162) and review escrow (\$1,000).

If you should have any questions, please feel free to call me at (207) 591-7000 ext. 41 or email me at peterm@stgermaincollins.com.

Sincerely,

ST.GERMAIN COLLINS

Peter Mohlin Project Manager

enclosure

cc: Jim Hiltner, MB Bark, LLC

Bill Crawford, MB Bark, LLC



APPLICATION FOR SITE PLAN REVIEW TOWN OF POLAND PLANNING BOARD

MB Bark, LLC 100 Bark Mulch Drive Auburn, Maine

Submitted on behalf of:

MB Bark, LLC 100 Bark Mulch Drive Auburn, Maine 04210

Prepared by:

St.Germain Collins 846 Main Street Westbrook, Maine 04092

January 2019

St.Germain Collins File No.: 2265-0002



Town of Poland, Maine Planning Board

Formal Site Plan Review

Instructions:

- 1. Read every part of this document. Failure to follow requirements can and will delay the Planning Board's decisions.
- 2. Fill out the forms on pages 1 through 6. Obtain or get copies of information as required by the application on these pages.
- 3. Use the "Submission Checklist" on pages 5 and 6 to make sure submission requirements are met.
 - a. The checklist is a summary of the standard requirements in Section 509.8 of the Comprehensive Land Use Code.
 - i. The actual Code wording may be found on-line at www.polandtownoffice.org. Go to the "Code Enforcement" page, select "Comprehensive Land Use Code" at that bottom of the page. Hardcopies are available for purchase at the town office.
 - b. Make sure all waiver requests have a written statement for each request. Check with the Code Enforcement Office to make sure items stated as "On File" are indeed in the town office.
 - c. Some requirements may need only a one paragraph or one sentence statement. Make sure all requests are answered.
- 4. NUMBER OF COPIES OF THE APPLICATION AND DUE DATE
 - a. A total of at least ten (10) copies of the plans and one PDF copy (on either cd or usb) are needed. Be sure to make a copy for yourself.
 - b. The Code Enforcement Office must receive the original application, an additional 9 copies, and a digital PDF copy (either cd or usb) with appropriate fees by 1:00 p.m. eleven (11) days before the stated meeting to be put on the upcoming agenda.
 - c. If review for missing information by the Code Enforcement Officer is desired, a copy must be submitted to the CEO at least 14 days prior to the meeting.
 - d. The application must be on file for public review for at least 10 days prior to the meeting. Applications received after the Agenda is posted may not be reviewed by the Board for your scheduled meeting date.
- 5. Check with this office to make sure that all departments have responded to your application prior to the meeting.

PROJECT NAME:			
Date of Planning Board Review:			Application #
LOT INFORMATION:			
Tax Assessor's Map #		Lot #	Sub lot #
Watershed:			
Property's Road Location:			
Lot Size:	Acres or Sq. Ft.		Road Frontage:Ft. In, give best estimate with "est." after date)
Year lot created:		(If unknow	n, give best estimate with "est." after date)
Zoning District(s):	F1000 Z	Zone:	Aquifer Overlay:
Current use of lot:			
LAND OWNER(s):			
Name(s)			
Company			
Mail Address:			Main Phone
Iviali Audiess.			ividili Filolic
Town/State/Zip			Alternate Phone:

APPL	ICANT or Co	ONTACT PERSON:		
Applic If land permi inforn	ant is: lowner, writ ssion to co nation:	LandownerContractorRenterBuy e "Same" below and continue to next block below. If not the landowner, sulpstruct on or use the land, or copy of a contract to buy from the landowner,	bmit a letter of	ollowing
Comp	any			
Mail A	ddress:	Main Phone:	-	-
Town/	State/Zip	Alternate Phone:		
THIS A	APPLICATI(ON IS FOR: (Check all that apply)		
_	mmercial	New Development		
	dustrial	Change In Use		
	stitutional	Expansion of Use		
	vernmental	Expansion of Structure(s)		
Op	en Space	Resumption of Use		
(This µ	page is to de <mark>eneral</mark>	ONDITIONS: scribe what is on your lot currently) have any development? (If No, go to "Proposed Development")	Yes	
_	No			
a.		n existing Well No	Yes	
b.		n existing Septic System No	Yes	
C.		s, submit a copy of a septic permit, or drawing(s) showing size & location. n existing Road Entry	Yes	
		No No		
		s, will there be any changes/modifications? No	Yes	
		o, submit copy of appropriate road entry application if entrance is onto a state or to		
d.	,	tures to be removed No	Yes	
	i) If ve:	s, submit information about the structure to be removed and how any debris will b	e disposed of.	
2. E :		Development & Improvements NOT Including Buildings	o allopooda oli	
		· · · · · · · · · · · · · · · · · · ·	0	Ca Et
a.		WIID		Sq. Ft.
	or Acres			C F1
b.		elds (landscaped areas + stormwater mgmt. water surface)		Sq. Ft.
	or Acres			_
C.	Size of dr	iveways/roads (gravel, concrete, pavement)		Sq. Ft.
d.	Size of ot	her non-vegetated areas		Sq. Ft.
e.		already filled		Sq. Ft.
		n <u>Structure</u> (Bark Mulch Production Bldg.)		
a.				Sq. Ft.
b.		ss Floor Space (exterior dimensions of all floors)		Sq. Ft. Sq. Ft.
υ. C		ntage Sethack	-	Sq. r t. Ft

	d.	Side Setback			-		Ft.	
	e.	Rear Setback					Ft.	
	f.	Distance to Great Pond		_Not applicable (over 250')	_		Ft.	
	g.	Distance to Stream		Not applicable (over 250')			Ft.	
	h.	Distance to Wetlands		Not applicable (over 250')	_		Ft.	
4.	<u>Fοι</u>	undation Type	Full Basement	Frost Walls	Sla	b	_Piers	
5.	<u>Exi</u>	sting Accessory Structure(s) (E-Waste Bldg., Maint	enance Bldg. Compost				
	a.	Total Number of Structures	Bldgs., Main Office Bu	- ·	_	_		
	b.	Total Ground Footprint		,	_		Sq.	
	C.	Total Floor Space			_		Sq.	Ft.
	d.	Closest Road Setback			-		Ft.	
	e.	Closest Side Setback			-		Ft.	
	f.	Closest Rear Setback			-		Ft.	
	g.	Distance to Great Pond		_Not applicable (over 250')			Ft.	
	h.	Distance to Streams		Not applicable (over 250')			Ft.	
	l. — .	Distance to Wetlands		_Not applicable (over 250')			Ft.	
6.		al Existing Impervious Surfa	<u>ices</u>			2,409,303.6	Sq.	۲t.
	a.	Add 2c +2d + 3a + 5b						
	<u> </u>	CED DEVEL ODMENT						
		SED DEVELOPMENT:					•	
1.		tlands to be impacted	(a)				Sq.	۲t.
2.		w footprint(s) and developed a Changes in building footprint(Sa	C+
		Changes in driveway/roadway					Sq. Sq.	
		Changes in patios, walkways,					Sq.	
	d.	TOTAL (2a+2b+2c)					Sq.	
3.		centage of lot covered by impe					%'	
	a.	(Equals [areas on line 6 page	2 + line 2d above] / [To	tal lot area measured in sq.	ft.] * 100%)			

SUBMISSIONS:

- 1. Attach drawings and/or statements describing the following items, if applicable:
 - a. Provide a copy of deed and Tax Assessor's information card.
 - b. Provide a map of the general area showing land features within at least ½ mile of this lot.
 - c. Provide site plan(s) of your lot with <u>existing</u> development and its dimensions shown.
 - d. Provide site plan(s) of your lot with proposed development and its dimensions shown.
 - . (May be combined on existing development drawing.)
 - e. Provide detailed plans of proposed structural development and changes.
 - f. Provide statements or drawings of methods of infrastructure:
 - i. Water supply
 - ii. Sewage disposal
 - iii. Fire protection
 - iv. Electricity
 - v. Solid waste disposal
 - g. Type, size, and location of signs.
 - h. Number of parking spaces.
 - Provide phosphorus loading calculation if in a great pond watershed area.
 - j. Anticipated date for start of construction.
 - k. Anticipated date for completion of construction.
 - I. Standard submissions requirements shall follow Section 509.8 of the Comprehensive Land Use Code. Copies of the Code are available for viewing at the Town Office and Library. Copies are available for purchase (\$25.00) in the Code Enforcement Office.
 - i. (Use checklist starting on page 6 for summary of usual requirements.)
 - m. Other requirements unique to your project added by the Planning Board.

2. List all state and federal approvals, permits, and licenses required, if any, for the project:

This includes but is not limited to the following:

- 1. State highway entrance permit.
- 2. Soil disturbances involving more than one acre.
- 3. Impact on more than 4,300 square feet of any type wetland.
- 4. Soil disturbances within 100 feet of lakes, rivers or streams.
- 5. Activity within 75 feet, over the water, or in the water of lakes, rivers, or streams.
- Timber harvesting.
- 7. Flood zones.
- 8. Discharges and emissions

DISCLOSURE: (READ BEFORE SIGNING)

- I hereby acknowledge that I have read this application and pertinent sections of the ordinances, and state that the
 information in this document is to the best of my knowledge true and accurate. I agree to comply with all the Town of
 Poland's ordinances and the State of Maine's statutes regulating the activities sought in this application as well as any
 permit(s) approved for this application.
- 2. I understand that all construction of structures shall conform to or exceed the minimum requirements of the Maine Uniform Building and Energy Code, and the NFPA-101 Life Safety Code, 2009
- 3. I understand that any approval is valid for only the use(s) as specified in this application. The permitting authority must approve any change(s) made to the use(s) sought in the application. Any approval issued for this application is approved on the basis of truthful information provided by the applicant(s), and as allowed by the ordinances of the town.
- 4. I understand that it is my responsibility to assure that the lot description herein accurately describes its ownership, its boundary lines, and the setback measurements from the legal boundary lines.
- 5. I understand that I have the burden of proof as to the legal right to use the property, and that approval of this application in no way relieves me of this burden. Any approval issued does not constitute a resolution in favor of me or the landowner in any matters regarding the property boundaries, ownership, or similar titles.
- 6. I understand that all necessary **Building and Use Permits** shall be secured from the Code Enforcement Office after the Planning Board grants approval of this application.
- 7. I understand that a Certificate of Occupancy or Compliance shall be required prior to the start of any use or occupancy associated with this application unless a signed written waiver is issued with the permit. Fines and penalties may be issued if use or occupancy is started prior to the issuance of the certificate.
- 8. I understand that the approval becomes invalid if construction or use has not commenced within twelve (12) months of the Planning Board's approval date, construction is suspended for more than six (6) months and no notice for just cause is submitted prior to the end of the six (6) months, or it is found that false statements have been furnished in this application.
- 9. I understand that if I fail to comply with the aforementioned statements, a "STOP WORK" order may be issued for which I will immediately halt any construction and/or use(s) that are approved for this application. This failure may also require that I return the property to its natural state or as closely thereto before the use(s) was/were approved.
- 10. I understand that failure to follow these requirements will lead to Violation Notices and Citations that have fines and penalties. This in turn can lead to civil proceedings in District and/or Superior Court.

11. I understand that all state and federal permits are my responsibility as the applicant and/or owner and will secure the same prior to the start of the project.

Applicant's Signature(s)

Doto

Submission CHECKLIST

The <u>following list is a short summary</u> of the information required in Chapter 509.8 of the Comprehensive Land Use Code for the Town of Poland, Maine. Please checkmark or place an "X" in the left-hand columns if the information has been provided, if you request a waiver from submitting the information, or you believe the information is not applicable to your application. If a waiver(s) is requested, or the information is not applicable, a written explanation is required. Columns on the right are for the Planning Board's use.

For	Applicant l	Jse		Fo	r Planr	ning Board	d Use
Provided	vided Waiver Not Request Applicable		Section 509.8.A Submission requirements	Received	On File	Waived	Not Applicable
			1. Site Plan Drawings				
			2. Signed copy of application				
			3.a. Name & address of owner				
			Name of development				
			Name & address of abutters within 500' of lot for development				
			Map of general location				
			Show all contiguous properties				
			Names, Map, & lot #'s on drawings				
			Copy of deeds, agreements				
			Engineer/ designer of plans				
			Existing Conditions (Site Plan)				
			Zoning Districts on and/or abutting project's lot shown				
			Bearings & Distances shown on drawings				
			Location of utilities, culverts, drains				
			Location, name of existing r/w				
			Location, dimensions of existing structures				
			Location, dimensions of existing roads, walks, parking, loading, etc.				
			Location of intersection within 200'				
			Location of open drains, wetlands, wildlife areas, historic sites, etc.				
			Direction of surface drainage				
			100-yr. Floodplain				
			Signs				
			Easement, covenants, restrictions				
			Proposed Development (Site Plan)				
			Location & dimensions of all new structures. New development delineated from existing development				
			Setback dimensions shown & met				
			Exterior lighting (Will meet full cutoff requirements)				
			Incineration devices				
			Noise of machinery and operations				
			Type of odors generated				
			Septic system and other soils reports				
			Water supply				
			Raw & finished materials stored outside				
			Contours shown at PB specified intervals				
			Curbs, sidewalks, drives, fences, retaining walls, parking, etc.				
			Landscaping plan				
			Easements, r/w, legal restrictions				
			Abutters' property lines, names				
			TRAFFIC DATA				

For <i>F</i>	Applicant L	Jse		For Planning Board Use		d Use	
Provided	Waiver		Section 509.8.A Submission requirements	Received	On	Waived	Not
	Request	Applicable			File		Applicable
			Peak hour traffic				
			Traffic counts				
			Traffic accident data				
			Road capacities				
			Traffic signs, signals				
			STORMWATER & EROSION				
			Method for handling stormwater shown				
			Flow direction				
			Catch basins, dry wells, ditches, etc.				
			Engineering Analysis of stormwater				
			Erosion control measures				
			Hydrologist groundwater impact				
			Utility plans for all utilities				
			Cross-section profile of roads, walks				
			Construction drawings of roads, utilities				
			Cost analysis of project and financial capability demonstrated				
			Phosphorus control plan if in watershed of a great pond				
			Submission of waiver requests				
			Copies of state, federal applications, permits, &/or licenses required for this project.				
			Condition A.				
			Condition B.				
			Condition C.				
			Condition D.				
			Condition E.				
					-		

This application was first looked at by the Planning Board on of the review process.	/ / but does not create vested rights in the initiation
By vote of the Board this application requires an on-site inspection If yes, an onsite inspection is scheduled for/	on:YesNo /atAMPM
By vote of the Board this application requires a public hearing: If yes, public hearing is scheduled for/	YesNo
Conditions of Approval for Formal Site Review:	
Planning Board Chair	

Site Review and Shoreland Zoning Review Fees:

Type of fee	<u>Fee</u>	<u>Units or Comments</u>
Application – sketch plans, Rough design	\$75.00	Each application (no other fees)
Application – formal ³	\$150.00	Each application + fees below
Approval extension, Planning Board Approval only	\$50.00	One extension only (no other fees)
Escrow 4, minimum amount	\$700.00	When required by Planning Board
Extension of approval	\$100.00	Before approval expires
Auto graveyards, recycle business	\$5.00	Per vehicle storage slot (parking space)
Junkyard, Storage Lots	\$1.50	Per ft ² of outside storage
Residential Towers	\$20.00 + \$5.00	Based on Cost of Work
	per \$1,000.00	
Commercial Towers	\$20.00 + \$10.00	Based on Cost of Work
	per \$1,000.00	
Notifications	\$.75	Each Notification, First Class Mail sent by Town

- 1. Building and Structures may include up to five times the footprint area of the building for grounds improvements, exclusive of the building footprint, as part of the building review fee.
- 2. <u>Building and Grounds Improvement Fees</u>. The sum of these two fees may be limited to \$2,500.00 per application at the discretion of the Planning Board. (Junkyards, auto graveyards, recycling business, and towers excluded.
- 3. Reduced Fees: The Planning Board may, upon application therefore, allow a reduced total site review fees to \$50.00 in any case which it determines that the work for which the permit is sought will be performed within the Shoreland Zone. The project shall be intended solely for the purpose of protecting a Great Pond, Stream, River, or other Natural Resources through the implementation of Conservation, Best Management Practices, or other environmental safeguards. Also, the project shall not result in the enlargement of any building or structure or an intensification of the existing use of the property.
- 4. Review Escrow Funds may be used by the Town to pay for professional reviews an advice requested by the Planning Board or Code Enforcement Officer related to the applicant's proposed development. Review escrow funds deposited by the applicant not spent during the course of the Town's review shall be returned to the developer within sixty days after the Planning Board's decision on the application is final. If Professional review and advice fees exceed the amount deposited, the developer shall pay the amount outstanding before final approval or any permit is granted.

	Per vehicle storage slot (parking space)
J	Per ft ² of outside storage
	Based on Cost of Work
	Based on Cost of Work
	Each Notification, First Class Mail sent by Town
_	
-	

Town of Poland, Maine PLANNING BOARD AGENDA REQUEST

Date of meeting you are requesting to be scheduled for: 02 / 1 2 / 1 9 Meetings are normally conducted from 7:00 to 10:00 PM in the Municipal Conference Room at the Town Office Map 004 Lot 015 & 017 Sub-lot 002
Applicant's Name: MB Bark, LLC Mailing Address: 100 Bark Mulch Drive Town, State, Zip: Auburn, Maine
Home Phone: Hours: Work Phone: 207-786-0600 Hours:
Type of application: Sketch Plan X Site Review Shoreland Subdivision Informational Road location for project: 100 Bark Mulch Drive, Auburn Maine 04210
Zoning: General Purpose III Lake Watershed:Nature of business to be discussed (Brief description): Site Plan Review for improvements at recyclable product processing facility. IMPORTANT - READ CAREFULLY:
This Office must receive the original application, plus nine (9) copies, a digital PDF copy (on either cd or usb), and appropriate fees by Friday at 1:00 p.m., eleven (11) days before the stated meeting to be put on the upcoming agenda.
 New business is scheduled on the agenda in the order this office receives this form. If you want your application reviewed for contents prior to the meeting, it must be in this office 14 days before the meeting. Should the Board choose to adjourn before all business is addressed, all remaining business will be tabled until the next available meeting. Unfinished business is conducted before new business is addressed.
Applicant's Signature: Date: 02 0(20/9
OFFICE USE ONLY: Request Taken By: Date: / Time::a.m. p.m.

TABLE OF CONTENTS

COVER LETTER

FORMAL SITE PLAN REVIEW APPLICATION FORM

Attachment 1 – Project Description

- Introduction
- Noise
- Soils
- Odors
- Septic
- Water
- Fire
- Electricity
- Solid Waste
- Outside Storage
- Landscaping
- Lighting
- Signs
- Schedule
- Parking

Attachment 2 - Title, Right, and Interest

- Deeds
- Tax Assessor's Information Cards
- Letter from Landowner
- Lease Agreement

Attachment 3 - Abutter Information

- List of Abutters
- Abutter Map

Attachment 4 - Soils

- Geotechnical Evaluation Pad #1
- Pad #1 Construction Recommendations

Attachment 5 - Financial Capacity

• Letter from Bank of America Merrill Lynch

Attachment 6 - Traffic

- Maine Department of Transportation Traffic Counts
- Maine Department of Transportation Crash Summary

Attachment 7 - Stormwater and Erosion and Sediment Control

• Stormwater Management Plan

Attachment 8 - Plan Set

Attachment 9 - Copies of Required State and Federal Licenses and Permits

- Maine Department of Environmental Protection Solid Waste Processing Facility License Amendment #S-021741-WK-I-A (November 28, 2018) Beneficial Use of Solid Waste #S-021741-W4-J-N (November 28, 2018)
- Maine Department of Environmental Protection
 Wetland Alteration
 L-19467-TG-C-N and L-19467-L6-D-N (June 19, 2018)
- Army Corps of Engineers General Permit, Category 2 NAE – 2018 – 00135 (June 19, 2018)

Application for Site Plan Review Town of Poland Planning Board 100 Bark Mulch Drive, Auburn, Maine 04240 St.Germain Collins File No.: 2265-0002 January 2019 Attachment 1

Page 1

Project Description

Introduction

St.Germain Collins, on behalf of MB Bark, LLC (MB Bark) is submitting a Formal Site Plan Review Application to the Town of Poland for site improvements at their existing recycled products handling facility located at 100 Bark Mulch Drive in Poland and Auburn Maine. Proposed site improvements include the construction of two additional recycled products processing and storage areas (Pad 2 and Pad 3) in the undeveloped area of the property to the west, and extending the existing recycled products handling and storage area (Pad 1) to the north. Pads 2 and 3 will be used for asphalt shingle processing and handling operations currently conducted at CPRC Group LLC's (CPRC's) solid waste processing facility located in Scarborough, Maine, and the Pad 1 extension will provide additional area for MB Bark's existing material handling and storage activities. All site improvements will occur in Poland. The proposed site features are shown on plan C-3.0 – Overall Site Plan, provided in Attachment 8.

Noise

Based upon knowledge of existing operations in Scarborough, noise from the asphalt shingle processing activities are not expected to exceed the ordinance limits.

As a condition of the Maine Department of Environmental Protection (MEDEP) Solid Waste Processing Facility license amendment, a qualified noise consultant will complete a noise survey at the facility before MB Bark begins operating in the new areas. The survey will include an assessment of MB Bark's non-exempt noise sources to estimate the maximum daytime and nighttime noise levels at nearby protected locations. The survey will be provided to the Town of Poland.

Soils

Two geotechnical reports were prepared by Summit Geoengineering Services and are included in Attachment 4. The reports indicate that soil conditions are suitable to support the Pad 1 extension. A condition of the MEDEP Solid Waste Processing Facility license amendment requires additional subsurface investigation in the areas of the proposed road, Pad 2 and Pad 3. This will be provided to the Town of Poland.

Odors

Nuisance odors from facility operations are not anticipated as none exist at the Scarborough facility and no new putrescible wastes will be handled on site.

Septic

The MB Bark facility is currently served by the City of Auburn Water District and several existing septic systems (see 1997 Approved Site Plan). The proposed changes do not include any new structures or plumbing.

Application for Site Plan Review Town of Poland Planning Board 100 Bark Mulch Drive, Auburn, Maine 04240 St.Germain Collins File No.: 2265-0002 January 2019 Attachment 1

Page 2

Water

The MB Bark facility is currently served by the City of Auburn Water District. The proposed changes do not include any new water uses or connections.

Fire

Fire protection at the facility is primarily provided by the City of Auburn Fire Department with its Central Station located approximately 3.5 miles away at 550 Minot Avenue. Facility fire protection equipment consists of a dry sprinkler system located in the bark mulch processing building and wet sprinkler systems located in the maintenance building and E-Waste building. There are portable fire extinguishers located in the office building, maintenance building, processing building, beside the 10,000-gallon diesel aboveground storage tank (AST), and on all mobile equipment. There are four fire hydrants including one dry hydrant, as well as two fire department connections (FDC-A, FDC-B) connected to a main water supply fed from Hardscrabble Road, located on the E-Waste building and bark mulch processing building.

Fire prevention best management practices at the facility include pile management techniques such as "first in, first out", pile movement, rotation and limited residence time, and routine inspections for signs of heating. The facility addresses fires as either "incidental" or requiring an "emergency response". An incidental fire or other event is one that is limited in extent due to a lack of flammable or combustible material, is not likely to expand quickly, and does not involve hazardous matter that could explode or ignite violently. All other events are considered to require an emergency response. Facility employees will use fire extinguishers on incidental fires only. Evacuation protocol will be initiated immediately if a fire requiring an emergency response is encountered.

Poland Fire Chief Tom Printup toured the MB Bark facility on January 16, 2019, reviewed the project with MB Bark personnel and determined MB Bark has made adequate provision for fire prevention and protection.

Electricity

No additional electrical service or connections are proposed.

Solid Waste

MB Bark will coordinate the removal of non-processable solid wastes from its facility to existing licensed solid waste processing, recycling and disposal facilities.

Ground asphalt shingles will primarily be shipped to various hot-mix facilities in Northern New England to be used in asphalt manufacturing. Ground shingles may also be shipped to the CPRC facility and blended into Crushed and Recycled (C+R), an aggregate-based product used in construction projects.

Application for Site Plan Review Town of Poland Planning Board 100 Bark Mulch Drive, Auburn, Maine 04240 St.Germain Collins File No.: 2265-0002 January 2019 Attachment 1

Page 3

Non-processable by-pass material will be transported to the Riverside Recycling facility in Portland, Maine, then to the Juniper Ridge Landfill for disposal.

Outside Storage

The Pad 1 extension will include additional area for rail tie processing and storage, and finished product stockpiling. Pad 2 is designed to store $\frac{1}{2}$ " minus ground asphalt shingles and screen them to a $\frac{1}{4}$ " minus finished product referred to as Recycled Asphalt Shingles (RAS) in the pavement industry. Pad 3 is designed to store incoming raw asphalt shingles and grind them to a $\frac{1}{2}$ " minus product. Inert material (asphalt, brick and concrete) will also be stored on a portion of Pad 3. Per the facility's Operations Manual, no waste will remain onsite longer than 2 years.

Landscaping

There is no landscaping planned or required for this project. Buffers will be left with natural existing vegetation.

Lighting

No additional facility lighting is proposed.

Signs

No additional signs that are visible from offsite are proposed. Signs will be added interior to the site to direct traffic and designate areas for Pads 2 and 3.

Schedule

Construction will begin in the spring of 2019 and is expected to be complete within 18 months.

Parking

No additional parking spaces are proposed.

DEED

No. 224, LLC, a Maine limited liability Company with a place of business in Auburn, Maine, (hereinafter referred to as the "Grantor"), being a debtor under Chapter 11 of the U.S. Bankruptcy Code, for consideration paid, grants to MB Investment Properties, LLC a Maine limited liability company with a place of business in Scarborough, Maine (hereinafter sometimes referred to as "Grantee") the real property, together with any improvements thereon, situated in the Towns of Auburn and Poland, Androscoggin County, Maine, more particularly described on Exhibit A attached hereto and made a part hereof.

This Deed is given pursuant to the United States Bankruptcy Code, 11 U.S.C. Sections 105, 363 and 364 and pursuant to Final Order (1) Authorizing Sale of Certain of the Debtors' Assets to JWA Holdings Corp., or its Designee, Free and Clear of All Interests, Pursuant to 11 U.S.C. Sec 363; (2) Approving Procedures for the Assumption and Assignment of Executory Contracts; and (3) Granting Other Relief in Aid of Consummation of Sale, a true copy of which has been recorded in the Androscoggin County Registry of Deeds in Book 6658, Page 1.

The real estate conveyed by this deed is hereinafter sometimes referred to as the "Premises", the "Property" or the "premises herein conveyed".

The Premises are conveyed TOGETHER WITH all rights, easements, leases, licenses, permits, privileges and appurtenances belonging thereto including those described in **Exhibit A** attached hereto.

All easements and rights herein granted or excepted and reserved are intended to be permanent appurtenant easements benefiting the lands so designated for the benefit of the Grantee or the Grantor as the case may be and their respective, heirs, successors and assigns.

IN WITNESS WHEREOF, the Grantor has caused this instrument to be signed and sealed this 17th day of February, 2006, by and through Randall E. Male as authorized by the Order.

GRANTOR

NO. 224, LLC

By: Randall E. Male, authorized agent

FINANCIAL CONSULTANT

State of Maine County of Cumberland, ss.

February 17, 2006

PERSONALLY APPEARED the above-named Randall E. Male, in his capacity as aforesaid on behalf of Debtor No. 224, LLC pursuant to the Order under Chapter 11 of the U.S. Bankruptcy Code, and acknowledged the foregoing instrument to be his free act and deed in his said capacity and the free act and deed of said No. 224, LLC.

Before me,

Notary Public

Print Name:

Commission Expires:

SEAL

KATHLEEN L. GRIFFITH
Notary Public, Maine
My Commission Expires November 26, 2006

A certain lot or parcel of land with the buildings and improvements located thereon situated on the southwesterly side of West Hardscrabble Road in the City of Auburn and the Town of Poland, County of Androscoggin, State of Maine bounded and described as follows:

Beginning at a point on the southwesterly sideline of West Hardscrabble Road in the City of Auburn at the easterly corner of land now or formerly of Dancing Bear Realty Trust as recorded in the Androscoggin Registry of Deeds in Book 4709, Page 43 said point also being S 37⁻-21⁻-38" E along West Hardscrabble Road a distance of six hundred twenty-seven and 07/100 (627.07) feet from a 3 inch pipe found on the Auburn and Poland Town Line. Thence by the following courses and distances:

- 1) S 37'-21'-38" E along West Hardscrabble Road a distance of one hundred fifty and 43/100 (150.43) feet to an iron pin set (5/8" rebar) at land now or formerly of George H. & Kathryn H. Field as recorded in the said Registry in Book 799, Page 21 and Book 2479, Page 242.
- 2) S 56'-58'-54" W along land of said Field a distance of five hundred fifty-five and 76/100 (555.76) feet to an old axle in concrete found.
- 3) S 34'-46'-10" W along land of said Field a distance of one thousand seven hundred seventeen and 28/100 (1717.28) feet to an iron pipe found (1" pipe) at other land now or formerly of George H. & Kathryn H. Field as recorded in the said Registry in Book 2533, Page 79.
- 4) S 83'-49'-50" W along land of said Field a distance of six hundred eighty-one and 60/100 (681.60) feet to an iron pin set (5/8" rebar) on the said Auburn and Poland Town Line.
- 5) S 34'-35'-19" W along land of said Field and said Town Line a distance of one hundred nineteen and 81/100 (119.81) feet to an iron pin set (5/8" rebar) at land now or formerly of Port of Auburn, LLC as recorded in the said Registry in Book 6421, Page 120.
- 6) S 83'-49'-50" W along land of said LLC a distance of one thousand one hundred fifty-five and 95/100 (1155.95) feet to an iron pin set (5/8" rebar).
- 7) S 36-20'-14" W along land of said LLC a distance of nine hundred eighty-three and 07/100 (983.07) feet to an iron pin found (5/8" rebar) at land now or formerly of St. Lawrence & Atlantic Railroad.
- 8) N 39'-26'-40" W along said Railroad a distance of nine hundred ninety-five and 12/100 (995.12) feet to an iron pin set at land now or formerly of Dancing Bear Realty Trust.
- 9) N 83° 49' 50" E along said land of Dancing Bear Realty Trust a distance of one hundred ninety-four and 73/100 (194.73) feet to an iron pin set;
- 10) N 34° 23' 17" E along said land of Dancing Bear Realty Trust a distance of one thousand seventy-two and 31/100 (1072.31) feet to an iron pin set at land now or formerly of William T. & Karin Turner as recorded in said Registry of Deeds in Book 1037, Page 367;

- 11) N 34'-23'-17" E along land of said Turner and partly along an old wire fence a distance of two thousand one hundred sixty-one and 60/100 (2161.60) feet to an iron pin set (5/8" rebar) at land now or formerly of Albert R. Tibbetts Jr. and Gay E. & Garey J. Hodgkins as recorded in the said Registry in Book 4227, Page 345.
- 12) N 82'-34'-54" E along land of said Tibbetts and Hodgkins a distance of nine hundred sixteen and 97/100 (916.97) feet to an iron pin set (5/8" rebar).
- 13) N 33'-49'-45" E along land of said Tibbetts and Hodgkins a distance of eight hundred sixty-eight and 16/100 (868.16) feet to an iron pin set (5/8" rebar) on the southwesterly sideline of West Hardscrabble Road.
- 14) S 37-21'-38" E along West Hardscrabble Road a distance of two hundred eleven and 28/100 (211.28) feet to an iron pin set (5/8" rebar) at land now or formerly of the Town of Poland as recorded in the said Registry in Book 2680, Page 86.
- 15) S 33⁻-49⁻-45" W along land of said Town land now or formerly of Richard R. Lowell as recorded in the said Registry in Book 4547, Page 304 and land now or formerly of Alvin S. Jr. & Beverly Webster as recorded in the said Registry in Book 5832, Page 230 a distance of eight hundred and 03/100 (800.03) feet to an iron pin set (5/8" rebar).
- 16) S 41'-07'-04" E along land of said Webster a distance of eight hundred sixty-seven and 61/100 (867.61) feet to an iron pin set (5/8" rebar) at land of said Dancing Bear Realty Trust.
- 17) S 34'-27'-12" W along land of said Trust a distance of one hundred sixty-eight and 36/100 (168.36) feet to the centerline of Davis Brook.
- 18) Southeasterly along the centerline of Davis Brook and land of said Trust a distance of four hundred fifty-four (454) feet more or less to a point that is S 56°-58'-54" W a distance of seven hundred sixty-one and 17/100 (761.17) feet from the point of beginning.
- 19) N 56'-58'-54" E along land of said Trust a distance of seven hundred sixty-one and 17/100 (761.17) feet to the point of beginning

Reference is made to Plan of Property Hardscrabble Road, Auburn & Poland, Maine Made for MB Bark, LLC dated February 10, 2006 prepared by Cullenberg Land Surveying.

Also conveying rights for a driveway as reserved in deed from Granville L. Sawyer to J.F. Hammond dated April 19, 1920 and recorded in said Registry of Deeds in Book 295, Page 398.

ANDROSCOGGIN COUNTY

TING A CHOWNORD

REGISTER OF DEEDS



Town of Poland, Maine

Home Contact Admin

Last Updated 05/22/2018

Back to List

 Map/Lot
 0004-0017

 Book
 6674

 Page
 64

 Account
 267

Location OFF HARDSCRABBLE RD.

Owner MB INVESTMENT PROPERTIES, LLC

100 BARK MULCH DR. AUBURN ME 04210

Assessment

 Land
 47,760

 Building
 0

 Taxable
 47,760

Property Information

TypeResidentialAcreage31.84ZoneResidentialNeighborhoodTable 3

Land

 Description
 Type
 Units
 Value

 Rear Land 2
 Acres
 31.84
 47,760

 31.84
 47,760

Tax Detail

 Year
 Original
 Remaining

 2017
 687.27
 0.00

1 of 2



Town of Poland, Maine

Home Contact Admin

Last Updated 06/06/2018

Back to List

Map/Lot 0004-0015-0002

 Book
 6674

 Page
 60

 Account
 3332

Location 100 BARK MULCH DR.

Owner MB INVESTMENT PROPERTIES, LLC

100 BARK MULCH DR. AUBURN ME 04210

-Assessment-

Land896,050Building918,680Taxable1,814,730







Neighborhood

Property Information

Industrial

TypeResidentialAcreage119.20ZoneCommercial

-Land-Type **Description** Units Value Baselot (Fract) Fractional Acreage 200,000 1.84 67,500 Rear Land 2 45.00 Acres Rear Land 3 54.20 40,650 Acres Site Improvement 4 55,000 Improvements 1.00 7,900 Rear Land 1 Acres 3.16 525,000 Developed Rear Acres 15.00 896,050 119.20

Building———			
Area	9999		
Area	9999		
Area	48100		

	———Tax Detail————	
Year	Original	Remaining
2016	25,769.17	0.00

Back to List

MB Investments, LLC

January 14, 2019

MB Bark 100 Bark Mulch Drive Auburn, Maine 04210

RE: Formal Notice for Approval to use and construct on MB Investment Property

Dear Sirs:

This letter will serve as the Formal Notice to allow MB Bark to use and make any necessary improvements on the property currently owned by MB Investments for its purposes.

Should MB Bark require any other approvals or permissions for its future plans associated with the use of MB Investment's land, please contact me.

Sincerely

John W. Adelman

Sole Manager, MB Investments, LLC

LEASE

THIS LEASE, made and entered into effective the First day of August 1, 2007, by and between MB Investments, LLC, a Maine limited liability company (hereinafter referred to as "Landlord"), and MB Bark, LLC., a Maine limited liability company with a place of business in Brunswick Maine, (hereinafter referred to as "Tenant").

- 1. <u>Premises</u>. Landlord leases to Tenant, in consideration of the rent to be paid by Tenant and subject to the terms and conditions set forth herein, as more particularly described in **Exhibit A** attached hereto (the "Premises").
- 2. <u>Commencement and Term.</u> The term of this Lease (the "Lease Term") shall commence on the date hereof (the "Commencement Date"), and shall terminate twenty (20) years from the commencement date on July 31, 2027 (the "Termination Date").
- 3. Rent. Tenant covenants and agrees to pay to Landlord, during the Lease Term, an annual rental amount payable in monthly installments, in advance. This is a "net" lease and the Landlord shall not be required to provide any services or do any act in connection with the Premises except as specifically provided herein.

Base rent shall escalate on each anniversary date at the rate of increase in the Consumer Price Index (CPI-W) for the prior calendar year, which rent increase shall not exceed 3% annually.

4. <u>Security Deposit.</u> Tenant shall deposit an amount equal to the first and last month's rental, with the Landlord as a Security Deposit upon the execution of the Lease. Such deposit shall be kept in an interest bearing account by Landlord, and the original Deposit shall be returned to Tenant at the end of the Lease Term subject to the premises being left in their current 'broom clean' condition with no apparent defects.

5. Renewal Option.

The Tenant shall have the option to renew this lease for ten (10) additional years at rent based on terms and conditions acceptable to both Landlord and Tenant. The Tenant's renewal option shall be exercised in writing delivered at least six (6) months prior to the end of the final year of the initial lease term.

6. <u>Hazard Insurance</u>. Tenant agrees to maintain during the term hereof a policy of insurance insuring the Premises against loss or damage by fire and other perils commonly insured under so-called extended coverage in the greater Auburn, Maine area,. Tenant does hereby expressly release Landlord of and from and agrees to indemnify, hold harmless, and defend Landlord from any and all claims for damages or loss to Tenant's personal property regardless of the cause thereof, including, without limitation, damage or loss due to Landlord's

negligence and damage or loss due to theft, water, fire, explosion, sewer back up or any other hazard regardless of the cause thereof

- 7. <u>Holdover</u>. If Tenant continues to occupy the Leased Premised at the completion of the Lease Term, such continued occupancy shall be deemed a tenancy-at-will under the terms and conditions stated herein and shall be subject to a Rent equal to 125% of the Rent applicable at the end of the Lease Term until Tenant shall vacate the Leased Premises. Nothing contained in this Paragraph shall be deemed to constitute consent by Landlord to such occupancy or holdover by Tenant.
- 8. <u>Utilities</u>. During the Lease Term, Tenant covenants and agrees to pay the cost of all utility services provided to or for the Premises and directly metered including but not limited to the cost of installing a separate meter or sub meter for the building.
- 9. Repair and Maintenance. Tenant agrees that from and after the date that possession of the Premises is delivered to Tenant, and until the end of the Lease Term, it will maintain in the same order, condition and repair as the Premises is in as of the Commencement Date. Tenant shall be responsible for non-capital and routine maintenance of the heating, plumbing, air conditioning and lighting systems in the Premises. Should Tenant want to make any structural improvements to the building, such improvements will be done upon Landlord's consent.
- 10. Alterations, Renovations and Improvements. Tenant shall not make material alterations, renovations and improvements to the Premises without the prior written consent of Landlord, which consent shall not be unreasonably withheld, conditioned or delayed. Tenant shall perform such alterations, renovations and improvements in a good and workmanlike manner, and in accordance with all applicable laws and provided further that Tenant shall indemnify and hold Landlord harmless from and against all claims, demands, costs and mechanic's liens which may arise as a direct or indirect result of or in connection with such alterations, renovations and improvements, and Tenant shall assume all cost, liability and responsibility for such alterations, renovations and improvements, except for maintenance and repair of such alterations, renovations and improvements after expiration of the term of this Lease. Any and all alterations, renovations and improvements which may be made or installed by either Landlord or Tenant upon the Premises and which in any manner are attached to the floors, walls or ceilings (including, without limitation, any linoleum or other floor coverings of similar character which may be cemented or otherwise adhesively affixed to the floor) shall, remain upon the Premises, and at the expiration or termination of this Lease shall be surrendered with the Premises as a part thereof without disturbance, molestation or injury. However, the usual trade fixtures and furniture which may be installed in the Premises prior to or during the term hereof at the cost of Tenant may be removed by Tenant from the Premises upon the expiration or termination of this Lease, subject to the provisions of Paragraph 10 below.
- 11. <u>Fixtures</u>. All fixtures erected on and/or attached to the Premises by Tenant may be removed by Tenant at the termination of this Lease, provided such removal shall not permanently or substantially damage any portion of the Premises as they existed prior to the commencement of the Lease Term, and any minor damage created by such removal shall be repaired by Tenant at Tenant's expense prior to the expiration of the Lease Term.

12. <u>Subletting and Assignment</u>. Tenant shall not be entitled to assign this Lease or to sublet the Premises or any portion thereof, without the prior written consent of Landlord which consent shall be in Landlord's reasonable discretion. Notwithstanding the foregoing, Tenant may mortgage or collaterally assign this Lease to its Lender, and any such Lender may further assign this Lease to a third-party purchaser in the event of foreclosure, all without Landlord's consent.

13. <u>Indemnification and Liability Insurance</u>.

- (a) Tenant shall indemnify and hold Landlord harmless and, if requested by Landlord, defend Landlord with counsel reasonably satisfactory to Landlord, from and against any and all liabilities, losses, claims, causes of action, damages, costs, and expenses (including reasonable attorneys fees) incurred by or threatened against Landlord (i) arising out any occurrence on the Leased Premise or of the use of the Leased Premises by Tenant, its employees, agents, licensees, or invitees except to the extent caused by the negligence or willful misconduct of Landlord; or (ii) arising out of any negligence, or intentional misconduct of Tenant, its employees, agents, licensees, or invitees wherever occasioned; or (iii) arising out of any actual breach of the obligations to be performed or terms to be observed by Tenant under this Lease, as determined by a final, unappealable judgment. Tenant agrees that the foregoing agreement to indemnify, defend, and hold harmless extends to liabilities, losses, claims, causes of action, damages, costs and expenses (including reasonable attorneys fees) arising out of claims of Tenant's employees without regard to any immunity, statutory or otherwise, including any immunity under the workers compensation laws of Maine or any other applicable jurisdiction. Tenant's obligations under this paragraph shall survive the termination of this Lease.
- (b) Tenant agrees to maintain in full force during the Lease Term and any extension thereof a policy of Commercial General Liability Insurance on an occurrence basis, with a deductible in an amount not to exceed \$1,000.00, under which Tenant is named as an insured and Landlord by an endorsement satisfactory to Landlord or provided on ISO Form 2026 (1185) or its equivalent, without modification, are named as additional insureds with respect to this Lease, in a minimum amount of One Million Dollars (\$1,000,000.00) for injury or death of any one person or damage to property, and Two Million Dollars (\$2,000,000.00) for injury to or death of more than one person in a single accident or occurrence (the general aggregate limit shall apply on a "per location" basis). Such policy shall contain a provision requiring that written notice be given to Landlord not less than ten (10) days prior to cancellation, expiration or alteration of the policy. Tenant agrees to deliver a duplicate original insurance policy (together with evidence satisfactory to Landlord, naming Landlord as an additional insured), insurance binder (countersigned by the insurer) or Evidence of Insurance (in ACORD Form 27) for such insurance to Landlord on or before the Commencement Date and thereafter not less than thirty (30) days prior to the expiration of any such policy and at any time requested by Landlord.
- 14. <u>Use and Business Operation</u>. Tenant agrees to use and occupy the Premises for its electronic recycling business or any expansion of that business into related activities allowed by law on the Premises, and for no other purpose without the written consent of Landlord, and further agrees not to use the Premises for any purpose deemed hazardous. Tenant shall be responsible to provide all insurances necessary for the conduct of its business and be responsible for all liabilities created as a result of its business during the term of its lease

- 15. Permits and Licenses. Tenant agrees to attain and to maintain in full force and effect, during the Lease Term and, if applicable, any renewal term, at Tenant's cost and expense, any and all federal, state and local permits, licenses and registrations necessary for the specific use of the Premises by Tenant pursuant to Paragraph 14 hereof. Tenant further agrees that it will be responsible for all liabilities associated with such licenses and permits during its lease term. Furthermore, upon the termination of the lease, for any reason, Tenant shall release its licenses and permits to Landlord and cooperate with Landlord in the process of transferring such should it be deemed necessary by Landlord.
- 16. Taxes. Tenant agrees to reimburse Landlord for its prorata share all property taxes and assessments, ordinary or extraordinary, by whomsoever levied or assessed, for all tax years which fall within the Lease Term (or the portion thereof), which may be levied or assessed on the Premises, and agree to pay directly to the taxing authority any and all other taxes and assessments associated with Tenant's use of the Premises, including but not limited to personal property taxes and assessments and all federal, state and local forms of withholding and FICA taxes and assessments. In the event that any municipal betterment or special assessment is levied on the Premises during the term of this Lease, Landlord shall elect to make installment payments thereon over the longest period allowed, and Tenant shall only be responsible for those installment payments due during the term of this Lease. Upon receipt of a bill or statement for any such taxes or assessments, which are not charged, levied or otherwise assessed directly to or against Tenant by a federal, state or local taxing authority, Landlord shall pay the amount owed and forward a copy of the statement to Tenant. Tenant shall forward payment of its prorata share of such statement amount to Landlord on each such occasion within thirty (30) days of receipt of said statement from Landlord.
- 17. Right to Enter. Tenant agrees to permit Landlord or its duly authorized agents to enter on the Premises during Tenant's normal business hours, with at least 12 hours prior notice, to examine the condition of said Premises, provided such access to the Premises shall not unnecessarily interfere with Tenant's use of the Premises or the conduct of Tenant's business activities thereon. In the event that Landlord wishes to enter the Premises at any time other than Tenant's normal business hours, Landlord shall give Tenant written notice at least 24 hours in advance except that in case of an emergency, Landlord shall be relieved of said notice obligation.
- 18. Attorneys Fees. In the event either party defaults in any manner pursuant to the terms of this Lease, including but not limited the institution of bankruptcy proceedings by or against Tenant such as would constitute a default pursuant to Paragraph 22 of this Lease, the other party agrees to pay all reasonable costs, attorneys fees and expenses that shall be made and incurred by the first party in successfully enforcing the terms of this Lease.
- 19. <u>Total or Partial Destruction</u>. If the Premises shall be substantially damaged by fire or other casualty, either Landlord or Tenant may terminate this Lease within 30 days of such casualty. In the event that neither Tenant nor Landlord terminates this Lease, Landlord shall cause such damage promptly to be repaired or replaced but Landlord shall not be obligated to spend more on such repair or replacement than the insurance proceeds available to Landlord.

Rent shall be abated during the period of any repairs in proportion to the amount of the Premises rendered unusable by Tenant.

20. Eminent Domain.

- (a) If the Premises shall be taken, in whole or in substantial part, by condemnation or right of eminent domain, either party, upon written notice to the other, shall be entitled to terminate this Lease provided that such notice is given not later than thirty (30) days after Tenant has been deprived of possession. Should any part of the Premises be so taken or condemned, and should this Lease not be terminated in accordance with this Paragraph, Landlord shall, after such taking or condemnation and the determination of Landlord's award therein, expend a portion or all of the net amount which may be awarded to Landlord in such condemnation proceedings as may be necessary to restore the Premises to functionally as nearly like their condition at the commencement of the Lease Term as shall be practicable. Should the net amount so awarded to Landlord be insufficient to cover the cost of restoring the Premises, Landlord may supply the amount of such insufficiency and restore the Premises as above provided with all reasonable diligence, or terminate this Lease. If Tenant has not already exercised any right of termination accorded to it under this Paragraph, Landlord shall notify Tenant of Landlord's election with respect to restoration in the event of an insufficient award not later than ninety (90) days after the final determination of the amount of the award.
- (b) In the event of any award for any taking of the Premises in condemnation proceedings or by right of eminent domain, Landlord shall be entitled to receive and retain the amounts awarded for the Premises and for Landlord's business loss, and Tenant shall be entitled to receive and retain any amounts which may be specifically awarded to it in any such condemnation proceedings because of its business loss or the taking of its trade fixtures, furniture, or other property and relocation costs.
- (c) In the event of any such taking of the Premises, the rent, or a fair and just proportion thereof according to the nature and extent of the damage sustained, shall be suspended or abated.
- 21. <u>Limitation of Landlord's Liability</u>. Tenant agrees to look solely to Landlord's interest in the property of which the Premises is a part or Landlord's insurance coverage thereon for recovery of any judgment from Landlord. The foregoing shall not limit Tenant's right to seek injunctive relief for Landlord's breach of this Lease.
- 22. Waiver of Subrogation. Insofar as and to the extent that such agreement may be effective without invalidating or making it impossible to secure insurance coverage obtainable from responsible insurance companies doing business in the State of Maine, Landlord and Tenant agree that with respect to any loss covered by insurance then carried by them, respectively, the one carrying such insurance and suffering that loss releases the other of and from any and all claims with respect to such loss; and they further agree that their respective insurance companies shall have no right of subrogation against one another on account of such agreement even though extra premiums may result therefrom. If an extra premium is payable by

Tenant as a result of these provisions, Landlord shall not reimburse Tenant for any such extra premium.

23. Default.

(a) It is covenanted and agreed that

- (i) if Tenant shall neglect or fail to perform or observe, or fail or neglect diligently to attempt to so perform or observe any of the covenants, terms, provisions or conditions contained in this Lease and on Tenant's part to be performed or observed within thirty (30) days after notice of default; provided, however, that if such neglect or failure is not capable of being cured within said thirty (30) day period then Tenant shall have an additional period of time, not to exceed sixty (60) days, to cure the same provided Tenant commences to cure within said thirty (30) day period and diligently and continually prosecutes the same to completion (except for payment of Rent or other charges or sums payable by Tenant, in which case the cure period shall be five (5) days after notice thereof and except that, during any calendar year, Landlord shall be relieved of its notice obligations with respect to rent under this Paragraph if it gives any such notice twice in any calendar year);
- (ii) if the estate hereby created shall be taken on execution, by attachment or by other process of law, or if a petition in U.S. Bankruptcy Court shall be filed by Tenant, or if any assignment shall be made of the property of Tenant for the benefit of creditors;
- (iii) if a receiver, guardian, conservator, trustee in involuntary bankruptcy or other similar officer shall be appointed by a court of competent jurisdiction to take charge of all or any substantial part of Tenant's property;
- (iv) if an involuntary petition shall be filed for the reorganization of Tenant under any provisions of the Federal Bankruptcy Code now or hereafter enacted, and such proceeding is not dismissed within sixty (60) days after it is begun, or if Tenant shall file a petition for such reorganization under any provisions of the Federal Bankruptcy Code now or hereafter enacted;
- (v) if Tenant shall assign its interest in this Lease or sublet any portion of the Premises, or attempt to do either of the foregoing, except as may be expressly permitted in this Lease; or

(vi) intentionally deleted;

then, and in any of said cases (notwithstanding any license of any former breach of covenant or waiver of the benefit hereof or consent in a former instance), Landlord lawfully may, immediately or at any time thereafter, terminate this lease by sending written notice of termination to Tenant, or, in accordance with Maine law, enter into and upon the Leased Premises or any part thereof in the name of the whole and repossess the same as of its former

estate, and expel Tenant and those claiming through or under it and remove it or their effects without being deemed guilty of any manner of trespass, and without prejudice to any remedies which might otherwise be used for collection of damages for breach of covenant, and upon entry as aforesaid or upon sending of such notice, this Lease shall terminate.

- (b) Without limiting other remedies of Landlord, Tenant covenants that in case of such termination under subparagraph (a) Tenant shall, notwithstanding such termination, pay to Landlord for the remainder of the Lease Term (or Renewal Term, if applicable) on the last day of each calendar month the difference, if any, between the Rent which would have been due for such month had there been no such termination and the sum of the amount being received by Landlord as rental from the then occupants of the Leased Premises, if any. Landlord shall make reasonable efforts to secure a rental equal to the prevailing local rate for the Leased Premises. In addition, Tenant agrees to pay to Landlord as damages for any above-described breach, all commercially reasonable costs of reletting the Leased Premises, including but not limited to commissions, attorneys fees, and court costs.
- (c) If Tenant shall default in the performance or observance of any covenant, agreement, or condition in this Lease contained on its part to be performed or observed, other than an obligation to pay money, and shall not cure any such default as provided herein, Landlord may, at its option, without waiving any claim for damages for breach of this Lease, at any time after expiration of applicable cure period, cure such default. Any commercially reasonable amount paid or any liability incurred by Landlord in so doing shall be deemed paid or incurred for the account of Tenant, and Tenant agrees to immediately reimburse Landlord therefor, as additional Rent, or save Landlord harmless therefrom.
- (d) Landlord shall in no event be in default in the performance of any of its obligations hereunder unless and until Landlord shall have failed to perform, or failed diligently to attempt to perform, such obligations within thirty (30) days or such additional time as is reasonably required to correct any such default after notice by Tenant to Landlord properly specifying wherein Landlord has failed to perform any such obligation, but in no event longer than sixty (60) days.

24. Landlord's Right to Sell or Mortgage Fee.

- (a) Nothing contained in this Lease shall limit or curtail Landlord's right to sell, mortgage, or otherwise deal with its fee interest in the Leased Premises, or affect Landlord's right to assign the net Rent payable under this Lease either as further collateral security under a fee mortgage or otherwise. Any such assignment of Rent shall be honored by Tenant.
- (b) In the event Landlord shall sell, transfer, or otherwise convey to a purchaser the Premises, subject to the leasehold estate, Landlord, upon the written assumption thereof by the new owner, shall be entirely freed and relieved of all covenants and obligations of Landlord hereunder thereafter accruing, and it shall be deemed and construed without further agreement between the parties and the purchaser at any such sale that said purchaser has assumed and agreed to carry out any and all covenants and obligations of Landlord hereunder. Nothing herein shall be construed to affect this Lease or Tenant's quiet enjoyment of the

Premises free of any claims by, through or under Landlord, so long as Tenant performs and observes the covenants and terms of this Lease on its part to be performed and observed.

25. <u>Notices</u>. All notices required to be given pursuant to this Lease, to be effective, shall be in writing and shall be delivered by hand or by certified mail, postage prepaid, return receipt requested, to the following addresses:

(i) To Tenant at: MB Bark, LLC.

100 Bark Mulch Drive Auburn, Maine 04210

(ii) To Landlord at: MB Investments, LLC

70 Pleasant Hill Rd Scarborough, ME 04074

With a copy to: Wayne E. Tumlin, Esq.

Bernstein, Shur, Sawyer & Nelson 100 Middle Street, P.O. Box 9729 Portland, Maine 04104-5029

Any notice given pursuant to this Paragraph shall be deemed to have been given upon proven receipt. Either party may, by such manner of notice, substitute persons or addresses for notice other than those listed above.

- 26. <u>Hazardous Waste</u>. Tenant covenants and agrees that it will permit no hazardous or toxic waste, substance, material or matter, as those terms may be defined from time to time by applicable state, local or federal law to be brought, used, maintained or stored upon the Premises except in compliance with applicable law. Tenant hereby covenants and agrees to protect, exonerate, defend, indemnify and save Landlord harmless from and against any and all loss, damage, cost, expense or liability, including reasonable attorneys fees, court costs and clean-up costs, and including but not limited to, such loss, damage, cost, expense or liability based on personal injury, death, loss or damage to property suffered or incurred by any person, corporation or other legal entity, which may arise out of the removal or clean-up of any such waste, substance, material or matter placed upon or within the Leased Premises by Tenant after the date of this lease, whether or not in violation of law, or as the result of a breach by Tenant of Tenant's obligations under this Paragraph-
- 27. <u>Subordination</u>. This Lease, at Landlord's option, shall be subordinate to any ground lease, mortgage, deed of trust, or any other hypothecation or security now or hereafter placed upon the Premises and to any and all advances made on the security thereof and to all renewals, modifications, consolidations, replacements and extensions thereof, provided, however, that the holder of such right agrees not to disturb Tenant's rights under this Lease so long as Tenant is not in default beyond applicable grace or cure periods. Notwithstanding such subordination, Tenant's right to quiet possession of the Premises shall not be disturbed if Tenant is not in default and so long as Tenant shall pay the Rent and observe and perform all of the

provisions of this Lease, unless this Lease is otherwise terminated pursuant to its terms. If any mortgagee, trustee, or ground lessor shall elect to have this Lease made prior to the lien of its mortgage, deed of trust or ground lease, and shall give written notice thereof to Tenant, this Lease shall be deemed prior to such mortgage, deed of trust, or ground lease, whether this Lease is dated prior to or subsequent to the date of said mortgage, deed of trust, or ground lease or the date of recording thereof. Tenant agrees to execute any reasonable documents reasonably required to effectuate an attornment, a subordination or to make this Lease prior to the lien of any mortgage, deed of trust or ground lease, as the case may be, provided that holder of such rights agrees not to disturb Tenant's rights under this Lease as provided above. Tenant's failure to execute such documents within ten (10) days after written demand shall constitute a material default by Tenant hereunder.

28. Miscellaneous Provisions.

- (a) <u>Invalidity of Particular Provisions</u>. If any term or provision of this Lease, or the application thereof to any person or circumstance shall, to any extent, be invalid or unenforceable, the remainder of this Lease, or the application of such term or provision to persons or circumstances other than those as to which it is held invalid or unenforceable, shall not be affected thereby, and each term and provision of this Lease shall be valid and be enforced to the fullest extent permitted by law.
- (b) Governing Law. This Lease shall be governed exclusively by the provisions hereof and by the laws in effect in the State of Maine as those laws may be amended from time to time.
- (c) <u>Recording</u>. Landlord and Tenant agree that a Memorandum of Lease, upon the request of either party, may be recorded in the Cumberland County Registry of Deeds, as soon as possible after the execution hereof. The Memorandum shall expressly state that it is executed pursuant to the provisions contained in this Lease, and is not intended to vary the terms and conditions hereof.
- (d) <u>Paragraph Headings</u>. The Paragraph headings throughout this instrument are for convenience and reference only, and the words contained therein shall in no way be held to explain, modify, amplify, or aid in the interpretation, construction, or meaning of the provisions of this Lease.
- (e) <u>Interpretation</u>. Whenever in this Lease provision is made for the doing of any act by any party, it is understood and agreed that said act shall be done by such party at its own cost and expense, unless a contrary intent is expressed.
- (f) Entire Agreement; Binding Effect. All negotiations, considerations, representations, and understandings between Landlord and Tenant with respect to the specific subject matter of this Lease are incorporated herein and may be modified or altered only by agreement in writing between Landlord and Tenant, and no act or omission of any employee or agent of Landlord or Tenant shall alter, change, or modify any of the provisions hereof. All rights, obligations and liabilities contained herein given to, or imposed upon, Landlord and Tenant shall extend to and bind the several respective administrators, trustees, receivers, legal

representatives, successors, heirs and permitted assigns of Landlord and Tenant, and if there shall be more than one tenant, they shall all be bound jointly and severally by the terms, covenants and agreements herein. Those portions of a previous Lease between Gibson Realty, LLC and Tenant dated April 9, 2004, related to the Premises are superseded by this Lease. The effectiveness and enforceability of this Lease is conditioned upon the purchase by Landlord of the real property located at 12 Runway Road, Scarborough, Maine, of which the leased premises is a part.

- (g) <u>Compliance with Laws</u>. Tenant agrees to abide by and comply with all federal, state and local statutes, ordinances, rules and regulations applicable to Tenant's use of the Premises.
- (h) Gender: Words of any gender used in this instrument shall be held and construed to include any other gender, and words in the singular number shall be held to include the plural, unless the context otherwise requires.

IN WITNESS WHEREOF, Landlord and Tenant have executed this Lease Agreement as an instrument under seal as of the day and year first above-written.

By:

By:

By:

By:

By:

By:

John Adelman
Its: Manager

TENANT
MB Bark, LLC

By:

John Adelman
Its: President/CEO

EXHIBIT A TO LEASE

DESCRIPTION OF PREMISES

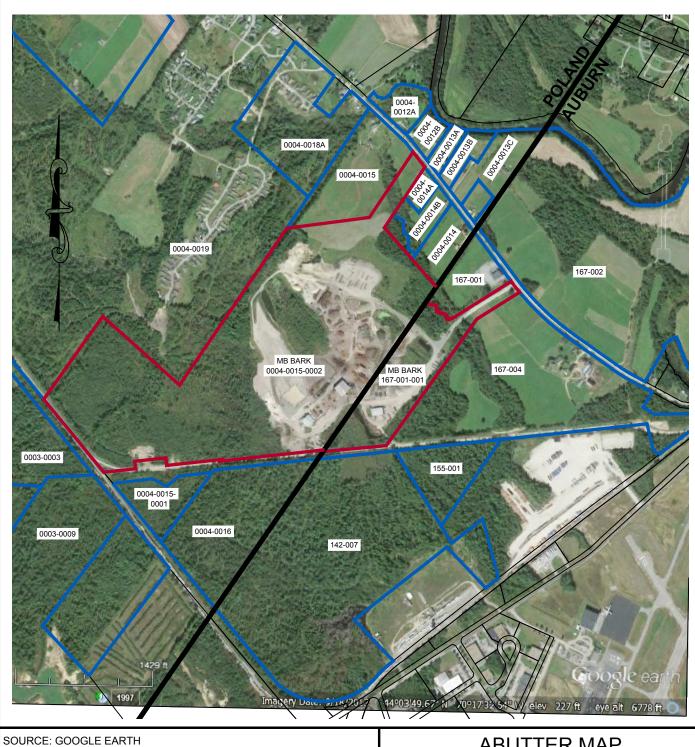
All Land, Buildings and Equipment necessary to operate a bark mulch business.

Abutters within 500' MB Bark, LLC 100 Bark Mulch Drive Auburn, Maine

	Parcel Number	Property Address	Owner Name	Owner Address	Owner City	Owner State	Owner Zip
	0003-0003	34 TORREY RD.	DICK, STEPHEN W. II	34 TORREY RD.	POLAND	ME	04274
	0003-0009	202 TORREY RD.	FERLAND, DENNIS R.	193 HARDSCRABBLE RD.	POLAND	ME	04274
	0004-0012A	411 HARDSCRABBLE RD.	MERCHANT, HAROLD W.	417 HARDSCRABBLE ROAD	MINOT	ME	04274
	0004-0012B	429 HARDSCRABBLE RD.	DAIGNEAULT, JOHN P.	429 HARDSCRABBLE RD.	POLAND	ME	04274
	0004-0013A	439 HARDSCRABBLE RD.	PROVOST, SETH T.	439 HARDSCRABBLE RD.	POLAND	ME	04274
	0004-0013B	449 HARDSCRABBLE RD.	HUDSON, MICHELLE	449 HARDSCRABBLE RD.	POLAND	ME	04274
9	0004-0013C	469 HARDSCRABBLE RD	FIELD, CINDY L.	714 BALD HILL RD.	NEW GLOUCESTER	ME	04260
Poland	0004-0014	470 HARDSCRABBLE RD.	WEBSTER, ALVIN S., JR.	470 HARDSCRABBLE RD.	POLAND	ME	04274
Po	0004-0014A	HARDSCRABBLE RD.	POLAND, TOWN OF	1231 MAINE ST.	POLAND	ME	04274
	0004-0014B	450 HARDSCRABBLE RD.	LOWELL, RICHARD R.	450 HARDSCRABBLE RD.	POLAND	ME	04274
	0004-0015	404 HARDSCRABBLE RD.	TIBBETTS, ALBERT R., JR.	404 HARDSCRABBLE RD.	POLAND	ME	04274
	0004-0015-0001	BARK MULCH DR.	LEWISTON AND AUBURN RAILROAD CO.	415 LISBON ST., SUITE 400	LEWISTON	ME	04240
	0004-0016	OFF HARDSCRABBLE RD.	PORT OF AUBURN, LLC	54 BARTOL ISLAND RD	FREEPORT	ME	04032
	0004-0018A	HARDSCRABBLE RD.	BROOKDALE VILLAGE LLC	314 CENTER ST.	AUBURN	ME	04210
	0004-0019	338 HARDSCRABBLE RD.	BROOKDALE VILLAGE, LLC	314 CENTER ST.	AUBURN	ME	04210
	167-002	WEST HARDSCRABBLE RD	FIELD GEORGE H	116 WEST HARDSCRABBLE ROAD	AUBURN	ME	04210
_	167-004	116 WEST HARDSCRABBLE RD	FIELD GEORGE H	116 WEST HARDSCRABBLE ROAD	AUBURN	ME	04210
בַ	142-007 LEWISTON JUNCTION RD	DODT OF AUDURNIUS	C/O KAREN & FORD REICHE	FREEPORT	ME	04032	
Auburn		LEWISTON JUNCTION RD PORT OF AUBURN LLC	54 BARTOL ISLAND RD				
٩	155-001	LEWISTON JUNCTION RD	LEWISTON & AUBURN RAILROAD CO	415 LISBON STREET, STE 400	LEWISTON	ME	04240
	167-004-001	WEST HARDSCRABBLE RD	LEWISTON & AUBURN RAILROAD CO	415 LISBON STREET, STE 400	LEWISTON	ME	04240

St.Germain Collins File No.: 2265-0002

1/31/2019 Page 1



MB BARK PROPERTY BOUNDARY

ABUTTERS

ABUTTER MAP

MB BARK, LLC 100 BARK MULCH DRIVE AUBURN, MAINE

MB BARK, LLC 100 BARK MULCH DRIVE AUBURN, MAINE



FIGURE 1

SCALE: 1"=1,000'

PROJECT NO.: 2265

FILE: MB Bark Abutter Map

The key to success starts with a solid foundation. ENGINEERING | EXPLORATION | EXPERIENCE

Geotechnical Evaluation

Proposed Pad #1 Expansion MB Bark Mulch, Auburn, Maine





Client

St. Germain Collins 846 Main Street Westbrook, Maine 04092

> Project #: 18019 Date: 2/23/18

145 Lisbon Street (PO Box 7216) Lewiston, Maine 04243 | (207) 576-3313 173 Pleasant Street Rockland, Maine 04841 | (207) 318-7761 www.summitgeoeng.com



February 23, 2018 SGS #18019

Peter Mohlin St.Germain Collins 846 Main St. Westbrook, ME 04092

Reference: Geotechnical Investigation, MB Bark Recycling Facility Expansion

Bark Mulch Drive, Auburn, Maine

Dear Peter;

Summit Geoengineering Services, Inc. (SGS) has completed a geotechnical investigation relative to the construction of three storage pads at the MB Bark Recycling Facility (MB Bark). This report addresses our evaluation of the suitability of the existing soil and the potential reuse of the various materials anticipated to be excavated at the site and used to construct the pads. Specifically this report is intended to address the following:

MEDEP Ch. 409 Section 3(H) - Subsurface Investigation. A subsurface investigation must be conducted whenever the proposed processing facility includes the use of in-situ soils as any part of a soil base pad for handling solid wastes, includes structures requiring foundations, or includes subsurface wastewater holding or disposal systems. The data must consist of soil test data in the proposed handling areas from a certified professional describing and evaluating the surficial geology and/or the subsurface soils. This information must demonstrate that the facility design is compatible with the site's soil characteristics, as determined by applicable engineering standards of practice.

Project Description

The project consists of the expansion of one existing storage pad (Pad #1) and construction of two new storage pads (Pads #2&3)at the MB Bark site in Auburn, Maine. Pad #1 will be located directly adjacent to and north of the existing facility. Pads #2 and #3 will be located in undeveloped areas west of the existing facility.

The pads will be constructed using a combination of an existing recycled (reclaim) product and native soil excavated during construction. Based on the anticipated cuts and fills, the vast majority of the pads will be constructed using the recycled material. For Pad #1, the final embankment height is as high as 25 feet above the existing ground surface. The maximum fill height for Pads #2 and #3 is in the range of 10 to 12 feet.



This report is specifically focused on Pad #1. We anticipate that the evaluations and recommendations in this report will also be applicable to Pad #2 and Pad #3. Additional subsurface explorations will be required at the appropriate time to confirm this opinion.

Exploration and Subsurface Conditions

A total of 6 test pits were excavated in the area of the proposed Pad #1 expansion to explore the existing subsurface conditions. The test pits were excavated on February 15, 2018 by MB Bark under the direction of SGS. The location of the test pits is shown on the Exploration Location Plan in Appendix A. The test pits were excavated to depths ranging from 3.5 (bedrock) to 17 feet below the existing ground surface using a large tracked excavator.

A significant geological feature in the Pad #1 area is a bedrock knoll in the west side of the site. The bedrock in this area has been partially mined and a small quarry remains. Surficial geologic maps indicate that the soil at the site consists of a sandy marine regressive deposit and silty clay (Presumpscot Formation). Based on this mapping and our observations of the test pits, we believe that soil material above approximately elevation 250 feet will consist of the marine regressive deposit and soil below approximately elevation 250 feet will consist of the Presumpscot Formation. Shallow bedrock can be anticipated in knolls present at the site above elevation 260 feet.

The marine regressive deposit, encountered in TP-4 and TP-6, is described as olive-brown silty fine sand. This soil is moist and loose and has a USCS classification of SM.

The silty clay soil was encountered in all the test pits except for TP-4. This soil is described as olive-brown silty clay. This soil is friable and blocky in the upper 10 feet and then becomes wetter and less blocky with depth. Pocket penetrometer tests (a rough measure of the soil shear strength) ranged from 3.0 to 4.5 tsf at a depth of 4 feet and 0.7 to 1.0 tsf at a depth of 16 feet. This soil has a USCS soil classification of CL.

TP-5 was excavated in the existing pad area to observe the reclaim material. This material is described as black sand with a little gravel and silt and a few cobbles. This material was dense and stable. The sidewalls of the excavation were vertical, showing no signs of sloughing.

Bedrock was encountered at the TP-4 location at a depth of 3.5 feet (approximately elevation 251.5 ft).

Groundwater was observed at the following depths and approximate elevations.



GROUNDWATER OBSERVATIONS				
Location	Depth (ft)	Approx. Elevation (ft)		
TP-1	9.5	222.5		
TP-2	10	221		
TP-3	None	Deeper than 224		
TP-4	None	Deeper than 251.5		
TP-5	None	Deeper than 247		
TP-6	None	Deeper than 245		

Evaluation

In general, the bedrock, marine regressive, and silty clay soil at the site are inherently stable and will provide adequate support for the proposed reclaim pads. Silty clay soil is susceptible to circular global failures under large fill loads. We understand that up to 25 feet of fill will be placed to expand the existing Pad #1 to the east. The existing pad with an equivalent height of fill is currently stable. There are no signs of global stability issues with the existing pad. We anticipate that the stiff silty clay soil has adequate strength to support the proposed fill. A numerical global stability analysis will be performed to confirm this opinion.

Existing materials expected to be cut to achieve final pad grades include bedrock and the marine regressive deposit. With proper conditioning, both of these materials are suitable for use in constructing the proposed pads. Proper conditioning would include crushing the bedrock to the required sizes and placing the marine regressive soil in 12" to 24" compacted lifts. A grain size analysis is being conducted on a sample of the marine regressive soil collected at a depth of 3 feet at the TP-6 condition.

Based on the proposed finished grades of Pad #1, excavation into the silty clay soil is not expected. In the event that this soil is excavated it will be suitable for reuse if properly conditioned, placed and compacted. Specific recommendations for conditioning, placing, and compacting this soil may include moisture conditioning (drying), placing in relatively thin lifts, and compacting with the appropriate equipment (e.g. a sheepsfoot roller).

The reclaim material appears to be stable. The soil exposed in TP-5 was granular and dense. This material has sufficient fines to bind the larger particles together into a dense state. Grain size analyses and direct shear tests are currently being performed to further identify the physical characteristics of the reclaim material.



Conclusions

In general the soil at the site is suitable to support the proposed Pad #1 construction. The existing native soil, bedrock, and reclaim materials appear to be suitable for reuse in the Pad #1 construction. Additional testing and analyses are currently being performed to determine specific material conditioning, handling, and placement and compaction procedures.

Thank you once again for this opportunity. If there are any questions, please do not hesitate to contact me.

Sincerely,

Summit Geoengineering Services, Inc.

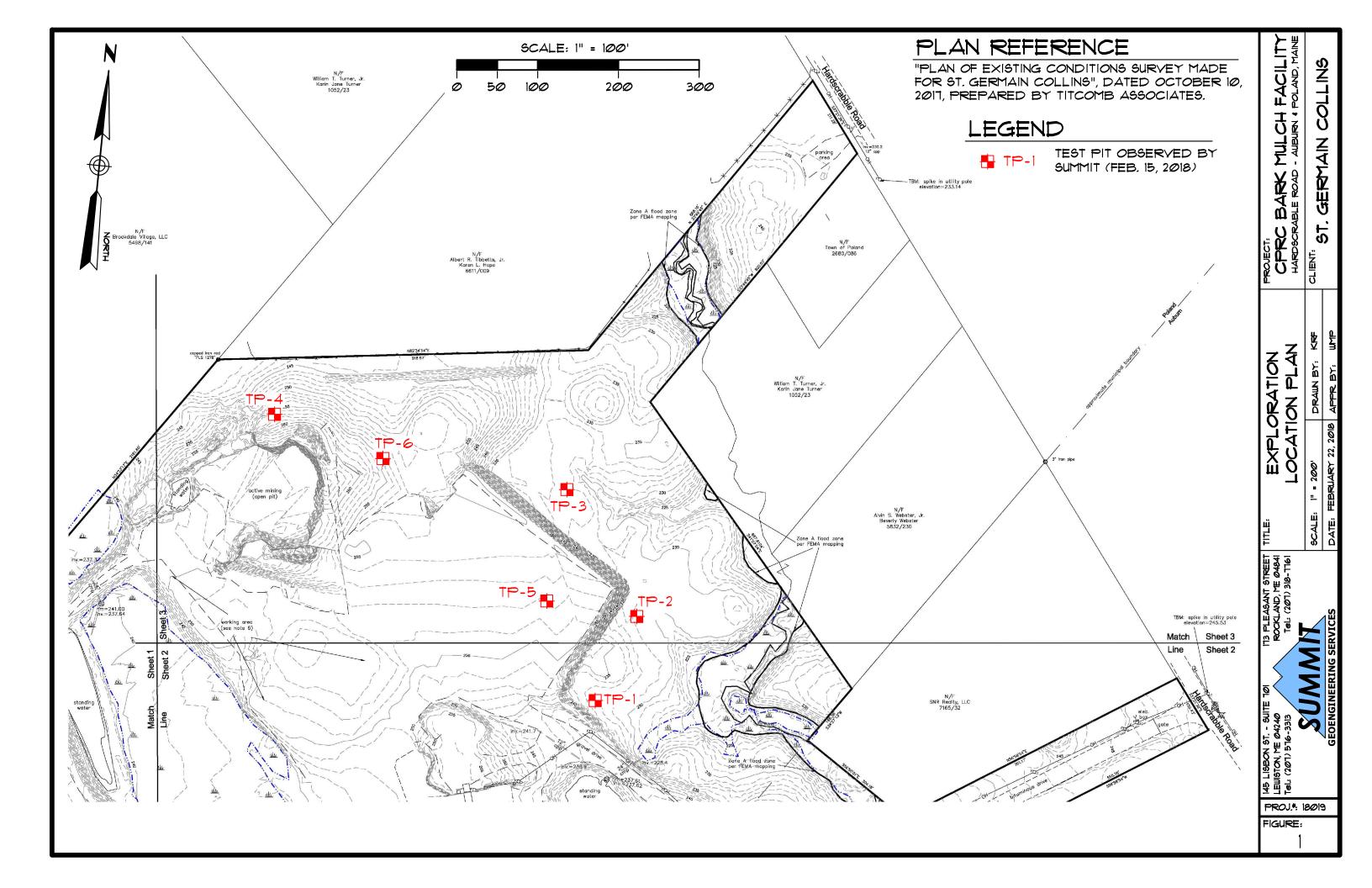
William M. Peterlein, P.E.

President & Principal Engineer

Wholen on Return



APPENDIX A EXPLORATION LOCATION PLAN

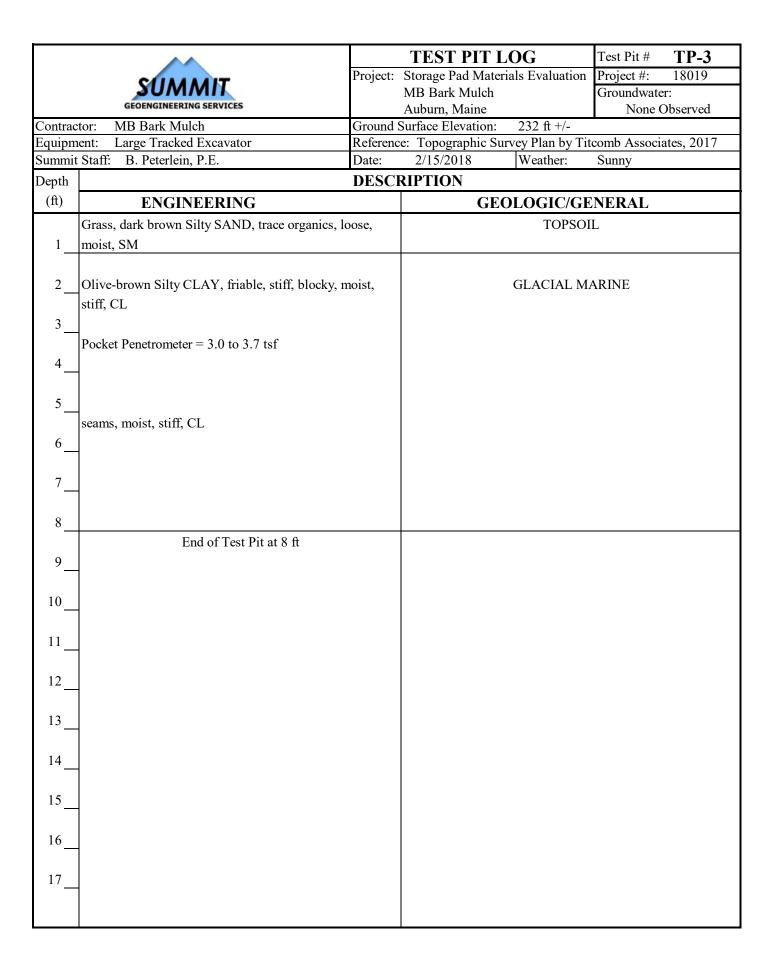


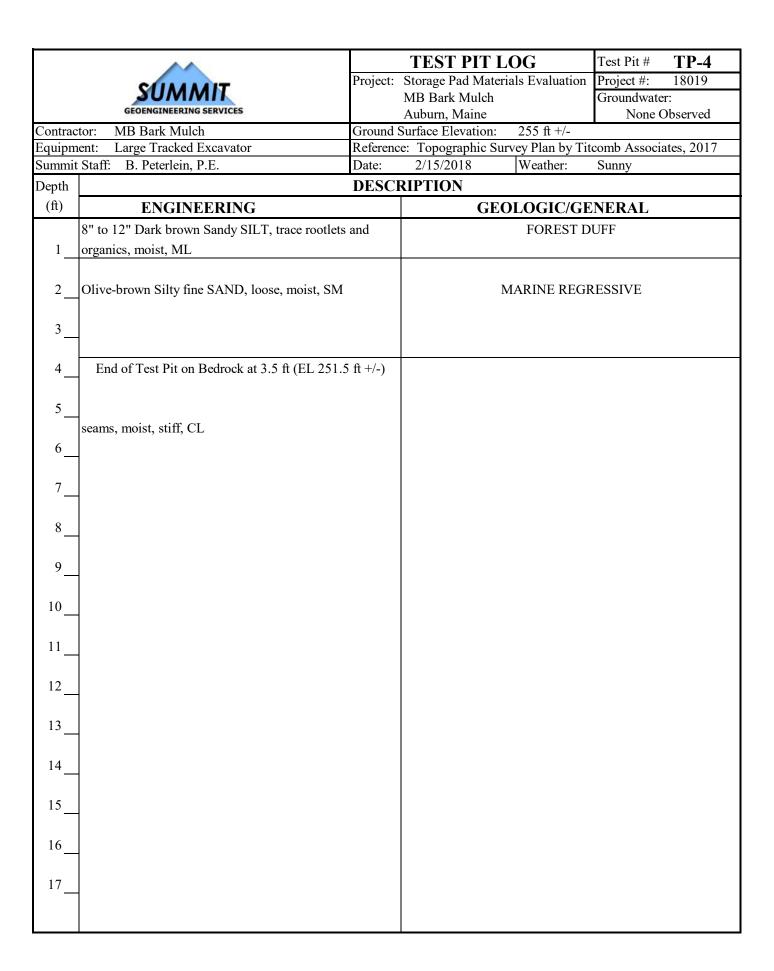


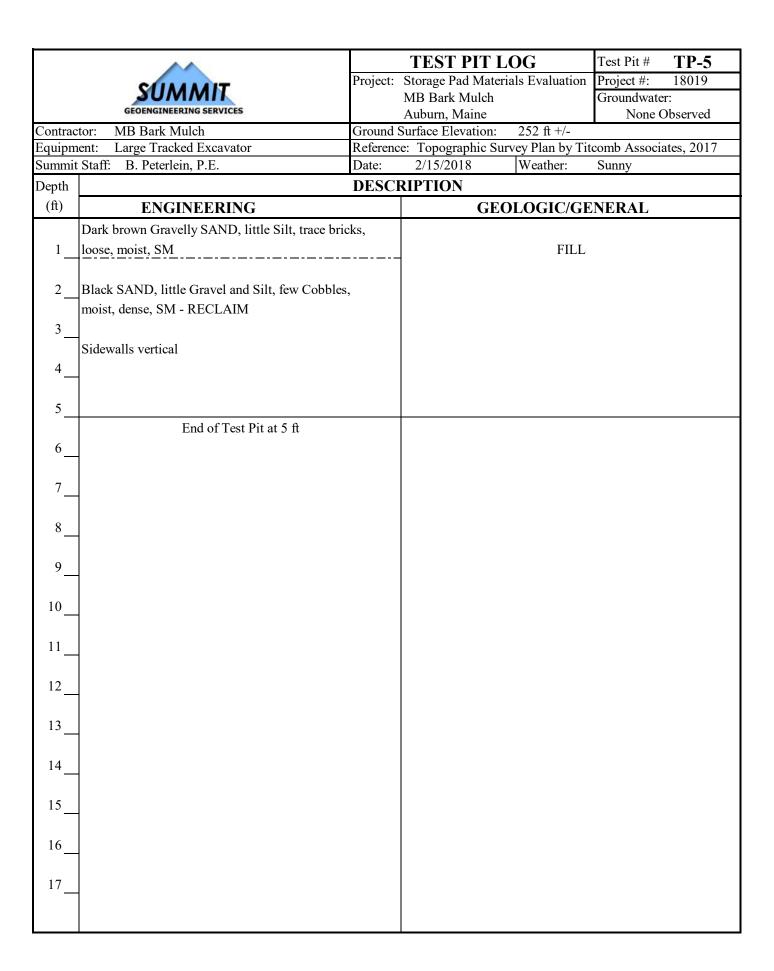
APPENDIX B TEST PIT LOGS

			TEST PIT LOG			TP-1
CHANAIT			Storage Pad Materia	Project #:	18019	
	SUMMII		MB Bark Mulch		Groundwate	r:
	GEOENGINEERING SERVICES		Auburn, Maine		Slow seep	age at 9.5 ft
Contrac			Surface Elevation:	232 ft +/-		
Equipm			e: Topographic Surv			ates, 2017
Summit	Staff: B. Peterlein, P.E.	Date:	2/15/2018	Weather:	Sunny	
Depth		DESCI	RIPTION			
(ft)	ENGINEERING		GEO	LOGIC/GE	NERAL	
	8" dark brown Silty SAND, trace organics, loose,			TOPSOI	L	
1	moist, SM					
2	Olive-gray Silty CLAY, blocky, stiff, CL			GLACIAL MA	ARINE	
3						
4						
5						
	seams, moist, stiff, CL					
6	,,,					
_						
7						
8						
9						
10	Slow groundwater seepage at 9.5 ft (EL 222.5 ft -	+/-)				
	Beomes wet and soft to firm					
11						
12						
12						
13						
14						
15						
	Becomes soft and gray at 15 ft					
16						
	End of Test Pit at 16 ft					
17						

Project:		TEST PIT LOG	Test Pit # TP-2	
		Project:	Storage Pad Materials Evaluation	Project #: 18019
			MB Bark Mulch	Groundwater:
	GEOENGINEERING SERVICES		Auburn, Maine	Slow seepage at 10 ft
Contrac			Surface Elevation: 231 ft +/-	1 4 2017
Equipm Summit		Date:	ee: Topographic Survey Plan by Tit 2/15/2018 Weather:	Sunny
Depth	Stan. B. Peterlein, P.E.		RIPTION	Sumiy
(ft)	ENGINEERING	DESC	GEOLOGIC/GE	ENERAL.
	8" dark brown Silty SAND, trace organics, loose,		TOPSOI	
1	moist, SM	,		
i -	mon, sin			
2_	Olive-gray Silty CLAY, blocky, stiff, CL		GLACIAL M.	ARINE
3				
4	Pocket Penetrometer = 4.0 to 4.5 tsf			
5	seams, moist, stiff, CL			
6				
7				
8				
9				
10	Water seepage slow in silt seams (EL 221 ft +/-)			
11_				
12				
13				
14				
15				
	Pocket Penetrometer = 0.7 to 1.0 tsf			
17				
	End of Test Pit at 17 ft			







^^		TEST PIT LOG		Test Pit #	TP-6	
	CHANAIT		Project: Storage Pad Materials Evaluation		Project #:	18019
	SUMMIT	MB Bark Mulch		Groundwater	:	
	GEOENGINEERING SERVICES		Auburn, Maine		None C	bserved
Contrac			Surface Elevation:	255 ft +/-		
Equipn			e: Topographic Surv			tes, 2017
Summi	t Staff: B. Peterlein, P.E.	Date:	2/15/2018	Weather:	Sunny	
Depth		DESCI	RIPTION			
(ft)	ENGINEERING		GEO	LOGIC/GE	NERAL	
1_	Olive-brown Silty fine SAND, moist, loose, froze	en in	M	ARINE REGR	ESSIVE	
	upper 6 to 8 inches, SM					
2						
3						
4						
5	Olive-brown to olive-gray Silty CLAY, few fine s	silt		GLACIAL MA	ARINE	
	seams, moist, stiff, CL					
6						
7						
8						
_	-					
9						
´—	-					
10						
10_	End of Test Pit at 10 ft					
11	End of Test I it at 10 it					
11_	-					
12						
12_	-					
13						
13_	-					
1.4						
14_	-					
1.5						
15_	-					
16_	-					
17	-					

The key to success starts with a solid foundation. ENGINEERING | EXPLORATION | EXPERIENCE

Pad #1 Construction Recommendations MB Bark Mulch, Auburn, Maine





Client

St. Germain Collins 846 Main Street Westbrook, Maine 04092

> Project #: 18019 Date: 6/18/18

145 Lisbon Street (PO Box 7216) Lewiston, Maine 04243 | (207) 576-3313 173 Pleasant Street Rockland, Maine 04841 | (207) 318-7761 www.summitgeoeng.com



June 19, 2018 SGS #18019

Peter Mohlin St.Germain Collins 846 Main St. Westbrook, ME 04092

Reference: Supplemental Geotechnical Evaluation, Pad #1 Fill Placement and Slope Stability

MB Bark Recycling Facility Expansion, Bark Mulch Drive, Auburn, Maine

Dear Peter;

This report is intended to provide additional material properties information and evaluate the overall stability of the material and site for a C&R and Reclaim fill pad. This report follows a previous report prepared by SGS titled "Geotechnical Evaluation, Proposed Pad #1 Expansion, MB Bark Mulch, Auburn Maine" and dated February 23, 2018. Included in this supplemental report are recommendations for construction of the pad expansion.

Our scope of services for this phase of the project consisted of laboratory testing of the C&R and Reclaim materials and global stability analyses of the proposed embankment.

Beyond the pad construction evaluated in this report, we believe that both the C&R and the Reclaim will be suitable to construct roadways connecting to the future Pads #2 and #3. Additional information on the existing roadway subgrade conditions, not included in our current scope, will be required to determine the most effective roadway subgrade preparation and material types and thicknesses.

1.0 Exploration and Laboratory Testing

A series of test pits were excavated at the site under the direction of SGS on February 15, 2018. A detailed description of the explorations and subsurface conditions are included in the SGS February 2018 report. Logs of the test pits and a test pit location plan are included in Appendix A.

A sample of the *Reclaim* soil was obtained in test pit TP-5 on February 15, 2018. The sample was tested by SGS in our laboratory. The tests included grain size analysis and direct shear. The results of these tests are presented in Appendix B.



Two bucket samples of the C&R were delivered to SGS at the MB Bark mulch site. One sample was free of asphalt shingles and the other sample contained asphalt shingles. These samples were tested by Geotesting Express, Acton, Massachusetts, for grain size analysis and direct shear. The results of these tests are presented in Appendix C.

2.0 Material Description

Reclaim (Stabilized Petroleum Containing Soils) is a waste derived product, manufactured from 2" minus aggregate materials which are pug mill processed utilizing an asphalt emulsifier (Stabilizer Plus®). CRS manufactures two basic types of Reclaim, Virgin Petroleum Contaminated Soils (VPCS), and Non Virgin Petroleum Contaminated Soils (NVPCS). NVPCS includes sandblast grit, bottom ash, catch basin grit, and other soils containing non virgin polycyclic aromatic hydrocarbons (PAHs).

The results of the grain size analysis indicates that the *Reclaim* material consists of 21% gravel, 67% sand, and 12% silt sized particles. The direct shear test indicated a peak friction angle of 36 degrees and an assumed cohesion of 0 psf at a unit weight of 110 pcf.

C&R (Crushed and Recycled) is manufactured from crushed and screened demolition aggregate-based materials. This crusher processed sized material includes brick, concrete, asphalt pavement, stone tailings, ledge and similar aggregate-based demolition materials. Ground shingles may be added at different percentages to enhance the compaction of the crushed materials.

The results of the grain size analyses and direct shear tests on the C&R material are summarized in the following table.

LABORATORY TEST RESULTS C&R MATERIAL						
Material	Grain Size Properties*		s* Direct Shear Properties			
Materiai	Gravel	Sand	Silt	Unit Weight	Phi (degrees)	Cohesion
C&R w/o Shingles	54.8%	40.6%	4.6%	135 pcf (avg)	41.2	432 psf
C&R w/ Shingles	53.9%	40.3%	5.8%	136 pcf (avg)	47.3	285 psf

^{*} Refers to particle sizes only, not composition

3.0 Global Stability Analysis

A global stability analysis was performed using Slide 6.0 computer program. The analysis was performed at a section through the existing pad at the approximate location shown on Figure 1 in Appendix D. The intent of this analysis was to model the stability of the embankment face and



the underlying soil with the embankment fully built. The geometry, soil properties, and lowest safety factor failure circle are shown on the Slide output in Appendix D.

The C&R and Reclaim soil properties were taken from the direct shear tests and used in interchangeably in the analyses. The stiff clay soil properties were based on observations and pocket penetrometer tests taken in the February 2018 test pits. A layer of soft silty clay was encountered at a depth of 15 feet at the TP-1 location. The soft silty clay soil properties were estimated conservatively based on our knowledge and experience. A series of analyses were made with various material properties to ascertain the sensitivity of the model to these properties and the geometry. Included in the variations were the facing slope angles, which ranged from 3H:1V to 1H:1V.

The lowest safety factor for the proposed embankment was computed to be 1.32. A safety factor equal to or greater than 1.3 for this condition is acceptable. This safety factor is based on the assumption that the construction recommendations listed below are followed.

4.0 Pad Construction Recommendations.

We recommend that the pad be constructed in the following manner.

- Grub the area to be filled to remove the topsoil and vegetation. The topsoil should be salvaged for future use. Grubbing should be performed with a smoothed-edge excavator bucket or a dozer to minimize disturbance to the stiff silty clay soil. Excavate into the silty clay soil as little as possible so that the stiff supporting crust it creates is not compromised. The exposed silty clay should be protected from rain until the base material can be placed.
- Place a 6" to 8" layer of ¾ crushed stone OR a 12" to 18" layer of crushed bedrock (8" maximum size) beneath a 4" thick ¾ inch crushed stone choke layer. The crushed stone or rock can be placed directly on the exposed subgrade soil.
- Place the Reclaim or C&R directly on the crushed stone. These materials can be placed in 18" lifts. Each lift should be compacted with a vibratory compactor having a minimum operating weight of 10 tons and making 4 passes in each of two perpendicular directions. Both materials can be placed in their existing condition; no special conditioning is necessary. If the methods described above are used to place the material, no compaction testing will be necessary. We recommend that SGS be contacted so we can observe the initial lifts of material as they are placed to confirm the above methods are effective.



- Both the C&R and Reclaim are susceptible to erosion. Exposed slopes should be protected from runoff during construction by sloping the grade back from the crest at a minimum of 2 degrees.
- If rip rap is used, place rip rap on the face of the slope as indicated in the construction drawings as the embankment is created. If it is desirable to place all the rip rap after the pad is brought up to it final grade, we recommend that the face of the slope be protected from erosion while filling proceeds.

Based on the results of the global stability analysis, the face of the pad can be sloped as steep as 1.5H:1V in lieu of the currently proposed 3H:1V vegetated slope. This will allow for more material to be used in the embankment. If a 1.5H:1V slope is used, rip rap protection will be required on the facing to protect it from slumping and surface erosion. Details of the rip rap have been provided by others in the Site Development Plan Permit Set dated February 2018.

5.0 Closure

Our recommendations are based on professional judgment and generally accepted principles of geotechnical engineering and project and subsurface information provided by others. Some changes in subsurface conditions or material properties from those presented in this report may occur. Should these conditions differ from those described in this report, SGS should be notified so that we can re-evaluate our recommendations.

Thank you once again for this opportunity. If there are any questions, please do not hesitate to contact me.

Sincerely,

Summit Geoengineering Services, Inc.

Who matel

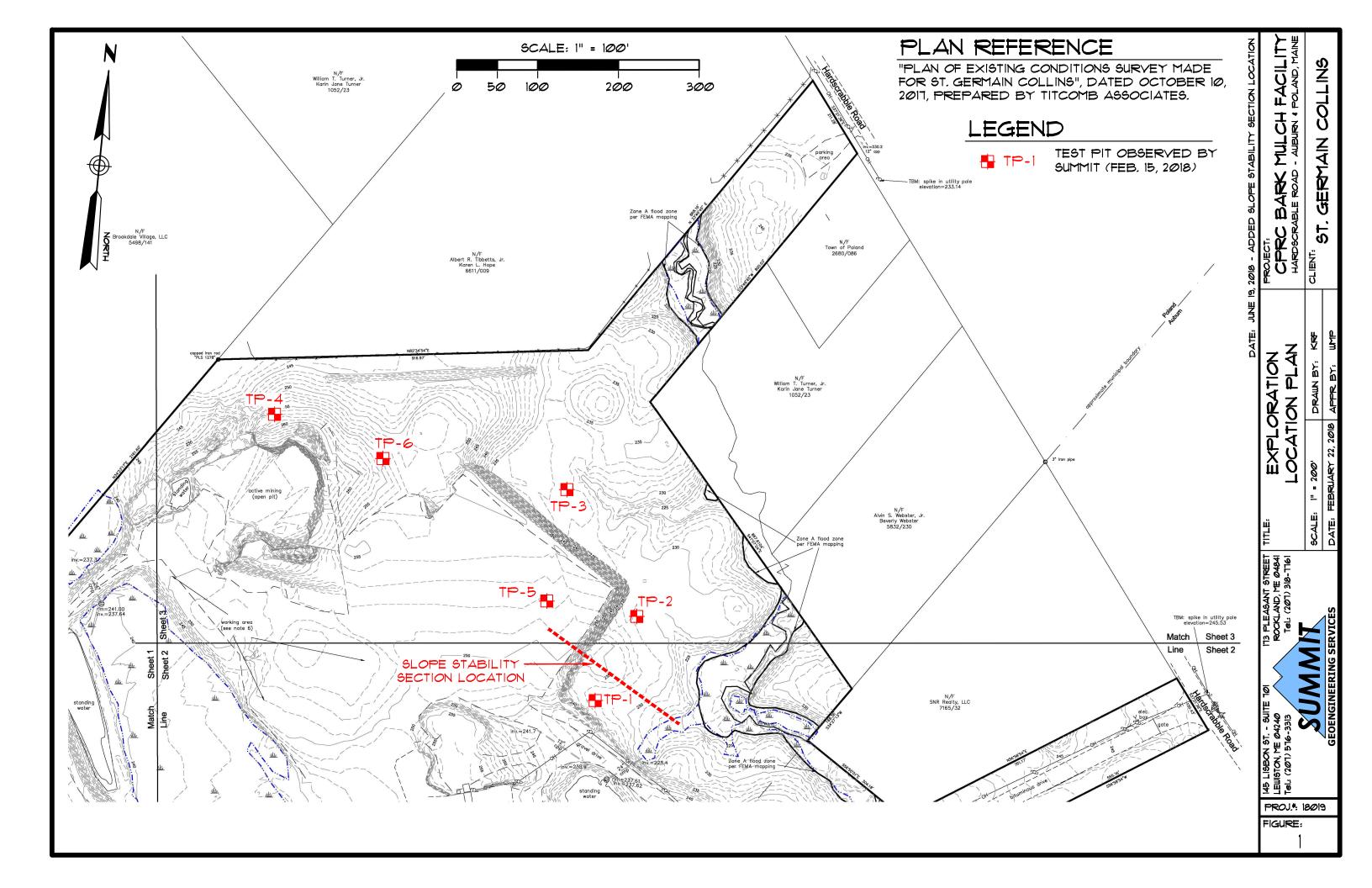
William M. Peterlein, P.E.

President & Principal Engineer



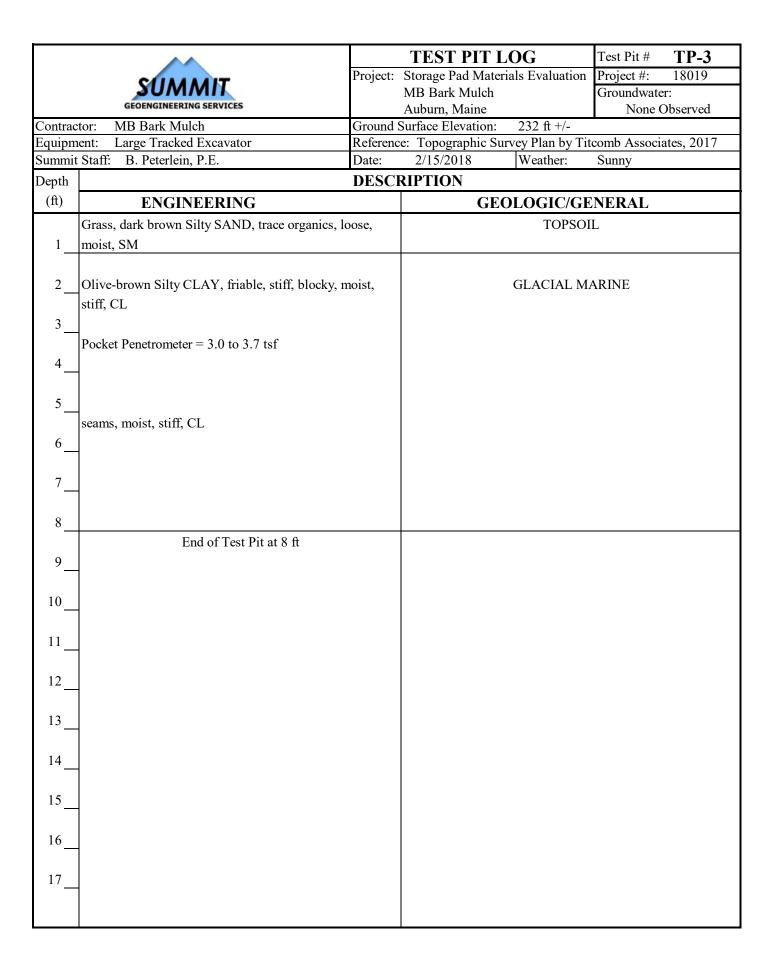
APPENDIX A

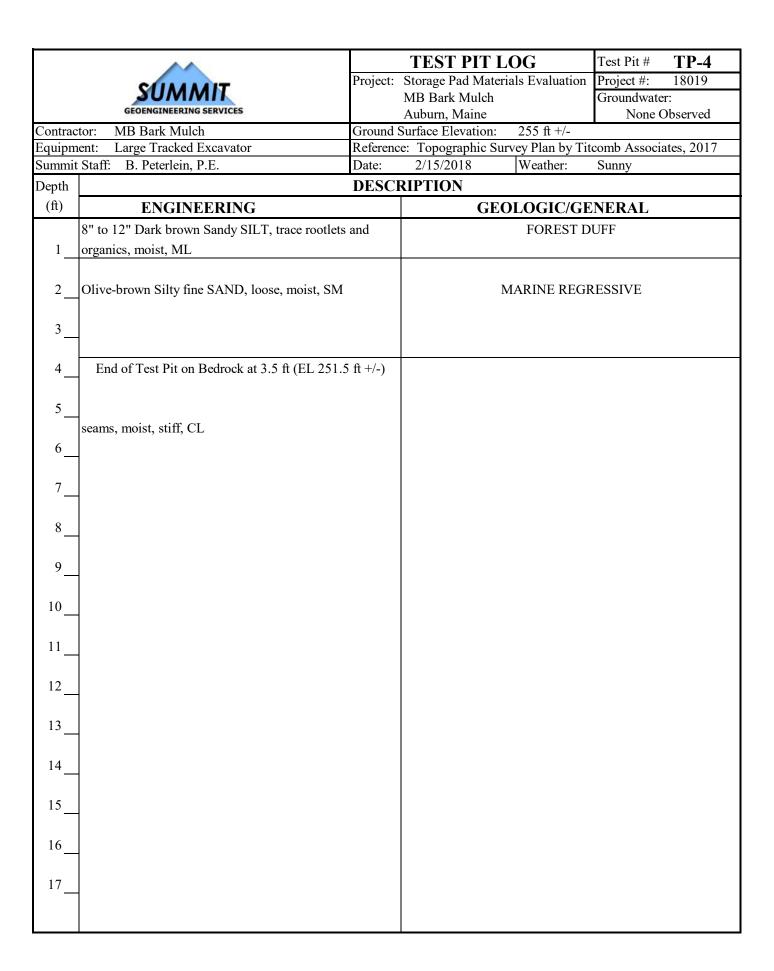
EXPLORATION LOCATION PLAN AND TEST PIT LOGS

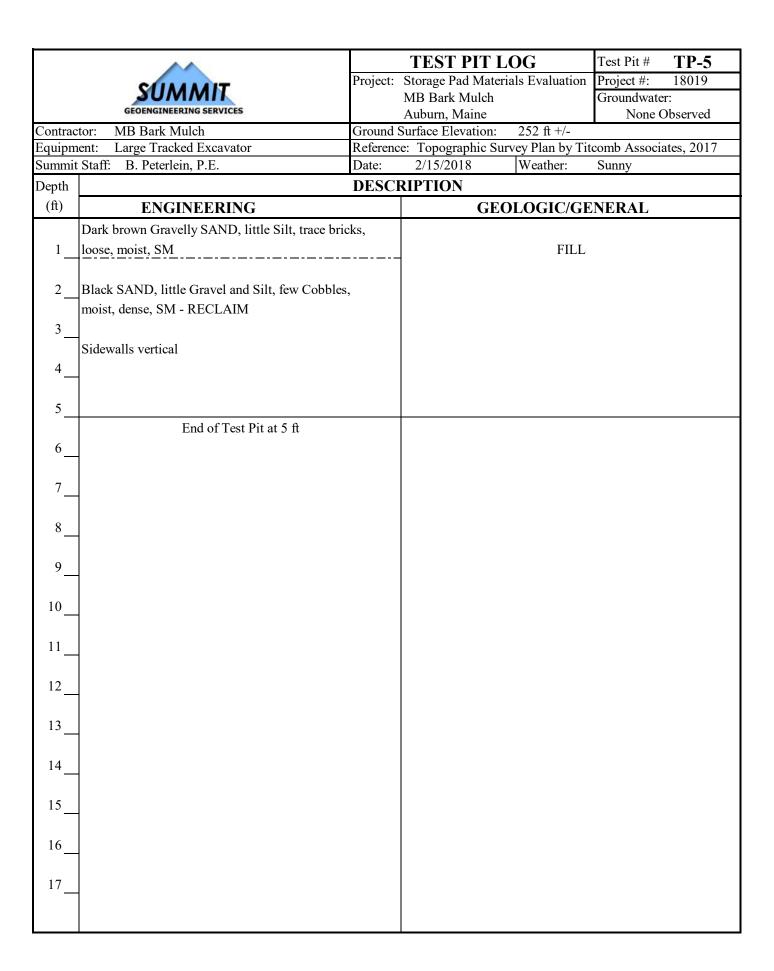


			TEST PIT LOG			TP-1
CHANAIT			Storage Pad Materia	Project #:	18019	
	SUMMII		MB Bark Mulch		Groundwate	r:
	GEOENGINEERING SERVICES		Auburn, Maine		Slow seep	age at 9.5 ft
Contrac			Surface Elevation:	232 ft +/-		
Equipm			e: Topographic Surv			ates, 2017
Summit	Staff: B. Peterlein, P.E.	Date:	2/15/2018	Weather:	Sunny	
Depth		DESCI	RIPTION			
(ft)	ENGINEERING		GEO	LOGIC/GE	NERAL	
	8" dark brown Silty SAND, trace organics, loose,			TOPSOI	L	
1	moist, SM					
2	Olive-gray Silty CLAY, blocky, stiff, CL			GLACIAL MA	ARINE	
3						
4						
5						
	seams, moist, stiff, CL					
6	,,,					
_						
7						
8						
9						
10	Slow groundwater seepage at 9.5 ft (EL 222.5 ft -	+/-)				
	Beomes wet and soft to firm					
11						
12						
12						
13						
14						
15						
	Becomes soft and gray at 15 ft					
16						
	End of Test Pit at 16 ft					
17						

Project:		TEST PIT LOG	Test Pit # TP-2	
		Project:	Storage Pad Materials Evaluation	Project #: 18019
			MB Bark Mulch	Groundwater:
	GEOENGINEERING SERVICES		Auburn, Maine	Slow seepage at 10 ft
Contrac			Surface Elevation: 231 ft +/-	1 4 2017
Equipm Summit		Date:	ee: Topographic Survey Plan by Tit 2/15/2018 Weather:	Sunny
Depth	Stan. B. Peterlein, P.E.		RIPTION	Sumiy
(ft)	ENGINEERING	DESC	GEOLOGIC/GE	ENERAL.
	8" dark brown Silty SAND, trace organics, loose,		TOPSOI	
1	moist, SM	,		
i -	mon, sin			
2_	Olive-gray Silty CLAY, blocky, stiff, CL		GLACIAL M.	ARINE
3				
4	Pocket Penetrometer = 4.0 to 4.5 tsf			
5	seams, moist, stiff, CL			
6				
7				
8				
9				
10	Water seepage slow in silt seams (EL 221 ft +/-)			
11_				
12				
13				
14				
15				
	Pocket Penetrometer = 0.7 to 1.0 tsf			
17				
	End of Test Pit at 17 ft			







^^		TEST PIT LOG		Test Pit #	TP-6	
	CHANAIT		Project: Storage Pad Materials Evaluation		Project #:	18019
	SUMMIT	MB Bark Mulch		Groundwater	:	
	GEOENGINEERING SERVICES		Auburn, Maine		None C	bserved
Contrac			Surface Elevation:	255 ft +/-		
Equipn			e: Topographic Surv			tes, 2017
Summi	t Staff: B. Peterlein, P.E.	Date:	2/15/2018	Weather:	Sunny	
Depth		DESCI	RIPTION			
(ft)	ENGINEERING		GEO	LOGIC/GE	NERAL	
1_	Olive-brown Silty fine SAND, moist, loose, froze	en in	M	ARINE REGR	ESSIVE	
	upper 6 to 8 inches, SM					
2						
3						
4						
5	Olive-brown to olive-gray Silty CLAY, few fine s	silt		GLACIAL MA	ARINE	
	seams, moist, stiff, CL					
6						
7						
8						
_	-					
9						
´—	-					
10						
10_	End of Test Pit at 10 ft					
11	End of Test I it at 10 it					
11_	-					
12						
12_	-					
13						
13_	-					
1.4						
14_	-					
1.5						
15_	-					
16_	-					
17	-					



APPENDIX B

RECLAIM MATERIAL LABORATORY TEST RESULTS



GRAIN SIZE ANALYSIS - ASTM D6913

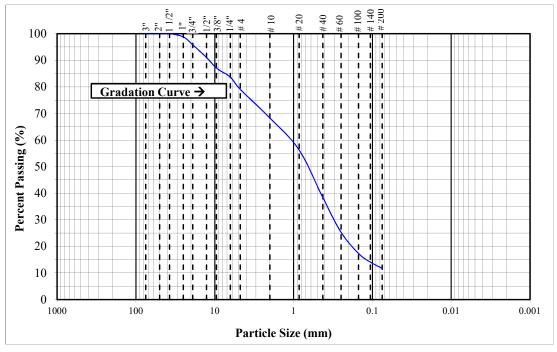
PROJECT NAME: MB Bark Mulch Storage Pads PROJECT #: 18019 TP-5 PROJECT LOCATION: Bark Mulch Drive, Auburn, Maine EXPLORATION #: CLIENT: St. Germain Collins SAMPLE #: S-1 TECHNICIAN: Erika Stewart, P.E. SAMPLE DEPTH: 4 ft SOIL DESCRIPTION: SAND, some Gravel, little Silt, SW-SM to SM TEST DATE: 3/5/2018

TEST PROCEDURE

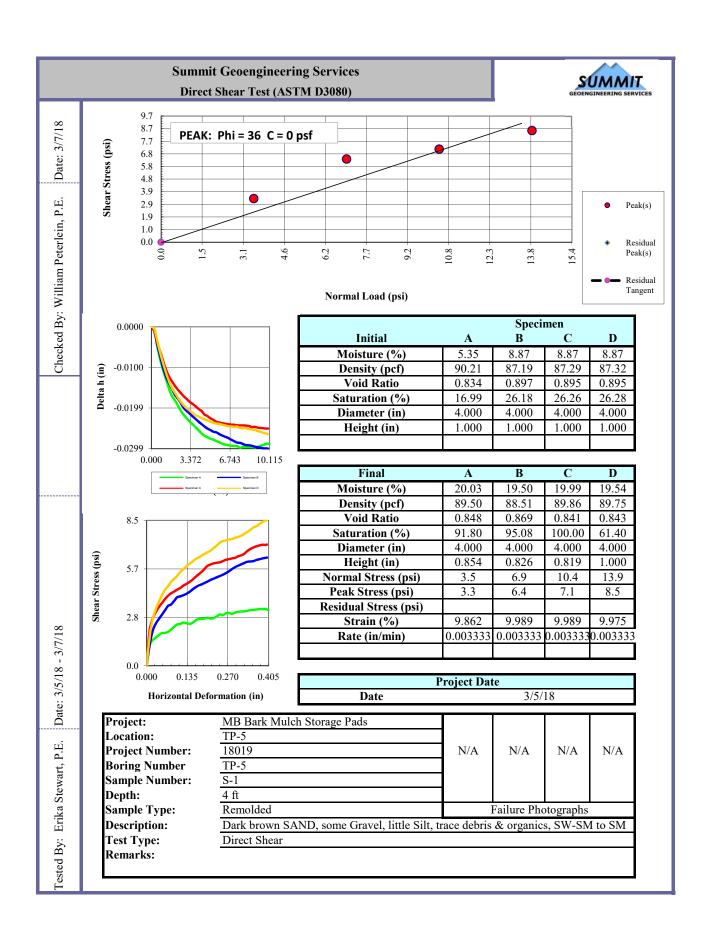
Sample Source: Test Pit	Sieve Stack: Composite	Specimen Procedure: Moist
Test Method: Method A	Separating Sieve(s): 3/8 Inch	Dispersion Type: Tap Water

DATA

STANDARD SIEVE DESIGNATION (mm)	ALTERNATIVE SIEVE DESIGNATION (in)	PERCENT PASSING (%)
75	(3 in)	100
50	(2 in)	100
37.5	(1-1/2 in)	100
25.0	(1 in)	99
19.0	(3/4 in)	96
12.7	(1/2 in)	91
9.5	(3/8 in)	87
6.35	(1/4 in)	84
4.75	(No. 4)	79
2.00	(No. 10)	68
0.850	(No. 20)	56
0.425	(No. 40)	38
0.250	(No. 60)	25
0.150	(No. 100)	17
0.106	(No. 140)	14
0.075	(No. 200)	12



REMARKS: Moisture Content = 8.0%. Sample contains trace organics, brick, glass, and soil conglomerates from chemical treatment. Trace soil particles floating in wash water due to chemical coating.





APPENDIX C

C&R MATERIAL LABORATORY TEST RESULTS



Client: Summit Geoengineering Services

Project: MB Bark Mulch

Location:Auburn, MEProject No:GTX-308107Boring ID:CPRCSample Type: bucketTested By: jbr

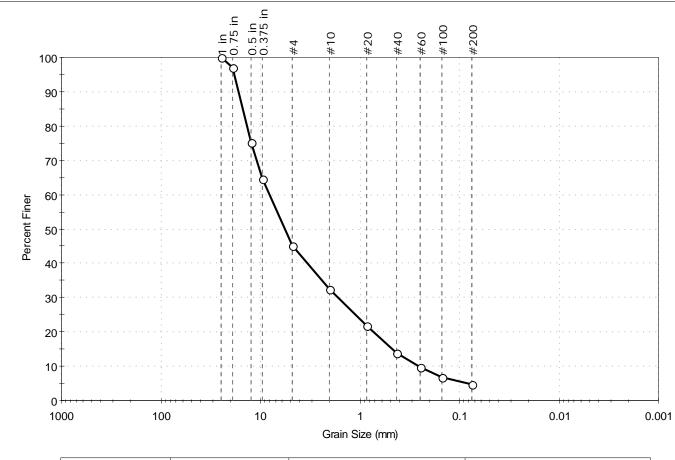
Boring ID: CPRC Sample Type: bucket Tested By: jbr Sample ID: No Shingles Test Date: 06/05/18 Checked By: emm

Depth: Grab Test Id: 453697

Test Comment: --Visual Description: Moist, dark brown gravel with sand

Sample Comment: Sample contains asphalt

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
	54.8	40.6	4.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	97		
0.5 in	12.50	75		
0.375 in	9.50	65		
#4	4.75	45		
#10	2.00	33		
#20	0.85	22		
#40	0.42	14		
#60	0.25	10		
#100	0.15	7		
#200	0.075	4.6		

<u>Coefficients</u>				
$D_{85} = 15.0606 \text{ mm}$	$D_{30} = 1.6258 \text{ mm}$			
$D_{60} = 8.0608 \text{ mm}$	$D_{15} = 0.4634 \text{ mm}$			
$D_{50} = 5.6312 \text{ mm}$	$D_{10} = 0.2558 \text{ mm}$			
$C_{11} = 31.512$	$C_c = 1.282$			

Classification

ASTM Well-graded GRAVEL with Sand (GW)

AASHTO Stone Fragments, Gravel and Sand (A-1-a (1))

<u>Sample/Test Description</u> Sand/Gravel Particle Shape : ANGULAR

Sand/Gravel Hardness: HARD



Client: Summit Geoengineering Services

Project: MB Bark Mulch

Test Comment:

Location: Auburn, ME Project No:

Boring ID: CPRC Sample Type: bucket Tested By: jbr

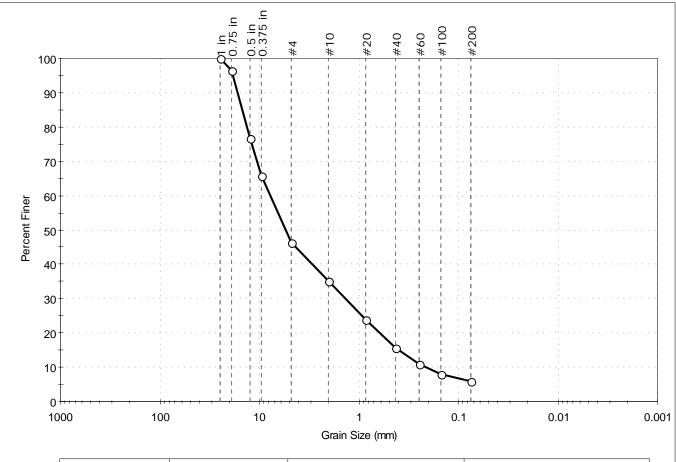
Boring ID: CPRC Sample Type: bucket Tested By: jbr Sample ID: With Shingles Test Date: 06/05/18 Checked By: emm

Depth: Grab Test Id: 453698

Visual Description: Moist, dark grayish brown gravel with silt and sand

Sample Comment: Sample contains asphalt

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
	53.9	40.3	5.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	96		
0.5 in	12.50	77		
0.375 in	9.50	66		
#4	4.75	46		
#10	2.00	35		
#20	0.85	24		
#40	0.42	16		
#60	0.25	11		
#100	0.15	8		
#200	0.075	5.8		

<u>Coefficients</u>				
D ₈₅ = 14.8953 mm	$D_{30} = 1.3539 \text{ mm}$			
$D_{60} = 7.7209 \text{ mm}$	$D_{15} = 0.3948 \text{ mm}$			
$D_{50} = 5.4344 \text{ mm}$	$D_{10} = 0.2117 \text{ mm}$			
$C_{11} = 36.471$	$C_{c} = 1.121$			

GTX-308107

<u>Classification</u> <u>ASTM</u> N/A

AASHTO Stone Fragments, Gravel and Sand (A-1-a (1))

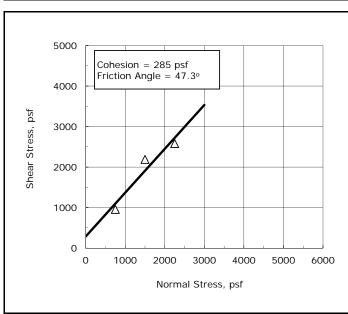
<u>Sample/Test Description</u> Sand/Gravel Particle Shape : ANGULAR

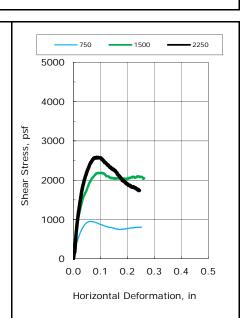
Sand/Gravel Hardness: HARD



Client:	Summit Geoengineering Services	
Project Name:	MB Bark Mulch	
Project Location:	Auburn, ME	
GTX #:	308107	
Test Date:	05/31/18	
Tested By:	md	
Checked By:	njh	
Boring ID:	CPRC	
Sample ID:	No Shingles	
Depth, ft:	Grab	
Visual Description: Moist, dark brown gravel with sand		

Direct Shear Test of Soils Under Consolidated Drained Conditions by ASTM D3080





Test No.:	DS-4	DS-5	DS-6
Initial Diameter, in:	2.5	2.5	2.5
Initial Height, in:	1.0	1.0	1.0
Initial Mass, grams:	153	153	153
Initial Dry Density, pcf:	109.9	109.9	109.9
Initial Moisture Content, %:	8.4	8.4	8.4
Initial Bulk Density, pcf:	119.1	119.1	119.1
Initial Degree of Saturation:	43.8	43.8	43.8
Initial Void Ratio:	0.51	0.51	0.51
Final Dry Density, pcf:	110.2	108.6	109.9
Final Moisture Content, %:	25.1	22.9	23.7
Final Bulk Density, pcf:	137.8	133.5	136.0
Normal Stress, psf:	750	1500	2250
Maximum Shear Stress, psf:	956	2190	2580
Shear Rate, in/min:	0.001	0.001	0.001
·			

	_	— 750 —— 1500 —— 2250
	-0.015	
	-0.010	
	-0.005	
Ë	0.000	
tion	0.005	
rma	0.010	
efo	0.015	
a	0.020	
Vertical Deformation, in	0.025	
Š	0.030	
	0.035	
	0.040	
	(0.0 0.1 0.2 0.3 0.4 0.5
	F	Horizontal Deformation, in

Sample Type:	reconstituted	
Estimated Specific Gravity:	2.65	
Liquid Limit:		
Plastic Limit:		
Plasticity Index:		
% Passing #200 sieve:	4.6	
Soil Classification:		
Group Symbol:		

Notes: Material greater than #5 sieve screened out of sample prior to testing

Moisture content obtained before shear from sample trimmings

Moisture Content determined by ASTM D2216

Percent passing #200 sieve determined by ASTM D422

Target Compaction: 90% of the maximum dry density (122.5 pcf) at the optimum moisture content (8.0%). Values specified by client.

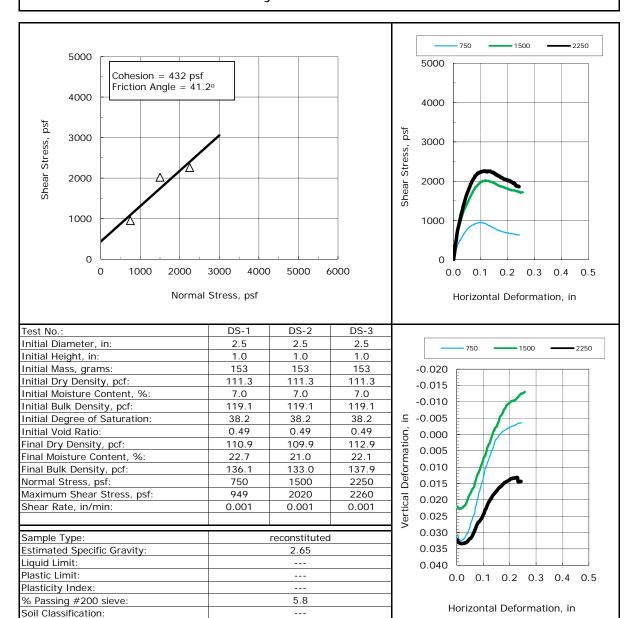
Values for cohesion and friction angle determined from best-fit straight line to the data for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site-specific conditions.

"---" indicates testing required to determine these values was not requested.



Client: Summit Geoengineering Services	
Project Name:	MB Bark Mulch
Project Location:	Auburn, ME
GTX #:	308107
Test Date:	05/26/18
Tested By:	md
Checked By:	njh
Boring ID:	CPRC
Sample ID:	With Shingles
Depth, ft:	Grab
Visual Description:	Moist, dark grayish brown gravel with silt and sand

Direct Shear Test of Soils Under Consolidated Drained Conditions by ASTM D3080



Notes: Material greater than #5 sieve screened out of sample prior to testing

Moisture content obtained before shear from sample trimmings

Moisture Content determined by ASTM D2216

Group Symbol:

Percent passing #200 sieve determined by ASTM D422

Target Compaction: 90% of the maximum dry density (122.5 pcf) at the optimum moisture content (8.0%). Values specified by client.

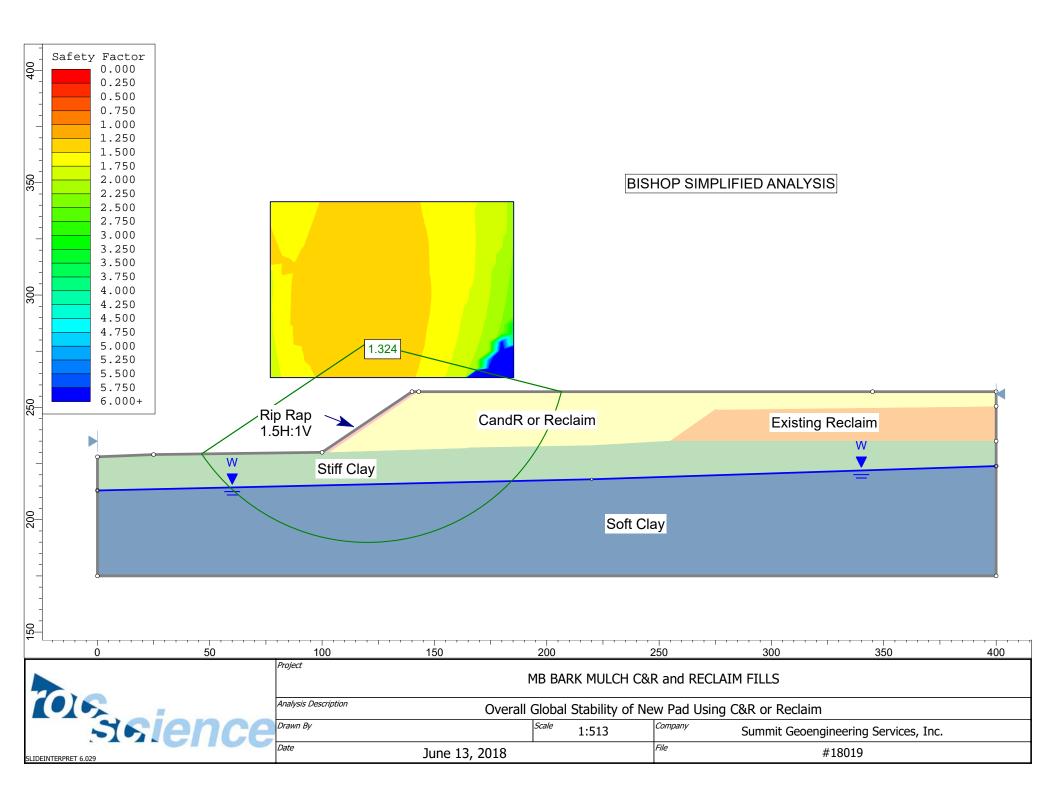
Values for cohesion and friction angle determined from best-fit straight line to the data for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site-specific conditions.

"---" indicates testing required to determine these values was not requested.



APPENDIX D

SLOPE STABILITY ANALYSIS



Slide Analysis Information MB Bark Mulch Embankment Stability Auburn ME

Project Summary

File Name: Proposed slope with C&R 1H to 1V.slim

Last saved with Slide version: 6.029

Project Title: MB Bark Mulch Embankment Stability Auburn ME

Analysis: Reclaim Stability

Date Created: 5/30/2018, 2:46:23 PM

General Settings

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

Analysis Options

Analysis Methods Used

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



MB BARK MULCH C&R and RECLAIM FILLS

Analysis Description

Overall Global Stability of ENw Pad Using C&R or Reclaim

Scale Summit Geoengineering Services, Inc.

#18019

File Name June 13, 2018

Surface Options

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

Material Properties

Property	C&R	Stiff Clay	Existing Reclaim	Soft Clay	Rip rap
Color					
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Undrained	Mohr-Coulomb
Unit Weight [lbs/ft3]	135	120	125	120	115
Cohesion [psf]	0	1500	0		0
Friction Angle [deg]	41	0	36		50
Cohesion Type				Function Of Depth	
Cohesion (Top) [psf]				520	
Cohesion Change [psf/ft]				12	
Water Surface	None	None	None	None	None
Ru Value	0	0	0	0	0

Probabilistic Analysis Input

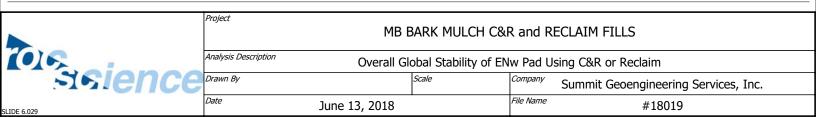
General Settings

Sensitivity Analysis: On Probabilistic Analysis: Off

Variables

Material	Property	Distribution	Mean	Min	Max
Soft Clay	Cohesion	Normal	520	520	1520

List Of Coordinates



External Boundary

Х	Υ
400	175
400	223.868
400	235
400	250.5
400	257
345	257
143	257
140	257
100	230
25	229
0	228
0	213
0	175

Material Boundary

Х	Y
103.529	230.109
165	232
220	233
255	235
275	249
400	250.5

Material Boundary

X	Υ
0	213
220	218
400	223.868

Material Boundary

Х	Υ
255	235
400	235



MB BARK MULCH C&R and RECLAIM F	FILLS

Analysis Description Overall Global Stability of ENw Pad Using C&R or Reclaim

Drawn By

| Scale | Company | Summit Geoengineering Services, Inc. |

Date June 13, 2018 File Name #18019

Application for Site Plan Review Town of Poland Planning Board 100 Bark Mulch Drive, Auburn, Maine 04240 St.Germain Collins File No.: 2265-0002 January 2019 Attachment 5

Page 1

Financial Capacity

Construction of the site expansion will be conducted primarily by MB Bark personnel in three phases. The first phase will include extending Pad 1. Phase two will consist of tree clearing, road and Pad 3 construction in the undeveloped portion of the site. The third phase will include the construction of Pad 2. See sheet C-3.1 – Construction Phasing Plan.

When the third phase is complete, MB Bark will be able to increase operating capacity to accommodate the asphalt shingle processing operation from Scarborough and conduct current site activities simultaneously.

Project Aspect	Estimate
Wetland Impact Compensation Fee	\$101,000
Construction	
Labor @50\$/hour = 4,000 hours/4 employees = 100 hours per employee over 2 years.	\$200,000
Tree Clearing	\$30,000
Blasting	\$12,000
Materials (manholes, piping and box culverts)	\$45,000
Construction Oversight	\$15,000
Total	\$403,000

The majority of the cost will be internal as the project's construction materials and labor will be provided by MB Bark. The applicant will fund the project out of operational cash flow, and the letter in Attachment 5 indicates the ability to fund such costs.



To Whom It May Concern,

Please accept this letter as conformation that our client CPRC Group/ MB Bark has the financial wherewithal to undertake the MB Bark Recycling Permit Project in Poland/Auburn ME. Between its own cash flow generation and its access to 2.5 million dollars of working capital, CPRC/MB Bark has plenty of available capital to undertake this project.

Please contact me with any further questions.

Sincerely,

Jason Lundy

Senior Vice President, Senior Relationship Manager

Business Banking

Bank of America Merrill Lynch

Bank of America, N.A.

1 Monument Square, 9th Floor

ME9-001-09-01, Portland, Maine 04101

Office 207.253.7444, Mobile 207.317.1821

Application for Site Plan Review Town of Poland Planning Board 100 Bark Mulch Drive, Auburn, Maine 04240 St.Germain Collins File No.: 2265-0002 January 2019 Attachment 6 Page 1

Traffic

No new entrances or exits are proposed. Vehicles accessing the expansion areas will enter through the existing MB Bark facility via controlled access. Two 10 mph gravel surface access drives connecting to existing internal roadways are proposed in the westerly portion of the facility. The gravel drives will be 24-feet wide to allow for two lane traffic and will be maintained on an as needed basis by onsite equipment. See sheet C-3.0 - Overall Site Plan for drive details, including traffic circulation patterns.

The average number of vehicle trips (incoming or outgoing) is expected to be 68 per day after the expansion, and peak hour is estimated to be 22 trips per hour. No traffic accidents occurred on roads or intersections within 0.25 miles of the facility entrance within the last three years. Maine Department of Transportation traffic counts and crash summary reports are provided in Attachment 6.

MAINE DEPARTMENT OF TRANSPORTATION TRAFFIC ENGINEERING DIVISION TRAFFIC MONITORING SECTION

Town 230101017807 Auburn Мар U/1B Sta.

County Town # 01010 Androscoggin Location W HARDSCRABBLE RD W/O LEWISTON JCT RD County # 01

Start Date 09/18/17	Counter #	596
---------------------	-----------	-----

Start Date	09/18/17			Counter #	596				
Date	09/17/17	09/18/17	09/19/17			09/22/17			
Time	Sunday	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	average	
0:00			19					19	
1:00			14					14	
2:00			11					11	
3:00			21					21	
4:00			28					28	
5:00			107					107	
6:00			124					124	
7:00			169					169	
8:00			150					150	
9:00			143					143	
10:00		107	127					117	
11:00		99	122					111	
12:00		152	128					140	
13:00		122	130					126	
14:00		182						182	
15:00		137						137	
16:00		168						168	
17:00		138						138	
18:00		117						117	
19:00		63						63	
20:00		37						37	
21:00		30						30	
22:00		20						20	
23:00		17						17	
Total	0	1389	1293	0	0	0	0	2189	

AM Peak 107 169

PM Peak 182 130

Peak hour count 182 **DHV Factor** 1.04

DHV % of AADT = 0.10 10%

Week # 38 **Old Group** I **New Group** Ī **Factor** 0.87 2017 **AADT** 1900 28 Duration

Maine Department Of Transportation - Traffic Engineering, Crash Records Section

Crash Summary Report

		Re	eport Selections a	and Input Pa	rameters		
REPORT SELECTIONS							
✓ Crash Summary I	Section De	etail	✓ Crash Summa	ary II	1320 Public	☐1320 Private	☐1320 Summary
REPORT DESCRIPTION							
W Hardscrabble Rd in Auburn	and Poland						
REPORT PARAMETERS							
Year 2014, Start Month 1 thro	ugh Year 2016	End Month: 12					
Route: 0100766	Start Node:	1274	Start C	Offset: 0		☐ Exclude First No	ode
	End Node:	1276	End C	Offset: 0		☐ Exclude Last No	ode
Route: 0110511	Start Node:	1276	Start C	Offset: 0		✓ Exclude First No	ode
	End Node:	4039	End C	Offset: 0		☐ Exclude Last No	ode

				Nodes										
Node	Route - MP	Node Description	U/R	Total		Injur	y Cras	shes		Percent	Annual M C	rash Rate	Critical	CRF
				Crashes	Κ	Α	В	С	PD	Injury	Ent-Veh	raon rate	Rate	Orti
1274	0100766 - 1.60	Int of HARDSCRABBLE RD LEVINE RD	1	0	0	0	0	0	0	0.0	0.596 Statew	0.00 vide Crash Rate	0.54 e: 0.13	0.00
1276	0100766 - 2.24	TL Auburn Poland	1	0	0	0	0	0	0	0.0	0.741 Statew	0.00 vide Crash Rate	0.53 e: 0.13	0.00
4039	0110511 - 0.68	Int of LEWISTON JUNCTION RD W HARDSCRABBLE RD	1	2	0	0	0	2	0	100.0	1.732 Statew	0.39 vide Crash Rate	0.44 e: 0.13	0.00
Study Y	ears: 3.00	NODE TOTAL	S:	2	0	0	0	2	0	100.0	3.069	0.22	0.38	0.57

							Sect	ions									
Start	End	Element	Offset	Route - MP	Section	U/F	R Total		Inju	iry Cr	ashes		Percent	Annual	Crash Rate	Critical	CRF
Node	Node		Begin - End		Length		Crashes	K	Α	В	С	PD	Injury	HMVM		Rate	
1274 Int of HARI	1276 DSCRABBI	169060 LE RD LEVIN	0 - 0.64 NE RD	0100766 - 1.60 RD INV 01 00766	0.64	1	1	0	0	0	0	1	0.0	0.00462	72.18 Statewide Crash F	521.04 Rate: 227.23	0.00
1276 TL Auburr		4333014	0 - 0.68	0110511 - 0 RD INV 01 10511	0.68	1	1	0	0	0	0	1	0.0	0.00517	64.54 Statewide Crash F	506.91 Rate: 227.23	0.00
Study Ye	ears: 3	3.00		Section Totals:	1.32		2	0	0	0	0	2	0.0	0.00978	68.14	436.85	0.16
				Grand Totals:	1.32		4	0	0	0	2	2	50.0	0.00978	136.29	472.52	0.29

Maine Department Of Transportation - Traffic Engineering, Crash Records Section

Crash Summary

	Section Details													
Start	End	Element	Offset	Route - MP	Total	•	Inju	ry Cr	ashes		Crash Report	Crash Date	Crash	Injury
Node	Node		Begin - End		Crashes	K	Α	В	С	PD			Mile Point	Degree
1274	1276	169060	0 - 0.64	0100766 - 1.60	1	0	0	0	0	1	2015-50745	12/07/2015	1.75	PD
1276	4039	4333014	0 - 0.68	0110511 - 0	1	0	0	0	0	1	2016-14720	05/31/2016	0.56	PD
				Totals:	2	0	0	0	0	2				

Crashes by Day and Hour																										
						AM					H	Hour c	f Day						PM							
Day Of Week	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	Un	Tot
SUNDAY	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
MONDAY	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
TUESDAY	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
WEDNESDAY	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
THURSDAY	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
FRIDAY	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SATURDAY	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Totals	0	0	0	0	0	0	1	0	2	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	4

			Vehicle Counts	ь by Туре
Unit Type	Total		Unit Type	Total
1-Passenger Car	3	23-Bicyclist		0
2-(Sport) Utility Vehicle	4	24-Witness		0
3-Passenger Van	0	25-Other		0
4-Cargo Van (10K lbs or Less)	0	Total		7
5-Pickup	0			•
6-Motor Home	0			
7-School Bus	0			
8-Transit Bus	0			
9-Motor Coach	0			
10-Other Bus	0			
11-Motorcycle	0			
12-Moped	0			
13-Low Speed Vehicle	0			
14-Autocycle	0			
15-Experimental	0			
16-Other Light Trucks (10,000 lbs or Less)	0			
17-Medium/Heavy Trucks (More than 10,000 lbs)	0			
18-ATV - (4 wheel)	0			
20-ATV - (2 wheel)	0			
21-Snowmobile	0			
22-Pedestrian	0			

Crash Summary II - Characteristics

Crashes by Driv	er Ac	tion at	Time	of Cra	sh		
Driver Action at Time of Crash	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Total
No Contributing Action	1	2	1	0	0	0	4
Ran Off Roadway	1	0	0	0	0	0	1
Failed to Yield Right-of-Way	1	0	0	0	0	0	1
Ran Red Light	0	0	0	0	0	0	0
Ran Stop Sign	0	0	0	0	0	0	0
Disregarded Other Traffic Sign	0	0	0	0	0	0	0
Disregarded Other Road Markings	0	0	0	0	0	0	0
Exceeded Posted Speed Limit	0	0	0	0	0	0	0
Drove Too Fast For Conditions	1	0	0	0	0	0	1
Improper Turn	0	0	0	0	0	0	0
Improper Backing	0	0	0	0	0	0	0
Improper Passing	0	0	0	0	0	0	0
Wrong Way	0	0	0	0	0	0	0
Followed Too Closely	0	0	0	0	0	0	0
Failed to Keep in Proper Lane	0	0	0	0	0	0	0
Operated Motor Vehicle in Erratic, Reckless, Careless, Negligent or Aggressive Manner	0	0	0	0	0	0	0
Swerved or Avoided Due to Wind, Slippery Surface, Motor Vehicle, Object, Non-Motorist in Roadway	0	0	0	0	0	0	0
Over-Correcting/Over-Steering	0	0	0	0	0	0	0
Other Contributing Action	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0
Total	4	2	1	0	0	0	7

Crashes by Appare	nt Phys	sical C	onditi	on An	d Driv	er	
Apparent Physical Condition	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Total
Apparently Normal	4	2	1	0	0	0	7
Physically Impaired or Handicapped	0	0	0	0	0	0	0
Emotional(Depressed, Angry, Disturbed, etc.)	0	0	0	0	0	0	0
III (Sick)	0	0	0	0	0	0	0
Asleep or Fatigued	0	0	0	0	0	0	0
Under the Influence of Medications/Drugs/Alcohol	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0
Total	4	2	1	0	0	0	7

		Drive	r Age by Uni	t Type		
Age	Driver	Bicycle	SnowMobile	Pedestrian	ATV	Total
09-Under	0	0	0	0	0	0
10-14	0	0	0	0	0	0
15-19	0	0	0	0	0	0
20-24	0	0	0	0	0	0
25-29	1	0	0	0	0	1
30-39	2	0	0	0	0	2
40-49	3	0	0	0	0	3
50-59	0	0	0	0	0	0
60-69	1	0	0	0	0	1
70-79	0	0	0	0	0	0
80-Over	0	0	0	0	0	0
Unknown	0	0	0	0	0	0
Total	7	0	0	0	0	7

Crash Summary II - Characteristics

Total

0

0

0

0

7

Most Harmful Event

38-Other Fixed Object (wall, building, tunnel, etc.)

40-Gate or Cable

41-Pressure Ridge

	Most Har	mful Event
Most Harmful Event	Total	mar =vom
1-Overturn / Rollover	0	38-Other Fix
2-Fire / Explosion	0	39-Unknown
3-Immersion	0	40-Gate or C
4-Jackknife	0	41-Pressure
5-Cargo / Equipment Loss Or Shift	0	Total
6-Fell / Jumped from Motor Vehicle	0	Total
7-Thrown or Falling Object	0	
8-Other Non-Collision	0	
9-Pedestrian	0	
10-Pedalcycle	0	
11-Railway Vehicle - Train, Engine	0	
12-Animal	1	
13-Motor Vehicle in Transport	6	
14-Parked Motor Vehicle	0	
15-Struck by Falling, Shifting Cargo or Anything	0	
Set in Motion by Motor Vehicle		
16-Work Zone / Maintenance Equipment	0	
17-Other Non-Fixed Object	0	1-Traffic Si
18-Impact Attenuator / Crash Cushion	0	2-Traffic Si
19-Bridge Overhead Structure	0	3-Advisory/
20-Bridge Pier or Support	0	4-Stop Sigr
21-Bridge Rail	0	5-Stop Sigr
22-Cable Barrier	0	6-Yield Sig
23-Culvert	0	7-Curve Wa
24-Curb	0	8-Officer, F
25-Ditch	0	9-School B
26-Embankment	0	10-School
27-Guardrail Face	0	11-R.R. Cr
28-Guardrail End	0	12-No Pass
29-Concrete Traffic Barrier	0	13-None
30-Other Traffic Barrier	0	14-Other
31-Tree (Standing)	0	 Total
32-Utility Pole / Light Support	0	Total
33-Traffic Sign Support	0	
34-Traffic Signal Support	0	
35-Fence	0	
36-Mailbox	0	
37-Other Post Pole or Support	0	

Traffic Control Devices	
Traffic Control Device	Total
1-Traffic Signals (Stop & Go)	0
2-Traffic Signals (Flashing)	0
3-Advisory/Warning Sign	1
4-Stop Signs - All Approaches	0
5-Stop Signs - Other	2
6-Yield Sign	0
7-Curve Warning Sign	0
8-Officer, Flagman, School Patrol	0
9-School Bus Stop Arm	0
10-School Zone Sign	0
11-R.R. Crossing Device	0
12-No Passing Zone	0
13-None	0
14-Other	1
Total	4

	Injury Data	
Severity Code	Injury Crashes	Number Of Injuries
K	0	0
Α	0	0
В	0	0
С	2	2
PD	2	0
Total	4	2

	Road Character	
	Road Grade	Total
1-Level		3
2-On Grade		0
3-Top of Hill		0
4-Bottom of Hill		1
5-Other		0
Total		4

Total
3
0
0
1
0
0
0
4

Maine Department Of Transportation - Traffic Engineering, Crash Records Section

Crash Summary II - Characteristics

Crashes by Year and Month

DECEMBER	0	1	0
NOVEMBER	0	0	0
OCTOBER	0	0	0
SEPTEMBER	0	0	0
AUGUST	0	0	0
JULY	0	0	0
JUNE	0	0	0
MAY	0	0	1
APRIL	0	0	0
MARCH	0	0	0
FEBRUARY	0	1	0
JANUARY	1	0	0
Month	2014	2015	2016

Report is limited to the last 10 years of data.

Maine Department Of Transportation - Traffic Engineering, Crash Records Section

Crash Summary II - Characteristics

					Crashes	s by Crash	Type ar	nd Type of L	ocation						
Crash Type	Straigh Road	t Curved Road	Three Leg Intersection	Four Leg Intersection	Five or More Leg Intersection	Driveways	Bridges	Interchanges	Other	Parking Lot	Private Way	Cross Over	Railroad Crossing	Traffic Circle- Roundabout	Total
Object in Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Rear End - Sideswipe	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Head-on - Sideswipe	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Intersection Movement	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Train	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Went Off Road	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
All Other Animal	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bicycle	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Jackknife	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Rollover	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Fire	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Submersion	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Thrown or Falling Object	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bear	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Deer	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Moose	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Turkey	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1	1	2	0	0	0	0	0	0	0	0	0	0	0	4

Weather										Water		
Light	Dry	Ice/Frost	Mud, Dirt, Gravel	Oil	Other	Sand	Slush	Snow	Unknown	(Standing, Moving)	Wet	Total
Blowing Sand, Soil, Dirt												
Dark - Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0	0
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0
Blowing Snow												
Dark - Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0	0
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0
Clear												
Dark - Lighted	1	0	0	0	0	0	0	0	0	0	0	1
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	1	0	0	0	0	0	0	1	0	0	0	2
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0
Cloudy												
Dark - Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0	0
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0

Weather Light	Dry	Ice/Frost	Mud, Dirt, Gravel	Oil	Other	Sand	Slush	Snow	Unknown	Water (Standing, Moving)	Wet	Total
Fog, Smog, Smoke												
Dark - Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0	0
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0
Other												
Dark - Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0	0
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0
Rain												
Dark - Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0	0
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0
Severe Crosswinds												
Dark - Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0	0
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0

			Crashe	s by Weat	ther, Light (Condition a	and Road S	urface				
Weather Light	Dry	Ice/Frost	Mud, Dirt, Gravel	Oil	Other	Sand	Slush	Snow	Unknown	Water (Standing, Moving)	Wet	Total
Sleet, Hail (Freezing Rain or D	rizzle)											
Dark - Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	1	0	0	0	0	0	0	0	0	0	1
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0
Snow												
Dark - Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Not Lighted	0	0	0	0	0	0	0	0	0	0	0	0
Dark - Unknown Lighting	0	0	0	0	0	0	0	0	0	0	0	0
Dawn	0	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0	0
Dusk	0	0	0	0	0	0	0	0	0	0	0	0
Unknown	0	0	0	0	0	0	0	0	0	0	0	0
OTAL	2	1	0	0	0	0	0	1	0	0	0	4



STORMWATER MANAGEMENT REPORT

MB BARK, LLC RECYCLING FACILITY EXPANSION 100 BARK MULCH DRIVE AUBURN, MAINE/POLAND, MAINE

PREPARED FOR

ST. GERMAIN COLLINS 846 MAIN STREET WESTBROOK, MAINE 04092

PREPARED BY

STANTEC CONSULTING SERVICES, INC. 482 PAYNE ROAD SCARBOROUGH, MAINE 04074

(207) 883-3355

FEBRUARY 2018

TABLE OF CONTENTS

Section	<u>Description</u>	<u>Page</u>
1.0	INTRODUCTION	3
2.0	EXISTING SITE CONDITIONS	3
3.0	PROPOSED SITE CONDITIONS	4
4.0	NATURAL RESOURCES	5
5.0	REFERENCES	5
6.0	METHOD OF ANALYSIS	5
7.0	EROSION AND SEDIMENTATION CONTROL (BASIC STANDARD)	6
8.0	WATER QUALITY MEASURES (GENERAL STANDARD)	7
9.0	STORMWATER MANAGEMENT FOR FLOOD CONTROL (FLOODING STANDARD)	7
10.0	MAINTENANCE AND GOOD HOUSEKEEPING MEASURES	8
11.0	CONCLUSIONS	8
12.0	ATTACHMENTS	9
<u>Attacl</u>	<u>nments</u>	
Attach	ment A – Predevelopment HydroCAD Computations	

Attachment B – Post Development HydroCAD Computations

Attachment C – Water Quality Treatment Summary

Attachment D – Redevelopment Computations and Figures

Attachment E – Wet Pond Computations Summary

Attachment F – Wet Pond Stage Storage Computations

1.0 INTRODUCTION

Stantec Consulting Services, Inc. has been retained by St. Germain Collins to prepare the following Stormwater Management analysis. The analysis is included as part of a Maine Department of Environmental Protection (MeDEP) Chapter 409 Solid Waste Processing Facility Amendment for the expansion of the recycling facility located at 100 Bark Mulch Drive. The facility is split between the municipalities of Auburn and Poland, Maine.

This Stormwater Management analysis has been prepared in accordance with the MeDEP Chapter 500 Rules for Stormwater Management, last revised June 2014. Specifically, this project meets the Basic, General, and Flooding Standards.

The purpose of this analysis is to identify what measures will be implemented to provide stormwater management for the proposed development specifically for water quality improvement, water quantity control, and erosion and sedimentation control. The analysis was prepared to ensure that the development will not result in any adverse effects to the environment, any natural resources, or to properties or infrastructure located downstream of the project site.

2.0 EXISTING SITE CONDITIONS

The project site, located at 100 Bark Mulch Drive, is split between two municipalities with approximately 24.08 acres in Auburn, Maine and approximately 148.10 acres in Poland, Maine. From approximately 1998 to 2006, the site functioned as a bark mulch manufacturing facility, owned and operated by Morse Brothers, Inc. In 2006, the site was purchased out of bankruptcy and has since been referred to as MB Bark, LLC, the current Owner/Operator. The site is located off Hardscrabble Road approximately 2,500 feet northwesterly of the Auburn-Lewiston Airport and approximately 1,000 feet southwesterly of the Little Androscoggin River and has a single gated entrance. The Lewiston and Auburn Railroad and Saint Lawrence Railroad run along the southerly and southwesterly property lines respectively.

The site currently functions as a solid waste recycling and staging facility which handles materials like wood waste, composted waste, inert materials such as brick, rip rap, pavement and concrete and other aggregates. The facility processes and recycles the waste products to produce usable material in the form of bark mulch, loam, fuel and base aggregates typically for construction use.

The site is located within the Little Androscoggin River watershed. Stormwater runoff is handled through a series of onsite controls and conveyances including ditches, swales, stormwater management ponds, stormwater management filters, natural wetlands, and tributary streams and brooks. Two streams bisect the site; a perennial tributary of Davis Brook flows through the easterly portion of the site and an intermittent stream located in the westerly undeveloped portion of the site. Each stream is a tributary of the Little Androscoggin River. There are two stormwater management ponds that appear to have been designed specifically for flood control rather than water quality treatment. They are located in the interior of the site and have existing gravel pad areas draining to them. They both discharge through control structures into channels tributary to the Davis Brook tributary. Two stormwater management filters have been recently constructed to provide water quality treatment and flood control to the compost processing area. The two filters discharge through control structures into the large natural wetland area in the interior of

the site. The natural wetland provides the majority of the flood control for the site. The westerly undeveloped portion of the site consists of primarily wooded area with many forested wetlands and the intermittent stream described above.

There are four Points of Interest for stormwater discharge from the project site. Two are the downstream points of the streams bisecting the site. The third is the outlet point from the large wetland area in the interior or the site. The last is a small culvert crossing Hardscrabble Road, which will not be impacted by development from this project. See the attached watershed plans for the numbering and location of each Point of Interest.

Of the 172-acre site, approximately 94 acres are currently developed consisting of roof area, gravel surfaces, hard surfaces, landscaped areas, and stormwater management areas. Table 18-1 below details the land cover breakdown for the existing site:

Table 1 Existing Land Cover Areas						
Land Cover	Area (ac.)					
Impervious Surface (roof, gravel, concrete, pavement)	55.31					
Stormwater Management Water Surface	1.78					
Landscaped Area	36.82					
Wooded Area	78.28					
Total	172.18					

The site is essentially split into two portions with the easterly portion being the developed portion and the westerly portion being the primarily wooded undeveloped portion. For the purpose of this report the two areas will be analyzed separately.

The site consists of a wide variety of soil types ranging from Hydrologic Soil Group A to Hydrologic Soil Group D. Table 2 below breaks down the soil type for landscaped and wooded areas. Soil type was not analyzed for impervious surfaces.

Table 2 Soil Type Summary for Landscaped and Wooded Areas						
Hydrologic Soil Group	Area (ac.)					
HSG A	65.40					
HSG B	5.49					
HSG C	20.45					
HSG D	28.73					
Total	120.07					

3.0 PROPOSED SITE CONDITIONS

The proposed development for the undeveloped westerly portion of the site includes an expansion of the existing recycling facility to provide two additional gravel pad areas for material staging and processing. The two pads are approximately 2.75 acres and 5.42 acres in size respectively. The development includes two 24' wide gravel surface access drives (approximately 1,650 LF and 950 LF respectively) to each of the pads, as well as stormwater management controls and conveyances required for the project. The access drives at wetland crossings will apply a low impact development permeable road base, or

"rock sandwich" technique to allow hydraulic connectivity of the wetland through the base course of the roadway. The access drives have been aligned to minimize the natural resource impact for the project.

Development within the easterly portion of the site is primarily redevelopment activity that will combine various pad and handling areas into a large, better drained, and more functional processing area. The development activity for this portion of the project consists of filling and regrading, as well as providing stormwater management to meet current regulatory standards.

The existing drainage patterns will remain the same in the post development condition. There are still four Points of Interest as described above. Conveyances and controls for stormwater runoff will be constructed or modified in order to ensure there is no adverse impacts from the development.

4.0 NATURAL RESOURCES

There are various natural resources located within the project site including natural wetlands, potential significant vernal pools, streams, and protected species. A natural resource report has been prepared separately and should be reviewed for further information. The proposed development has been designed to limit or prevent impact to natural resources on site.

5.0 REFERENCES

The following reference sources were used in preparation of the stormwater analysis:

- 1. MaineDEP <u>Stormwater Management for Maine Volume III BMP Technical Design Manual</u>
- 2. HydroCAD Stormwater Modeling Software, Version 10.00, build 20
- 3. <u>MaineDEP Erosion and Sediment Maine Erosion and Sediment Control BMP's,</u>
 October 2016 revision
- 4. MaineDEP Chapter 500 Rules for Stormwater Management, June 2014 revision
- 5. <u>U.S. Department of Agriculture Natural Resources Conservation Service Web Soil</u> Survey

6.0 METHOD OF ANALYSIS

The hydrologic analysis for predevelopment and post development conditions was conducted based upon the methodology contained in the USDA Soil Conservation Service's Technical Releases Nos. 20 and 55 (SCS TR-20 and TR-55). For Androscoggin County Maine, a 24-hour SCS Type III storm distribution was used for the analysis with the following storm frequencies and rainfall amounts:

Table 3					
Hydrologic Analysis Parameters					
Storm Event	24-Hour Rainfall				
2-Year Storm	3.0 inches				
10-Year Storm	4.3 inches				
25-Year Storm	5.4 inches				

The HydroCAD computer program was used in the analysis. This program allows critical points of the watershed to be analyzed using the SCS TR-20 methodology to calculate the anticipated conditions at these points. Drainage areas are defined with runoff curve numbers, times of concentration, and travel time data based on methods outlined in the USDA TR-55 manual. To assess storage and kinematic effects of runoff, the model uses reservoirs and pipes to imitate actual conditions. Specific hydrologic characteristics including travel times, storage capacity, and the effects of hydraulic head are considered for analysis with this program.

To model the watersheds, the drainage system is represented by a network consisting of three basic components:

- **Subcatchment:** A relatively homogenous area of land that drains into a single reach or pond. Each subcatchment generates a runoff hydrograph.
- Reach: A uniform stream, channel, or pipe that conveys water from one point to another reach or pond. The outflow of each reach is determined by a hydrograph routing calculation.
- Pond: A pond, swamp, dam, or other impoundment which fills with water from one or more sources and empties in a manner determined by a weir, culvert or other device(s) at its outlet. A pond may empty into a reach or into another pond. The outflow of each pond is also determined by a hydrograph routing calculation.

7.0 EROSION AND SEDIMENTATION CONTROL (BASIC STANDARD)

Erosion and sedimentation control (ESC) will be accomplished for this project through the application of various temporary construction and permanent ESC BMPs as described in the MeDEP Erosion and Sediment Control BMP Manual. BMPs proposed include but are not limited to the following:

- Stabilized construction entrance
- Siltation fence
- Temporary construction stabilization
- Slope stabilization

The contractor will also be required to employ ESC BMPs for any material stockpiles as well as any areas left denuded for extended periods of time during construction.

The ESC plan for this project complies with the requirements detailed in Appendix A of MeDEP's Chapter 500 Rules for Stormwater Management.

8.0 WATER QUALITY MEASURES (GENERAL STANDARD)

The development associated with this project is broken into two areas for computations in relation to the General Standard. Each area will apply different sections of the Chapter 500 rules; one follows guidelines for new development while the second follows the rules for redevelopment projects. The two areas and water quality measures associated with each are described below:

New Development – Westerly Portion of the Project Site:

The westerly portion of the project includes development of two new material processing and staging pads and access drives to each. This portion of the project is considered new development and therefore Section 4.C.2.a of MeDEP Chapter 500 applies. Table 1 of that section has been applied to prorate the required treatment level based on the ratio of developed land to land available for development. This calculation allows a reduction from the standard treatment levels of 95% of impervious area and 80% of developed area down to 90% of impervious area, and 75% of developed area.

Two BMPs will be utilized to provide treatment to stormwater runoff for this portion of the project: wet ponds and forested buffers. BMPs proposed for the project have been designed in accordance with the MeDEP BMP Design Manual. Computations for each BMP are provided in Attachment E & F. A water quality summary describing how the project meets the General Standard has been provided as Attachment C.

Redevelopment – Easterly Portion of the Project Site:

The easterly portion of the project includes development of one new material processing and staging pad. This area combines a number of existing "work" areas into a larger more uniform work area as well as develop previously unusable areas. Because the majority of this portion of the project is currently developed, rules from MeDEP Chapter 500 section 4.C.2.d.i have been applied. The computation described in the referenced section results in a required treatment level of 50% of total redeveloped area. Treatment will be provided through two wet ponds. The design of the ponds is described in Attachment E & F. A summary of water quality treatment for the redeveloped portion of the project is included as Attachment C.

The BMPs described above have been designed per MeDEP's BMP Design Manual to meet the requirements of the General Standard.

9.0 STORMWATER MANAGEMENT FOR FLOOD CONTROL (FLOODING STANDARD)

The stormwater system for this project has been designed to provide detention, and in turn, reduce peak discharge rates from stormwater runoff. Specifically, the 2, 10, and 25-year storm events have been analyzed for this project. Table 3 above summarizes rainfall amounts for each of the storm events analyzed. Table 4 below summarizes peak discharge rates for each storm event for the predevelopment and post development conditions:

Table 4 Comparison of Peak Discharge Rates at Points of Interest						
Storm Event & Condition	POI 301 (cfs)	POI 401 (cfs)	POI 501 (cfs)	POI 601 (cfs)		
2-Yr Predevelopment	1.54	30.36	1.23	1.02		
2-Yr Post Development	0.93	19.03	1.23	0.95		
Change in 2-Yr Peak Discharge Rate	-0.61	-11.33	0.00	-0.07		
10-Yr Predevelopment	4.55	56.67	2.32	6.85		
10-Yr Post Development	3.06	38.23	2.32	5.98		
Change in 10-Yr Peak Discharge Rate	-1.49	-18.44	0.00	-0.87		
25-Yr Predevelopment	7.66	88.24	3.31	16.23		
25-Yr Post Development	6.31	55.98	3.31	14.82		
Change in 25-Yr Peak Discharge Rate	-1.35	-32.26	0.00	-1.41		

Peak discharge rates for each storm event have been decreased from predevelopment to post development conditions for this project. Therefore, the Flooding Standard has been met.

10.0 MAINTENANCE AND GOOD HOUSEKEEPING MEASURES

The Owner or the Owner's Representative will be responsible for maintenance of permanent stormwater conveyance and treatment systems constructed as part of this project. Inspection, maintenance, and housekeeping action will comply with Appendix B of MeDEP's Chapter 500 Rules for Stormwater Management and includes, but is not limited to:

- Inspection of infrastructure at regular intervals as established within Appendix B
- Removal and proper disposal of sediment build up in conveyance systems and BMPs
- Replacement of any BMP or portion of BMP that is not operating correctly
- Proper documentation of all maintenance activity

The erosion and sediment control plan and maintenance plan meet the Basic Standard.

11.0 CONCLUSIONS

The stormwater management system for the proposed development will mitigate negative effects of stormwater runoff generated from the development by reducing peak discharge rates, improving water quality of stormwater runoff discharged from the project site, and minimizing potential erosion and sedimentation due to the development. There will be no adverse effects to downstream conveyance systems or properties anticipated due to stormwater runoff from this project.

12.0 ATTACHMENTS

Attachment A – Predevelopment HydroCAD Computations

Attachment B – Post Development HydroCAD Computations

Attachment C – Water Quality Treatment Summary

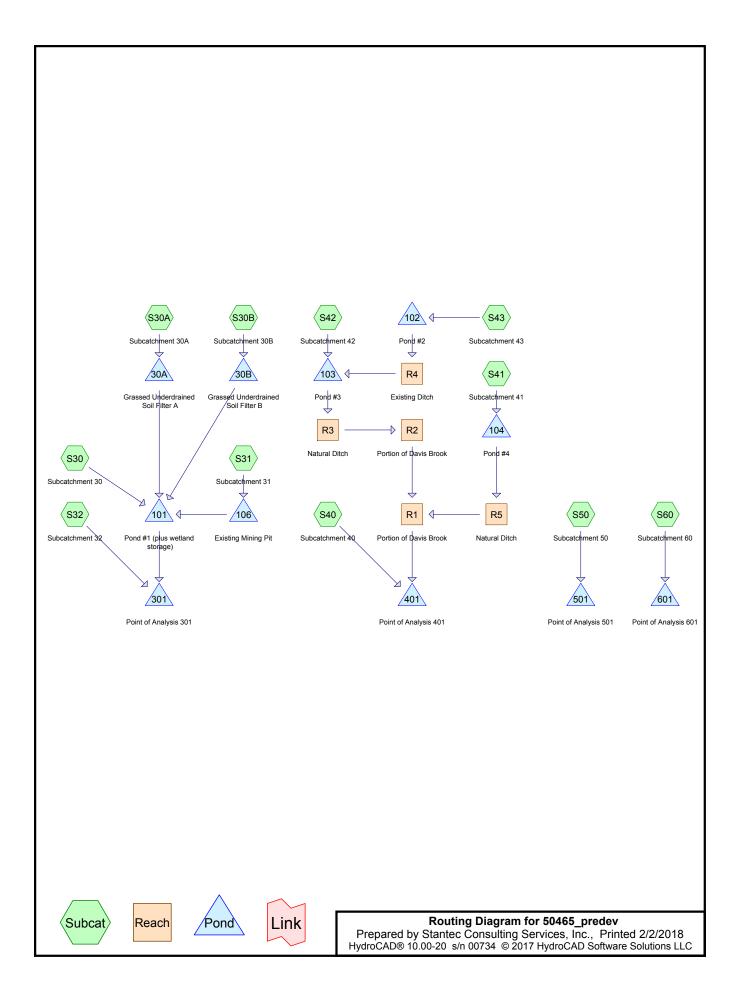
Attachment D – Redevelopment Computations and Figures

Attachment E – Wet Pond Computations Summary

Attachment F – Wet Pond Stage Storage Computations

ATTACHMENT A

PREDEVELOPMENT HYDROCAD COMPUTATIONS



Area Listing (all nodes)

Area	CN	Description (a) the ortal property and parely
(acres)		(subcatchment-numbers)
55.307	98	(S30, S30A, S30B, S31, S32, S40, S41, S42, S43, S50, S60)
11.573	49	50-75% Grass cover, Fair, HSG A (\$30, \$30A, \$30B, \$42, \$43, \$60)
0.860	69	50-75% Grass cover, Fair, HSG B (S42, S43)
4.616	79	50-75% Grass cover, Fair, HSG C (S30, S30A, S30B, S41, S42, S43)
0.872	84	50-75% Grass cover, Fair, HSG D (S41)
2.580	39	>75% Grass cover, Good, HSG A (S32, S40)
3.275	61	>75% Grass cover, Good, HSG B (S32, S40, S50)
4.140	74	>75% Grass cover, Good, HSG C (\$32, \$40, \$50)
8.904	80	>75% Grass cover, Good, HSG D (\$32, \$40)
1.263	98	Water Surface (S41, S42, S43)
0.514	98	Water Surface, HSG C (S30)
46.348	36	Woods, Fair, HSG A (S43, S60)
0.136	60	Woods, Fair, HSG B (S43)
3.109	73	Woods, Fair, HSG C (S60)
14.168	79	Woods, Fair, HSG D (S60)
4.903	30	Woods, Good, HSG A (S30, S32, S40)
1.215	55	Woods, Good, HSG B (S40)
7.831	70	Woods, Good, HSG C (S30, S32, S40)
4.784	77	Woods, Good, HSG D (S32, S40)
0.242	77	Woods, Poor, HSG C (S43)
176.641	69	TOTAL AREA

50465_predevPrepared by Stantec Consulting Services, Inc.
HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
65.404	HSG A	S30, S30A, S30B, S32, S40, S42, S43, S60
5.486	HSG B	\$32, \$40, \$42, \$43, \$50
20.452	HSG C	\$30, \$30A, \$30B, \$32, \$40, \$41, \$42, \$43, \$50, \$60
28.728	HSG D	S32, S40, S41, S60
56.570	Other	\$30, \$30A, \$30B, \$31, \$32, \$40, \$41, \$42, \$43, \$50, \$60
176.641		TOTAL AREA

Printed 2/2/2018 Page 4

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	0.000	55.307	55.307		S30, S30A, S30B,
							S31, S32, S40,
							S41, S42, S43,
							S50, S60
11.573	0.860	4.616	0.872	0.000	17.922	50-75% Grass cover, Fair	S30, S30A, S30B,
							S41, S42, S43,
							\$60
2.580	3.275	4.140	8.904	0.000	18.900	>75% Grass cover, Good	\$32, \$40, \$50
0.000	0.000	0.514	0.000	1.263	1.778	Water Surface	S30, S41, S42,
							\$43
46.348	0.136	3.109	14.168	0.000	63.761	Woods, Fair	\$43, \$60
4.903	1.215	7.831	4.784	0.000	18.732	Woods, Good	\$30, \$32, \$40
0.000	0.000	0.242	0.000	0.000	0.242	Woods, Poor	\$43
65.404	5.486	20.452	28.728	56.570	176.641	TOTAL AREA	

Page 5

Summary for Subcatchment S30: Subcatchment 30

Runoff = 14.85 cfs @ 12.77 hrs, Volume= 2.583 af, Depth= 1.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Ar	ea (sf)	CN	Des	scription		
	1	51,673	30	Wo	ods, Goo	d, HSG A	
	1	29,474	70	Wo	ods, Goo	d, HSG C	
	1	08,224	79	50-	75% Grass	s cover, Fair	r, HSG C
		39,615	49	50-	75% Grass	s cover, Fair	r, HSG A
		22,394	98	Wa	iter Surfac	ce, HSG C	
*	5	76,689	98				
	1,0	28,069	81	We	ighted A	verage	
		28,986			73% Pervi		
	5	99,083		58.2	27% Impe	rvious Area	
	_						
	TC	Length	Slop		·	Capacity	Description
_	(min)	(feet)	(ft/1		(ft/sec)	(cfs)	
	0.8	75	0.040	00	1.66		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	4.9	481	0.010)4	1.64		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	49.2	1,094	0.005	55	0.37		Shallow Concentrated Flow, C to D
_							Woodland Kv= 5.0 fps
	54.9	1.650	Total	I			

Summary for Subcatchment S30A: Subcatchment 30A

Runoff = 8.49 cfs @ 12.09 hrs, Volume= 0.642 af, Depth= 2.35"

_	Ar	ea (sf)	CN	Description		
*	1	15,960	98			
		24,290	79	50-75% Gras	s cover, Fair	r, HSG C
		2,477	49	50-75% Gras	s cover, Fair	r, HSG A
	1	42,727	94	Weighted A	verage	
		26,767		18.75% Perv	ious Area	
	1	15,960		81.25% Impe	ervious Area	
	Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description
	1.1	75	0.015	50 1.12		Sheet Flow, A to B
	2.8	333	0.015	50 1.97		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C
_						Unpaved Kv= 16.1 fps
	39	408	Total	Increased	to minimum	$T_{C} = 6.0 \text{ min}$

Page 6

Summary for Subcatchment S30B: Subcatchment 30B

Runoff = 12.82 cfs @ 12.09 hrs, Volume= 0.982 af, Depth= 2.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Ar	ea (sf)	CN	De	escription		
*	1	97,982	98				
		947	79	50-	-75% Gras	s cover, Fair	r, HSG C
		10,676	49	50-	-75% Gras	s cover, Fair	r, HSG A
	2	09,605	95	We	eighted A	verage	
		11,623		5.5	55% Pervio	us Area	
	1	97,982		94	.45% Impe	rvious Area	
	TC	Length	Slop	эе	Velocity	Capacity	Description
(min)	(feet)	(ft/	′f†)	(ft/sec)	(cfs)	
	1.0	75	0.02	00	1.26		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	1.9	312	0.03	00	2.79		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	2.9	387	Tota	al, Ir	ncreased	to minimum	TC = 6.0 min

Summary for Subcatchment S31: Subcatchment 31

Runoff = 10.59 cfs @ 12.09 hrs, Volume= 0.862 af, Depth= 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

_	Αı	rea (sf)	CN I	Description			
×	* 1	62,835	98				
-	1	62,835		100.00% Imp	ervious Are	a	
	Tc (min)	Length (feet)	Slope (ft/ft	•	Capacity (cfs)	Description	
Ī	1.0	75	0.020	0 1.26		Sheet Flow, A to B	
	5.1	700	0.020	0 2.28		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C Unpaved Kv= 16.1 fps	
	6.1	775	Total				

Summary for Subcatchment S32: Subcatchment 32

Runoff = 0.29 cfs @ 12.78 hrs, Volume= 0.100 af, Depth= 0.19"

Prepared by Stantec Consulting Services, Inc.
HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page	

	Area (sf)	CN	De	escription					
*	10,665	98							
	61,121	30	W	Woods, Good, HSG A					
	4,060	70	W	oods, God	od, HSG C				
	8,302	77	W	oods, God	od, HSG D				
	86,042	39	>7	5% Grass of	cover, Goo	d, HSG A			
	3,152	61		-,	cover, Goo	-,			
	35,319	74			cover, Goo				
	58,471	80	>7	5% Grass o	cover, Goo	d, HSG D			
	267,132	55		eighted A	_				
	256,467			.01% Pervi					
	10,665		3.9	9% Imper	vious Area				
-	To longth	SI.	5 0	\/alaaity	Canacity	Description			
	c Length		/ft)	•		Description			
(mi		,		(ft/sec)	(cfs)	Observation Andrews			
16	.9 75	0.00	080	0.07		Sheet Flow, A to B			
1 /	0 /00	0.00	000	0 /2		Grass: Dense n= 0.240 P2= 3.00"			
16	.0 600	0.00	000	0.63		Shallow Concentrated Flow, B to C			
	0 /75	·				Short Grass Pasture Kv= 7.0 fps			
32	.9 675	Tota	II						

Summary for Subcatchment \$40: Subcatchment 40

Runoff = 30.35 cfs @ 12.36 hrs, Volume= 3.592 af, Depth= 1.25"

	Area (sf)	CN	Description
*	429,894	98	
	200,076	77	Woods, Good, HSG D
	207,565	70	Woods, Good, HSG C
	52,912	55	Woods, Good, HSG B
	765	30	Woods, Good, HSG A
	329,379	80	>75% Grass cover, Good, HSG D
	120,178	74	>75% Grass cover, Good, HSG C
	135,075	61	>75% Grass cover, Good, HSG B
	26,357	39	>75% Grass cover, Good, HSG A
	1,502,201	80	Weighted Average
	1,072,307		71.38% Pervious Area
	429,894		28.62% Impervious Area

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 8

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	1.3	75	0.0100	0.95	(0.3)	Sheet Flow, A to B
	6.0	776	0.0180	2.16		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C
	4.3	0.40	0.0440	1.50		Unpaved Kv= 16.1 fps
	4.1	368	0.0462	1.50		Shallow Concentrated Flow, C to D Short Grass Pasture Kv= 7.0 fps
	13.6	1,500	0.0030	1.84	55.08	Channel Flow, D to E Area= 30.0 sf Perim= 35.0' r= 0.86' n= 0.040 Earth, cobble bottom, clean sides
_	25.0	2,719	Total			

Summary for Subcatchment S41: Subcatchment 41

Runoff = 17.48 cfs @ 12.10 hrs, Volume= 1.376 af, Depth= 2.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

_	Ar	ea (sf)	CN	De	scription		
*	2	25,409	98				
		22,827	79	50-	-75% Gras	s cover, Fair	, HSG C
		37,988	84	50-	-75% Gras	s cover, Fair	r, HSG D
*		7,519	98	Wo	ater Surfac	ce	
	2	93,743	95		eighted A		
	60,815			20.	.70% Pervi	ous Area	
	2	232,928			.30% Impe	rvious Area	
	Tc	Length			-	Capacity	Description
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)	
	1.3	75	0.010	00	0.95		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	3.3	383	0.01	43	1.93		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	2.5	243	0.05	35	1.62		Shallow Concentrated Flow, C to D
_							Short Grass Pasture Kv= 7.0 fps
	7.1	701	Tota	ıl			

Summary for Subcatchment \$42: Subcatchment 42

Runoff = 16.43 cfs @ 12.15 hrs, Volume= 1.398 af, Depth= 2.07"

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Soft

•	'n 00734 © 2017 HydroCAD Software Solutions LLC	Page 9
Ara a (af) (b)		-

_	Ar	ea (sf)	CN	Descri	iption		
*	2	78,726	98				
*		11,139	98	Water	r Surfac	ce	
		26,977	49	50-759	% Grass	s cover, Fair	r, HSG A
		29,325	69	50-759	% Grass	s cover, Fair	r, HSG B
_		6,630	79	50-759	% Grass	s cover, Fair	, HSG C
		52,797	91	Weigh	nted Av	verage	
	62,932				-	ous Area	
	2	89,865		82.169	% Impe	rvious Area	
	To	Lanath	Class	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	olo oity	Canacity	Description
	Tc (min)	Length	Slop (ft/f		•		Description
_	(min)	<u>(feet)</u>			t/sec)	(cfs)	
	1.3	75	0.010)()	0.95		Sheet Flow, A to B
		700					Smooth surfaces n=0.011 P2=3.00"
	6.1	700	0.014	40	1.90		Shallow Concentrated Flow, B to C
	0 1	005	0.00	-0			Unpaved Kv= 16.1 fps
	3.4	225	0.025	00	1.11		Shallow Concentrated Flow, C to D
_							Short Grass Pasture Kv= 7.0 fps
	10.8	1,000	Total				

Summary for Subcatchment \$43: Subcatchment 43

Runoff = 7.28 cfs @ 12.41 hrs, Volume= 0.938 af, Depth= 0.91"

	Are	ea (sf)	CN	De	escription						
*	23	32,052	98								
*	3	36,376	98								
	12	22,740	49	50-	-75% Gras	s cover, Fair	r, HSG A				
	3	88,160	79	50-	-75% Gras	s cover, Fair	r, HSG C				
		8,153	69	50-	-75% Gras	s cover, Fair	r, HSG B				
	3	36,000	36	W	oods, Fair,	HSG A					
		5,923	60	W	oods, Fair,	HSG B					
	1	0,549	77	W	ods, Poo	r, HSG C					
539,953 74 Weighted Average						verage					
	271,525		50.29% P		.29% Pervi	29% Pervious Area					
	268,428			49.71% Impervious Area							
	Tc	Length	Slop	ре	Velocity	Capacity	Description				
(m	nin)	(feet)	(ft/	ft)	(ft/sec)	(cfs)					
1	0.6	75	0.01	00	0.12		Sheet Flow, A to B				
							Grass: Short n= 0.150 P2= 3.00"				
1	6.3	488	0.01	00	0.50		Shallow Concentrated Flow, B to C				
							Woodland Kv= 5.0 fps				
2	6.9	563	Tota	I							

<u>Page 10</u>

Summary for Subcatchment S50: Subcatchment 50

Runoff = 1.23 cfs @ 12.10 hrs, Volume= 0.091 af, Depth= 1.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Area (sf)	CN	Description						
*	10,546	98							
	4,431	61	>75% Grass cover, Good, HSG B						
	24,859	74	>75% Grass cover, Good, HSG C						
	39,836	79	Weighted Average						
	29,290	29,290 73.53% Pervious Area							
	10,546		26.47% Impervious Area						
(m	Tc Length nin) (feet)		pe Velocity Capacity Description /ft) (ft/sec) (cfs)						
	6.0		Direct Entry,						

Summary for Subcatchment S60: Subcatchment 60

Runoff = 1.02 cfs @ 15.68 hrs, Volume= 0.657 af, Depth= 0.11"

	Ar	ea (sf)	CN	De	escription				
*	* 168,411 98								
	3	01,647	49	50	-75% Gras	s cover, Fair	r, HSG A		
	1,932,915 36 Woods, I					HSG A			
	1	35,432	73	W	oods, Fair,	HSG C			
617,169 79 Woods, Fair, HSG D									
3,155,574 51 Weighted Average									
	2,987,163			94	.66% Pervi	ous Area			
	168,411			5.34% Impervious Area					
	Tc	Length	Slop	ре	Velocity	Capacity	Description		
	min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)			
	23.2	75	0.01	00	0.05		Sheet Flow, A to B		
							Woods: Light underbrush n= 0.400 P2= 3.00"		
	83.4	1,640	0.00	43	0.33		Shallow Concentrated Flow, B to C		
							Woodland Kv= 5.0 fps		
	1.1	330	0.01	00	5.10	122.30	Channel Flow, C to D		
							Area= 24.0 sf Perim= 23.0' r= 1.04'		
							n= 0.030 Earth, grassed & winding		
1	07.7	2,045	Tota	ı					

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 11

Summary for Reach R1: Portion of Davis Brook

Inflow Area = 27.238 ac, 66.69% Impervious, Inflow Depth > 0.31" for 2-Yr event

Inflow = 4.27 cfs @ 12.57 hrs, Volume= 0.706 af

Outflow = 1.56 cfs @ 13.68 hrs, Volume= 0.698 af, Atten= 63%, Lag= 66.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.80 fps, Min. Travel Time= 33.1 min Avg. Velocity = 0.28 fps, Avg. Travel Time= 96.2 min

Peak Storage= 3,105 cf @ 13.13 hrs Average Depth at Peak Storage= 0.30' Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 176.49 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 1,600.0' Slope= 0.0031 '/'

Inlet Invert= 221.00', Outlet Invert= 216.00'



Summary for Reach R2: Portion of Davis Brook

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.05" for 2-Yr event

Inflow = 0.02 cfs @ 24.55 hrs, Volume= 0.086 af

Outflow = 0.02 cfs @ 27.25 hrs, Volume= 0.084 af, Atten= 0%, Lag= 162.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.20 fps, Min. Travel Time= 49.3 min Avg. Velocity = 0.20 fps, Avg. Travel Time= 49.3 min

Peak Storage= 53 cf @ 26.43 hrs

Average Depth at Peak Storage= 0.02'

Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 182.28 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 600.0' Slope= 0.0033 '/'

Inlet Invert= 223.00', Outlet Invert= 221.00'



HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Summary for Reach R3: Natural Ditch

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.05" for 2-Yr event

Inflow 0.02 cfs @ 24.29 hrs. Volume= 0.086 af

0.02 cfs @ 24.55 hrs, Volume= Outflow 0.086 af, Atten= 0%, Lag= 15.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.84 fps, Min. Travel Time= 5.9 min Avg. Velocity = 0.84 fps, Avg. Travel Time= 5.9 min

Peak Storage= 6 cf @ 24.45 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 998.06 cfs

10.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 5.0 '/' Top Width= 40.00'

Length= 300.0' Slope= 0.0433 '/'

Inlet Invert= 236.00', Outlet Invert= 223.00'



Summary for Reach R4: Existing Ditch

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 0.00" for 2-Yr event

0.00 cfs @ 0.00 hrs, Volume= Inflow 0.000 af

Outflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 168.09 cfs

8.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 20.00'

Length= 190.0' Slope= 0.0053 '/'

Inlet Invert= 244.00', Outlet Invert= 243.00'



50465 predev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Summary for Reach R5: Natural Ditch

6.743 ac, 79.30% Impervious, Inflow Depth > 1.11" for 2-Yr event Inflow Area =

Inflow 5.22 cfs @ 12.49 hrs, Volume= 0.623 af

4.27 cfs @ 12.57 hrs, Volume= Outflow 0.623 af, Atten= 18%, Lag= 4.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.60 fps, Min. Travel Time= 2.6 min Avg. Velocity = 0.74 fps, Avg. Travel Time= 5.6 min

Peak Storage= 677 cf @ 12.52 hrs

Average Depth at Peak Storage= 0.10'

Bank-Full Depth= 3.00' Flow Area= 165.0 sf, Capacity= 1,945.52 cfs

25.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 10.0 '/' Top Width= 85.00'

Length= 250.0' Slope= 0.0320 '/'

Inlet Invert= 229.00', Outlet Invert= 221.00'



Summary for Pond 30A: Grassed Underdrained Soil Filter A

Inflow Area = 3.277 ac, 81.25% Impervious, Inflow Depth = 2.35" for 2-Yr event

Inflow 8.49 cfs @ 12.09 hrs, Volume= 0.642 af

8.49 cfs @ 12.09 hrs, Volume= 0.642 af, Atten= 0%, Lag= 0.0 min Primary

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 30B: Grassed Underdrained Soil Filter B

Inflow Area = 4.812 ac, 94.45% Impervious, Inflow Depth = 2.45" for 2-Yr event

12.82 cfs @ 12.09 hrs, Volume= Inflow 0.982 af

12.82 cfs @ 12.09 hrs, Volume= 0.982 af, Atten= 0%, Laa= 0.0 min Primary

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 101: Pond #1 (plus wetland storage)

Inflow Area = 35.428 ac, 69.71% Impervious, Inflow Depth = 1.43" for 2-Yr event

24.15 cfs @ 12.09 hrs, Volume= Inflow 4.207 af

2.342 af, Atten= 94%, Lag= 355.7 min Outflow = 1.46 cfs @ 18.02 hrs, Volume=

Primary 1.46 cfs @ 18.02 hrs, Volume= 2.342 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 14

Peak Elev= 241.50' @ 18.02 hrs Surf.Area= 168,133 sf Storage= 139,682 cf

Plug-Flow detention time= 958.9 min calculated for 2.342 af (56% of inflow) Center-of-Mass det. time= 838.2 min (1,686.8 - 848.5)

Volume	Inve	rt Avail.Stor	age Stora	ge Description	
#1	240.00	0' 835,45	58 cf Custo	m Stage Data (I	Prismatic) Listed below (Recalc)
Elevation (feet	.)		Inc.Store	Cum.Store (cubic-feet)	
240.00		22,686	0	/O 205	
241.00		115,923	69,305	69,305	
242.00	_	221,289	168,606	237,911	
243.00	0	313,044	267,167	505,077	
244.00	0	347,717	330,381	835,458	
Device	Routing	Invert	Outlet De	vices	
#1	Primary	237.64'	24.0" Rou	nd Culvert	
			Inlet / Out	tlet Invert= 237.6	, no headwall, Ke= 0.900 64' / 237.30' S= 0.0039 '/' Cc= 0.900 tal, Flow Area= 3.14 sf
#2	Device 1	241.00'	24.0" Vert.	Orifice/Grate	C= 0.600

Primary OutFlow Max=1.45 cfs @ 18.02 hrs HW=241.50' (Free Discharge) 1=Culvert (Passes 1.45 cfs of 15.37 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.45 cfs @ 2.40 fps)

Summary for Pond 102: Pond #2

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 0.91" for 2-Yr event

Inflow = 7.28 cfs @ 12.41 hrs, Volume= 0.938 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 243.45' @ 25.60 hrs Surf.Area= 31,188 sf Storage= 40,866 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description	
#1	242.00'	190,120 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation	Surf.	Area Inc.	Store Cum.Store	

Elevalion	30H.Area	1110.31016	Com.siore
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.00	25,000	0	0
244.68	36,400	82,276	82,276
246.00	127,000	107,844	190,120

50465_predev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 15

Device	Routing	Invert	Outlet Devices
#1	Primary	246.53'	18.0" Round Culvert
	,		L= 165.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 246.53' / 244.00' S= 0.0153'/' Cc= 0.900 n= 0.025 Corrugated metal, Flow Area= 1.77 sf
#2	Device 1	248.05'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=242.00' (Free Discharge)

1=Culvert (Controls 0.00 cfs)

12=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 103: Pond #3

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth = 0.82" for 2-Yr event

Inflow = 16.43 cfs @ 12.15 hrs, Volume= 1.398 af

Outflow = 0.02 cfs @ 24.29 hrs, Volume= 0.086 af, Atten= 100%, Lag= 728.1 min

Primary = 0.02 cfs @ 24.29 hrs, Volume= 0.086 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 240.55' @ 24.29 hrs Surf.Area= 18,229 sf Storage= 60,150 cf

Plug-Flow detention time= 1,976.8 min calculated for 0.086 af (6% of inflow)

Center-of-Mass det. time=1,727.2 min (2,536.5 - 809.4)

Volume	Invert	Avail.Storage	Storage Description
#1	234.00'	94,130 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation	Surf.A	rea Inc.	Store Cum.Store

Elevanon	3011.AIEG	1110.31016	C0111.3101E
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
234.00	4,000	0	0
236.00	7,000	11,000	11,000
238.92	11,139	26,483	37,483
240.00	14,287	13,730	51,213
242.00	28,630	42,917	94,130

Device	Routing	Invert	Outlet Devices
#1	Primary	234.22'	24.0" Round Culvert
	•		L= 95.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 234.22' / 233.90' S= 0.0034 '/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=3.14sf
#2	Device 1	241.29'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	236.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.02 cfs @ 24.29 hrs HW=240.55' (Free Discharge)

-1=Culvert (Passes 0.02 cfs of 21.70 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

3=Orifice/Grate (Orifice Controls 0.02 cfs @ 10.25 fps)

Printed 2/2/2018 Page 16

Summary for Pond 104: Pond #4

Inflow Area = 6.743 ac, 79.30% Impervious, Inflow Depth = 2.45" for 2-Yr event

Inflow = 17.48 cfs @ 12.10 hrs, Volume= 1.376 af

Outflow = 5.22 cfs @ 12.49 hrs, Volume= 0.623 af, Atten= 70%, Lag= 23.3 min

Primary = 5.22 cfs @ 12.49 hrs, Volume= 0.623 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 238.00' @ 12.50 hrs Surf.Area= 16,786 sf Storage= 36,018 cf

Plug-Flow detention time= 501.8 min calculated for 0.622 af (45% of inflow)

Center-of-Mass det. time=382.6 min (1,166.6 - 784.0)

Volume	Invert	Avail.Storage	Storage Description
#1	230.00'	36,018 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Е	levation	Surf.Area	Inc.Store	Cum.Store
	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
	230.00	620	0	0
	232.00	3,391	4,011	4,011
	234.05	7,519	11,183	15,194
	235.00	9,087	7,888	23,082
	236.00	16,786	12,937	36,018

Device	Routing	Invert	Outlet Devices
#1	Primary	230.62'	24.0" Round Culvert
			L= 96.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 230.62' / 228.40' S= 0.0231 '/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=3.14sf
#2	Device 1	237.61'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	231.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=4.88 cfs @ 12.49 hrs HW=237.99' (Free Discharge)

-1=Culvert (Passes 4.88 cfs of 27.62 cfs potential flow)

2=Orifice/Grate (Weir Controls 4.85 cfs @ 2.02 fps)

-3=Orifice/Grate (Orifice Controls 0.02 cfs @ 12.71 fps)

Summary for Pond 106: Existing Mining Pit

Inflow Area = 3.738 ac,100.00% Impervious, Inflow Depth = 2.77" for 2-Yr event

Inflow = 10.59 cfs @ 12.09 hrs, Volume= 0.862 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 246.95' @ 24.40 hrs Surf.Area= 40,414 sf Storage= 37,563 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Type III 24-hr 2-Yr Rainfall=3.00" Printed 2/2/2018

50465_predev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 17

Volume	Inver	t Avail.Sto	rage Storag	ge Description	
#1	#1 245.00' 215,850		50 cf Custo	m Stage Data (I	Prismatic) Listed below (Recalc)
Elevatior (feet		urf.Area (sq-ft) (c	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	
245.00)	327	0	0	
246.00)	18,653	9,490	9,490	
247.00)	41,546	30,100	39,590	
248.00)	62,315	51,931	91,520	
249.00)	73,054	67,685	159,205	
249.75	5	78,000	56,645	215,850	
Device	Routing	Invert			
#1	Primary	249.75	_		Broad-Crested Rectangular Weir
	•		Head (fee	et) 0.20 0.40 0.6	60 0.80 1.00 1.20 1.40 1.60

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=245.00' (Free Discharge) 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 301: Point of Analysis 301

Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Inflow Area = 41.560 ac, 60.02% Impervious, Inflow Depth > 0.70" for 2-Yr event

Inflow = 1.54 cfs @ 17.66 hrs, Volume= 2.441 af

Primary = 1.54 cfs @ 17.66 hrs, Volume= 2.441 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 401: Point of Analysis 401

Inflow Area = 61.724 ac, 45.42% Impervious, Inflow Depth > 0.83" for 2-Yr event

Inflow = 30.36 cfs @ 12.36 hrs, Volume= 4.291 af

Primary = 30.36 cfs @ 12.36 hrs, Volume= 4.291 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 501: Point of Analysis 501

Inflow Area = 0.915 ac, 26.47% Impervious, Inflow Depth = 1.19" for 2-Yr event

Inflow = 1.23 cfs @ 12.10 hrs, Volume= 0.091 af

Primary = 1.23 cfs @ 12.10 hrs, Volume= 0.091 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 601: Point of Analysis 601

Inflow Area = 72.442 ac, 5.34% Impervious, Inflow Depth = 0.11" for 2-Yr event

Inflow = 1.02 cfs @ 15.68 hrs, Volume= 0.657 af

Primary = 1.02 cfs @ 15.68 hrs, Volume= 0.657 af, Atten= 0%, Lag= 0.0 min

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 18

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Subcatchment S30: Subcatchment 30

Runoff = 27.28 cfs @ 12.75 hrs, Volume= 4.673 af, Depth= 2.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Ar	ea (sf)	CN	De	scription					
	1	51,673	30	Wo	ods, Goo	d, HSG A				
	1	29,474	70	Wo	ods, Good, HSG C					
	1	08,224	79	50-75% Grass cover, Fair, HSG C						
		39,615	49	50-	-75% Grass	s cover, Fair	r, HSG A			
		22,394	98	Wo	ater Surfac	ce, HSG C				
*	5	76,689	98							
1,028,069 81 Weighted Average				We	eighted Av	verage				
	4	28,986		41.	.73% Pervi	ous Area				
	5	99,083		58.	.27% Impe	rvious Area				
						_				
	Tc	Length	Slop		•	•	Description			
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)				
	8.0	75	0.040	00	1.66		Sheet Flow, A to B			
							Smooth surfaces n= 0.011 P2= 3.00"			
	4.9	481	0.010	04	1.64		Shallow Concentrated Flow, B to C			
							Unpaved Kv= 16.1 fps			
	49.2	1,094	0.003	55	0.37		Shallow Concentrated Flow, C to D			
_							Woodland Kv= 5.0 fps			
	54 9	1.650	Tota							

Summary for Subcatchment S30A: Subcatchment 30A

Runoff = 12.75 cfs @ 12.09 hrs, Volume= 0.988 af, Depth= 3.62"

_	Ar	ea (sf)	CN	Description								
*	1	15,960	98									
		24,290	79	50-75% Gras	50-75% Grass cover, Fair, HSG C							
		2,477	49	49 50-75% Grass cover, Fair, HSG A								
142,727 94 Weighted Average												
		26,767		18.75% Perv	ious Area							
	1	15,960		81.25% Impervious Area								
	Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description						
	1.1	75	0.015	50 1.12		Sheet Flow, A to B						
	2.8	333	0.015	50 1.97		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C						
_						Unpaved Kv= 16.1 fps						
	39	408	Total	Increased	to minimum	$T_{C} = 6.0 \text{ min}$						

Summary for Subcatchment S30B: Subcatchment 30B

Runoff = 19.04 cfs @ 12.09 hrs, Volume= 1.495 af, Depth= 3.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Ar	ea (sf)	CN	De	escription					
*	1	97,982	98							
		947	79	50-	-75% Gras	s cover, Fair	, HSG C			
		10,676	49	50-75% Grass cover, Fair, HSG A						
209,605 95 Weighted Average										
11,623 5.55% Pervious Area										
	1	97,982		94	.45% Impe	rvious Area				
	Tc	Length	Slop		Velocity	Capacity	Description			
_	(min)	(feet)	(ft/	'ft)	(ft/sec)	(cfs)				
	1.0	75	0.02	00	1.26		Sheet Flow, A to B			
							Smooth surfaces n= 0.011 P2= 3.00"			
	1.9	312	0.03	00	2.79		Shallow Concentrated Flow, B to C			
_							Unpaved Kv= 16.1 fps			
	2.9	387	Tota	ıl, İr	ncreased	to minimum	Tc = 6.0 min			

Summary for Subcatchment S31: Subcatchment 31

Runoff = 15.29 cfs @ 12.09 hrs, Volume= 1.266 af, Depth= 4.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

_	Αı	rea (sf)	CN D	escription			
*	1	62,835	98				
_	1	62,835	1	00.00% lmp	ervious Are	a	
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description	
	1.0	75	0.0200	1.26		Sheet Flow, A to B	
	5.1	700	0.0200	2.28		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C Unpaved Kv= 16.1 fps	
_	6.1	775	Total				

Summary for Subcatchment S32: Subcatchment 32

Runoff = 1.83 cfs @ 12.59 hrs, Volume= 0.334 af, Depth= 0.65"

Printed 2/2/2018 Page 21

	rea (sf)	CN	De	escription						
*	10,665	98								
	61,121	30	W	oods, Goo	od, HSG A					
	4,060	70	W	oods, God	od, HSG C					
	8,302	77	W	oods, God	od, HSG D					
	86,042	39	>7	75% Grass cover, Good, HSG A						
	3,152	61	>7	>75% Grass cover, Good, HSG B						
	35,319 74 >75% Grass cover, Good, HSG C									
	58,471 80 >75% Grass cover, Good, HSG D									
267,132 55 Weighted Average										
	256,467		96	.01% Pervi	ous Area					
	10,665		3.9	9% Imper	vious Area					
Tc	Length	Slop	эе	Velocity	Capacity	Description				
(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)					
16.9	75	0.00	80	0.07		Sheet Flow, A to B				
						Grass: Dense n= 0.240 P2= 3.00"				
16.0	600	0.00	80	0.63		Shallow Concentrated Flow, B to C				
						Short Grass Pasture Kv= 7.0 fps				
32.9	675	Toto	ıl							

Summary for Subcatchment \$40: Subcatchment 40

Runoff = 56.65 cfs @ 12.35 hrs, Volume= 6.587 af, Depth= 2.29"

	Area (sf)	CN	Description
*	429,894	98	
	200,076	77	Woods, Good, HSG D
	207,565	70	Woods, Good, HSG C
	52,912	55	Woods, Good, HSG B
	765	30	Woods, Good, HSG A
	329,379	80	>75% Grass cover, Good, HSG D
	120,178	74	>75% Grass cover, Good, HSG C
	135,075	61	>75% Grass cover, Good, HSG B
	26,357	39	>75% Grass cover, Good, HSG A
	1,502,201	80	Weighted Average
	1,072,307		71.38% Pervious Area
	429,894		28.62% Impervious Area

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 22

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	1.3	75	0.0100	0.95		Sheet Flow, A to B
						Smooth surfaces n= 0.011 P2= 3.00"
	6.0	776	0.0180	2.16		Shallow Concentrated Flow, B to C
						Unpaved Kv= 16.1 fps
	4.1	368	0.0462	1.50		Shallow Concentrated Flow, C to D
						Short Grass Pasture Kv= 7.0 fps
	13.6	1,500	0.0030	1.84	55.08	Channel Flow, D to E
						Area= 30.0 sf Perim= 35.0' r= 0.86'
						n= 0.040 Earth, cobble bottom, clean sides
_	25.0	2.719	Total			

Summary for Subcatchment S41: Subcatchment 41

Runoff = 25.97 cfs @ 12.10 hrs, Volume= 2.094 af, Depth= 3.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

_	Ar	ea (sf)	CN	De	scription						
*	2	25,409	98								
		22,827	79	50-	50-75% Grass cover, Fair, HSG C						
		37,988	84	50-	-75% Gras	s cover, Fair	r, HSG D				
*		7,519	98	Wo	ater Surfac	ce					
293,743 95 Weighted Average											
		60,815		20.	70% Pervi	ous Area					
	232,928 79.30% Impervious Area										
	Tc	Length	-			Capacity	Description				
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)					
	1.3	75	0.010	00	0.95		Sheet Flow, A to B				
							Smooth surfaces n= 0.011 P2= 3.00"				
	3.3	383	0.014	43	1.93		Shallow Concentrated Flow, B to C				
							Unpaved Kv= 16.1 fps				
	2.5	243	0.053	35	1.62		Shallow Concentrated Flow, C to D				
_							Short Grass Pasture Kv= 7.0 fps				
	7.1	701	Tota								

Summary for Subcatchment \$42: Subcatchment 42

Runoff = 25.71 cfs @ 12.15 hrs, Volume= 2.231 af, Depth= 3.31"

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

	Ar	ea (sf)	CN	De	escription							
*	2	78,726	98		•							
*		11,139	98	W	/ater Surface							
		26,977	49	50-	-75% Gras	s cover, Fair	, HSG A					
		29,325	69	50-	-75% Gras	s cover, Fair	, HSG B					
_	6,630 79 50-75% Grass cover, Fair, HSG C											
	352,797 91 Weighted Average											
	62,932 17.84% Pervious Area											
	2		82.16% Impervious Area									
	т.	مالمان ما ما	Cl		\/_l_ =:h	C ib .	Description					
	TC	Length	Slok		•	Capacity	Description					
_	(min)	(feet)	(ft/	TT)	(ft/sec)	(cfs)						
	1.3	75	0.010	00	0.95		Sheet Flow, A to B					
							Smooth surfaces n= 0.011 P2= 3.00"					
	6.1	700	0.01	40	1.90		Shallow Concentrated Flow, B to C					
							Unpaved Kv= 16.1 fps					
	3.4	225	0.02	50	1.11		Shallow Concentrated Flow, C to D					
_							Short Grass Pasture Kv= 7.0 fps					
	10.8	1,000	Tota	I								

Summary for Subcatchment \$43: Subcatchment 43

Runoff = 15.40 cfs @ 12.39 hrs, Volume= 1.880 af, Depth= 1.82"

	Area (s	f)	CN	De	escription			
*	232,05	2	98					
*	36,37	6	98	Wo	ater Surfac	ce		
	122,74	0	49	50-	-75% Grass	s cover, Fair	r, HSG A	
	38,16	0	79	50-	-75% Grass	s cover, Fair	r, HSG C	
	8,15	3	69	50-	-75% Grass	s cover, Fair	r, HSG B	
	86,00	0	36	Wo	oods, Fair,	HSG A		
	5,923 60 Woods, Fair, HSG B							
	10,54	9	77	Wo	ods, Poor	r, HSG C		
	539,953 74 Weighted Average					verage		
	271,52	5		50.	.29% Pervi	ous Area		
	268,42	8		49.	.71% Impe	rvious Area		
	Tc Leng	gth	Slop	ре	Velocity	Capacity	Description	
<u>(m</u>	nin) (fe	et)	(ft/	ft)	(ft/sec)	(cfs)		
1	0.6	75	0.010	00	0.12		Sheet Flow, A to B	
							Grass: Short n= 0.150 P2= 3.00"	
1	6.3 4	88	0.010	00	0.50		Shallow Concentrated Flow, B to C	
							Woodland Kv= 5.0 fps	
2	6.9 5	63	Tota			-		

Summary for Subcatchment S50: Subcatchment 50

Runoff = 2.32 cfs @ 12.09 hrs, Volume= 0.168 af, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Area (sf)	CN	Description						
*	10,546	98							
	4,431	61	>75% Grass cover, Good, HSG B						
	24,859	74	75% Grass cover, Good, HSG C						
	39,836	39,836 79 Weighted Average							
	29,290		73.53% Pervious Area						
	10,546		26.47% Impervious Area						
	Tc Length	Slo	pe Velocity Capacity Description						
(m	nin) (feet)	(ft,	/ft) (ft/sec) (cfs)						
	6.0		Direct Entry,						

Summary for Subcatchment S60: Subcatchment 60

Runoff = 6.85 cfs @ 13.88 hrs, Volume= 2.849 af, Depth= 0.47"

	Ar	ea (sf)	CN	De	scription				
*	1	68,411	98						
	3	01,647	49	50-	-75% Gras	s cover, Fair	r, HSG A		
	1,9	932,915 36 Woods, Fair, HSG A							
	135,432 73 Woods, Fair, HSG C								
	6	17,169	79	Wo	ods, Fair,	HSG D			
	3,155,574 51 \			We	eighted A	verage			
	2,987,163			94.	.66% Pervi	ous Area			
	168,411			5.34% Impervious Area					
	Tc	Length	Slop	е	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)			
	23.2	75	0.010	00	0.05		Sheet Flow, A to B		
							Woods: Light underbrush n= 0.400 P2= 3.00"		
	83.4	1,640	0.00	43	0.33		Shallow Concentrated Flow, B to C		
							Woodland Kv= 5.0 fps		
	1.1	330	0.010	00	5.10	122.30	Channel Flow, C to D		
							Area= 24.0 sf Perim= 23.0' r= 1.04'		
_							n= 0.030 Earth, grassed & winding		
	107.7	2,045	Tota	l					

<u>HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC</u>

Page 25

Summary for Reach R1: Portion of Davis Brook

Inflow Area = 27.238 ac, 66.69% Impervious, Inflow Depth > 0.84" for 10-Yr event

Inflow = 23.24 cfs @ 12.17 hrs, Volume= 1.904 af

Outflow = 9.50 cfs @ 12.74 hrs, Volume= 1.896 af, Atten= 59%, Lag= 33.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.37 fps, Min. Travel Time= 19.4 min Avg. Velocity = 0.33 fps, Avg. Travel Time= 80.3 min

Peak Storage= 11,137 cf @ 12.41 hrs Average Depth at Peak Storage= 0.78'

Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 176.49 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 1,600.0' Slope= 0.0031 '/'

Inlet Invert= 221.00', Outlet Invert= 216.00'



Summary for Reach R2: Portion of Davis Brook

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.33" for 10-Yr event

Inflow = 0.99 cfs @ 15.95 hrs, Volume= 0.565 af

Outflow = 0.98 cfs @ 16.37 hrs, Volume= 0.563 af, Atten= 1%, Lag= 25.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.71 fps, Min. Travel Time= 14.1 min Avg. Velocity = 0.27 fps, Avg. Travel Time= 37.0 min

Peak Storage= 831 cf @ 16.13 hrs

Average Depth at Peak Storage= 0.23'

Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 182.28 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 600.0' Slope= 0.0033 '/'

Inlet Invert= 223.00', Outlet Invert= 221.00'



1.00, 12.00, 11.00, 10.0

Summary for Reach R3: Natural Ditch

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.33" for 10-Yr event

Inflow = 0.99 cfs @ 15.85 hrs, Volume= 0.565 af

Outflow = 0.99 cfs @ 15.95 hrs, Volume= 0.565 af, Atten= 0%, Lag= 5.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.44 fps, Min. Travel Time= 3.5 min Avg. Velocity = 0.90 fps, Avg. Travel Time= 5.6 min

Peak Storage= 206 cf @ 15.89 hrs Average Depth at Peak Storage= 0.07'

Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 998.06 cfs

10.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 5.0 '/' Top Width= 40.00'

Length= 300.0' Slope= 0.0433 '/'

Inlet Invert= 236.00', Outlet Invert= 223.00'



Summary for Reach R4: Existing Ditch

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 0.00" for 10-Yr event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 168.09 cfs

8.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 20.00'

Length= 190.0' Slope= 0.0053 '/'

Inlet Invert= 244.00', Outlet Invert= 243.00'



HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 27

Summary for Reach R5: Natural Ditch

6.743 ac, 79.30% Impervious, Inflow Depth > 2.39" for 10-Yr event Inflow Area =

Inflow 33.14 cfs @ 12.11 hrs, Volume= 1.341 af

23.24 cfs @ 12.17 hrs, Volume= Outflow 1.341 af, Atten= 30%, Lag= 4.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.16 fps, Min. Travel Time= 1.3 min Avg. Velocity = 0.76 fps, Avg. Travel Time= 5.5 min

Peak Storage= 2,075 cf @ 12.15 hrs Average Depth at Peak Storage= 0.30'

Bank-Full Depth= 3.00' Flow Area= 165.0 sf, Capacity= 1,945.52 cfs

25.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 10.0 '/' Top Width= 85.00'

Length= 250.0' Slope= 0.0320 '/'

Inlet Invert= 229.00', Outlet Invert= 221.00'



Summary for Pond 30A: Grassed Underdrained Soil Filter A

Inflow Area = 3.277 ac, 81.25% Impervious, Inflow Depth = 3.62" for 10-Yr event

12.75 cfs @ 12.09 hrs, Volume= Inflow 0.988 af

12.75 cfs @ 12.09 hrs, Volume= 0.988 af, Atten= 0%, Lag= 0.0 min Primary

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 30B: Grassed Underdrained Soil Filter B

Inflow Area = 4.812 ac, 94.45% Impervious, Inflow Depth = 3.73" for 10-Yr event

19.04 cfs @ 12.09 hrs, Volume= 1.495 af Inflow

19.04 cfs @ 12.09 hrs, Volume= 1.495 af, Atten= 0%, Lag= 0.0 min Primary

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 101: Pond #1 (plus wetland storage)

Inflow Area = 35.428 ac, 69.71% Impervious, Inflow Depth = 2.42" for 10-Yr event

38.13 cfs @ 12.09 hrs, Volume= Inflow 7.156 af

5.257 af, Atten= 89%, Lag= 222.9 min Outflow = 4.20 cfs @ 15.81 hrs, Volume=

Primary 4.20 cfs @ 15.81 hrs, Volume= 5.257 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 28

Peak Elev= 241.87' @ 15.81 hrs Surf.Area= 208,056 sf Storage= 210,950 cf

Plug-Flow detention time=735.7 min calculated for 5.253 af (73% of inflow) Center-of-Mass det. time=645.8 min (1,483.3 - 837.5)

Volume	Inver	t Avail.Stor	age Stora	ge Description	
#1	240.00	835,45	58 cf Custo	m Stage Data (Prismatic) Listed below (Recalc)
Elevatior (feet		rf.Area (sq-ft) (c	Inc.Store	Cum.Store (cubic-feet)	
240.00)	22,686	0	0	
241.00) '	15,923	69,305	69,305	
242.00) 2	221,289	168,606	237,911	
243.00) (313,044	267,167	505,077	
244.00) (347,717	330,381	835,458	
Device	Routing	Invert	Outlet De	vices	
#1	Primary	237.64	24.0" Rou	nd Culvert	
			Inlet / Out	tlet Invert= 237.6	, no headwall, Ke= 0.900 64' / 237.30' S= 0.0039 '/' Cc= 0.900 tal, Flow Area= 3.14 sf
#2	Device 1	241.00'	24.0" Vert.	Orifice/Grate	C= 0.600

Primary OutFlow Max=4.20 cfs @ 15.81 hrs HW=241.87' (Free Discharge) 1=Culvert (Passes 4.20 cfs of 16.64 cfs potential flow)

12=Orifice/Grate (Orifice Controls 4.20 cfs @ 3.18 fps)

Summary for Pond 102: Pond #2

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 1.82" for 10-Yr event

Inflow = 15.40 cfs @ 12.39 hrs, Volume= 1.880 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 244.67' @ 25.60 hrs Surf.Area= 36,354 sf Storage= 81,885 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

82,276

107,844

Center-of-Mass det. time= (not calculated: no outflow)

36,400

127,000

244.68

246.00

Volume	Invert	Avail.	Storage	Storag	e Description	
#1	242.00'	19	0,120 cf	Custon	n Stage Data (I	Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.A	Area a-ft)	Inc.S	Store	Cum.Store (cubic-feet)	
	<u> </u>		(CUDIC-	^	(CODIC-IGEI)	
242.00	25	,000		U	U	

82,276

190,120

50465_predev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 29

Device	Routing	Invert	Outlet Devices
#1	Primary	246.53'	18.0" Round Culvert
			L= 165.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 246.53' / 244.00' S= 0.0153 '/' Cc= 0.900 n= 0.025 Corrugated metal, Flow Area= 1.77 sf
#2	Device 1	248.05'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=242.00' (Free Discharge)

1=Culvert (Controls 0.00 cfs)

2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 103: Pond #3

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth = 1.31" for 10-Yr event

Inflow = 25.71 cfs @ 12.15 hrs, Volume= 2.231 af

Outflow = 0.99 cfs @ 15.85 hrs, Volume= 0.565 af, Atten= 96%, Lag= 222.3 min

Primary = 0.99 cfs @ 15.85 hrs, Volume= 0.565 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 241.42' @ 15.85 hrs Surf.Area= 24,457 sf Storage= 78,686 cf

Plug-Flow detention time=725.6 min calculated for 0.565 af (25% of inflow)

Center-of-Mass det. time=567.8 min (1,364.2 - 796.3)

Volume	Invert	Avail.Storage	Storage Description
#1	234.00'	94,130 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
234.00	4,000	0	0
236.00	7,000	11,000	11,000
238.92	11,139	26,483	37,483
240.00	14,287	13,730	51,213
242.00	28,630	42,917	94,130

Device	Routing	Invert	Outlet Devices				
#1	Primary	234.22'	24.0" Round Culvert				
	•		L= 95.0' CMP, projecting, no headwall, Ke= 0.900				
			Inlet / Outlet Invert= 234.22' / 233.90' S= 0.0034 '/' Cc= 0.900				
			n=0.025 Corrugated metal, Flow Area=3.14sf				
#2	Device 1	241.29'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads				
#3	Device 1	236.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600				

Primary OutFlow Max=0.96 cfs @ 15.85 hrs HW=241.42' (Free Discharge)

-1=Culvert (Passes 0.96 cfs of 23.64 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.94 cfs @ 1.17 fps)

3=Orifice/Grate (Orifice Controls 0.02 cfs @ 11.19 fps)

236.00

Printed 2/2/2018

Page 30

Summary for Pond 104: Pond #4

Inflow Area = 6.743 ac, 79.30% Impervious, Inflow Depth = 3.73" for 10-Yr event

Inflow 25.97 cfs @ 12.10 hrs, Volume= 2.094 af

33.14 cfs @ 12.11 hrs, Volume= Outflow 1.341 af, Atten= 0%, Lag= 0.5 min

Primary 33.14 cfs @ 12.11 hrs, Volume= 1.341 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.32' @ 12.11 hrs Surf.Area= 16,786 sf Storage= 36,018 cf

Plug-Flow detention time=298.7 min calculated for 1.341 af (64% of inflow)

12,937

Center-of-Mass det. time=199.0 min (972.3 - 773.3)

16,786

Volume	Invert ,	Avail.Storage	Storag	e Description	
#1	230.00'	36,018 cf	Custon	n Stage Data (F	Prismatic) Listed below (Recalc)
	0.5.		0.1		
Elevation	Surf.Ar	rea Inc.	Store	Cum.Store	
(feet)	(sq	-ft) (cubic-	-feet)	(cubic-feet)	
230.00	6	320	0	0	
232.00	3,3	391	4,011	4,011	
234.05	7,5	519 1	1,183	15,194	
235.00	9.0	087	7,888	23,082	

36,018

Device	Routing	Invert	Outlet Devices
#1	Primary	230.62'	24.0" Round Culvert
			L= 96.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 230.62' / 228.40' S= 0.0231 '/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=3.14sf
#2	Device 1	237.61'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	231.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=30.43 cfs@12.11 hrs HW=241.65' (Free Discharge)

-1=Culvert (Passes 30.43 cfs of 33.62 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 30.41 cfs @ 9.68 fps)

3=Orifice/Grate (Orifice Controls 0.03 cfs @ 15.70 fps)

Summary for Pond 106: Existing Mining Pit

Inflow Area = 3.738 ac,100.00% Impervious, Inflow Depth = 4.06" for 10-Yr event

Inflow 15.29 cfs @ 12.09 hrs, Volume= 1.266 af

Outflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min =

0.00 cfs @ 0.00 hrs, Volume= Primary 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 247.34' @ 24.40 hrs Surf.Area= 48,706 sf Storage= 55,147 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Type III 24-hr 10-Yr Rainfall=4.30" Printed 2/2/2018

50465_predev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

<u> Page 31</u>

Volume	Inver	t Avail.Sto	orage :	Storage D	escription	
#1	245.00)' 215,8	850 cf	Custom Sto	age Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Su	rf.Area (sq-ft)	Inc.St cubic-fe		Cum.Store ubic-feet)	
245.00		327		0	0	
246.00		18,653	9,	490	9,490	
247.00		41,546	30,	100	39,590	
248.00		62,315	51,	931	91,520	
249.00		73,054	67,	685	159,205	
249.75		78,000	56,	645	215,850	
	Routing	Inve		et Devices		Due and Considered Decadement with the State of the State
#1 F	Primary	249.7		•		Broad-Crested Rectangular Weir 60 0.80 1.00 1.20 1.40 1.60

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=245.00' (Free Discharge)
1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 301: Point of Analysis 301

Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.63

Inflow Are	a =	41.560 ac,	60.02% Impe	ervious, Intlow [Depth > 1.61"	tor 10-Yr event
Inflow	=	4.55 cfs @	15.40 hrs, V	olume=	5.591 af	
Primary	=	4.55 cfs @	15.40 hrs, V	olume=	5.591 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 401: Point of Analysis 401

Inflow Are	a =	61.724 ac,	45.42% lmp	pervious,	Inflow Depth > 1	.65" for 10)-Yr event
Inflow	=	56.67 cfs @	12.35 hrs,	Volume=	= 8.483 af		
Primary	=	56.67 cfs @	12.35 hrs,	Volume=	= 8.483 af,	Atten= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 501: Point of Analysis 501

Inflow Are	ea =	0.915 ac, 1	26.47% Imp	pervious,	Inflow Depth =	2.21"	for 10)-Yr event	
Inflow	=	2.32 cfs @	12.09 hrs,	Volume:	= 0.168 af				
Primary	=	2.32 cfs @	12.09 hrs.	Volume:	= 0.168 af	. Atten	= 0%.	Laa= 0.0 m	in

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 601: Point of Analysis 601

Inflow Are	ea =	72.442 ac,	5.34% Impervious, Inflo	ow Depth = 0.47"	for 10-Yr event
Inflow	=	6.85 cfs @	13.88 hrs, Volume=	2.849 af	
Primary	=	6 85 cfs @	13.88 hrs Volume=	2.849 af Atte	n = 0% Lag = 0.0 min

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 32

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Subcatchment S30: Subcatchment 30

Runoff = 38.36 cfs @ 12.74 hrs, Volume= 6.572 af, Depth= 3.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Ar	ea (sf)	CN	Des	scription							
	1	51,673	30	Wo	ods, Goo	d, HSG A						
	1	29,474	70	Wo	oods, Good, HSG C							
	1	08,224	79	50-	0-75% Grass cover, Fair, HSG C							
		39,615	49	50-75% Grass cover, Fair, HSG A								
		22,394	98	Wa	iter Surfac	ce, HSG C						
*	5	76,689	98									
	1,028,069 81 Weighted Average											
		28,986			73% Pervi							
	599,083 58.27% Impervious Area											
	_											
	TC	Length	Slop		·	Capacity	Description					
_	(min)	(feet)	(ft/1		(ft/sec)	(cfs)						
	0.8	75	0.040	00	1.66		Sheet Flow, A to B					
							Smooth surfaces n= 0.011 P2= 3.00"					
	4.9	481	0.010)4	1.64		Shallow Concentrated Flow, B to C					
							Unpaved Kv= 16.1 fps					
	49.2	1,094	0.005	55	0.37		Shallow Concentrated Flow, C to D					
_							Woodland Kv= 5.0 fps					
	54.9	1.650	Total	I								

Summary for Subcatchment S30A: Subcatchment 30A

Runoff = 16.32 cfs @ 12.09 hrs, Volume= 1.284 af, Depth= 4.70"

_	Ar	ea (sf)	CN	Description								
*	1	15,960	98									
		24,290	79	50-75% Grass cover, Fair, HSG C								
	2,477 49 50-75% Grass cover, Fair, HSG A											
142,727 94 Weighted Average												
		26,767		18.75% Perv								
	1	15,960		81.25% Impervious Area								
	Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description						
	1.1	75	0.015	50 1.12		Sheet Flow, A to B						
	2.8	333	0.015	50 1.97		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C						
_						Unpaved Kv= 16.1 fps						
	39	408	Total	Increased	to minimum	$T_{C} = 6.0 \text{ min}$						

Summary for Subcatchment S30B: Subcatchment 30B

Runoff = 24.25 cfs @ 12.09 hrs, Volume= 1.931 af, Depth= 4.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Ar	ea (sf)	CN	De	escription							
*	1	97,982	98									
		947	79	50-	50-75% Grass cover, Fair, HSG C							
		10,676 49 50-75% Grass cover, Fair, HSG A										
	209,605 95 Weighted Average											
	11,623 5.55% Pervious Area											
197,982 94.45% Impervious Area												
	Tc	Length	Slop		Velocity	Capacity	Description					
_	(min)	(feet)	(ft/	'ft)	(ft/sec)	(cfs)						
	1.0	75	0.02	00	1.26		Sheet Flow, A to B					
							Smooth surfaces n= 0.011 P2= 3.00"					
	1.9	312	0.03	00	2.79		Shallow Concentrated Flow, B to C					
_							Unpaved Kv= 16.1 fps					
	2.9	387	Tota	ıl, İr	ncreased	to minimum	Tc = 6.0 min					

Summary for Subcatchment S31: Subcatchment 31

Runoff = 19.26 cfs @ 12.09 hrs, Volume= 1.608 af, Depth= 5.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Αı	rea (sf)	CN [Description			
*	1	62,835	98				
	162,835		1	00.00% Imp	ervious Are	a	
	Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description	
	1.0	75	0.0200	1.26		Sheet Flow, A to B	
	5.1	700	0.0200	2.28		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C Unpaved Kv= 16.1 fps	
	6.1	775	Total			<u> </u>	

Summary for Subcatchment S32: Subcatchment 32

Runoff = 3.92 cfs @ 12.54 hrs, Volume= 0.606 af, Depth= 1.19"

50465_predev

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 35

	Area (sf)	CN	De	Description Description						
*	10,665	98								
	61,121	30	W	oods, God	od, HSG A					
	4,060	70	W	oods, Goo	od, HSG C					
	8,302	77	W	oods, God	od, HSG D					
	86,042	39	>7	5% Grass of	cover, Good	d, HSG A				
	3,152	61	>7	5% Grass of	cover, Good	d, HSG B				
	35,319	74	>7	5% Grass of	cover, Goo	d, HSG C				
	58,471 80 >75% Grass cover, Good, HSG D									
	267,132	55	We	eighted A	verage					
	256,467		96	.01% Pervi	ous Area					
	10,665		3.99% Impervious Area							
T	c Length			Velocity		Description				
(mir	<u>ı) (feet)</u>	(ft)	/ft)	(ft/sec)	(cfs)					
16.	9 75	0.00	080	0.07		Sheet Flow, A to B				
						Grass: Dense n= 0.240 P2= 3.00"				
16.	0 600	0.00	080	0.63		Shallow Concentrated Flow, B to C				
						Short Grass Pasture Kv= 7.0 fps				
32.	9 675	Toto	lc							

Summary for Subcatchment \$40: Subcatchment 40

Runoff = 80.22 cfs @ 12.35 hrs, Volume= 9.324 af, Depth= 3.24"

	Area (sf)	CN	Description
*	429,894	98	
	200,076	77	Woods, Good, HSG D
	207,565	70	Woods, Good, HSG C
	52,912	55	Woods, Good, HSG B
	765	30	Woods, Good, HSG A
	329,379	80	>75% Grass cover, Good, HSG D
	120,178	74	>75% Grass cover, Good, HSG C
	135,075	61	>75% Grass cover, Good, HSG B
	26,357	39	>75% Grass cover, Good, HSG A
	1,502,201	80	Weighted Average
	1,072,307		71.38% Pervious Area
	429,894		28.62% Impervious Area

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 36

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.3	75	0.0100	0.95		Sheet Flow, A to B
					Smooth surfaces n= 0.011 P2= 3.00"
6.0	776	0.0180	2.16		Shallow Concentrated Flow, B to C
					Unpaved Kv= 16.1 fps
4.1	368	0.0462	1.50		Shallow Concentrated Flow, C to D
					Short Grass Pasture Kv= 7.0 fps
13.6	1,500	0.0030	1.84	55.08	Channel Flow, D to E
					Area= 30.0 sf Perim= 35.0' r= 0.86'
					n= 0.040 Earth, cobble bottom, clean sides
25.0	2,719	Total			

Summary for Subcatchment S41: Subcatchment 41

Runoff = 33.08 cfs @ 12.10 hrs, Volume= 2.706 af, Depth= 4.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

_	Ar	ea (sf)	CN	De	scription							
*	2	25,409	98									
		22,827 79 50-75% Grass cover, Fair, HSG C										
		37,988	84	50-	-75% Gras	s cover, Fair	r, HSG D					
*		7,519	98	Wo	ater Surfac	ce						
	2	93,743	95	We	eighted A	verage						
		60,815		20.	.70% Pervi	ous Area						
	2	79.30% Impervious Area										
	Tc	Length	-		Velocity	Capacity	Description					
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)						
	1.3	75	0.01	00	0.95		Sheet Flow, A to B					
							Smooth surfaces n= 0.011 P2= 3.00"					
	3.3	383	0.01	43	1.93		Shallow Concentrated Flow, B to C					
							Unpaved Kv= 16.1 fps					
	2.5	243	0.05	35	1.62		Shallow Concentrated Flow, C to D					
_							Short Grass Pasture Kv= 7.0 fps					
	7.1	701	Tota	I								

Summary for Subcatchment \$42: Subcatchment 42

Runoff = 33.51 cfs @ 12.15 hrs, Volume= 2.950 af, Depth= 4.37"

50465_predev

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 37

	Ar	ea (sf)	CN	De	scription						
*	2	78,726	98								
*		11,139	98	W	ater Surfac	ce					
		26,977	49	50-	-75% Gras	s cover, Fair	r, HSG A				
		29,325	69	50-	-75% Gras	s cover, Fair	r, HSG B				
_		6,630	79	50	-75% Gras	s cover, Fair	r, HSG C				
	352,797 91			We	Weighted Average						
	62,932				17.84% Pervious Area						
	289,865			82.16% Impervious Area							
	Tc (min)	Length (feet)	Slop (ft/		Velocity (ft/sec)	Capacity (cfs)	Description				
	1.3	75	0.010	00	0.95		Sheet Flow, A to B				
	6.1 3.4	700 225	0.01		1.90 1.11		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C Unpaved Kv= 16.1 fps Shallow Concentrated Flow, C to D				
	5.4	223	0.02	JU	1,11		Short Grass Pasture Kv= 7.0 fps				
_	10.8	1,000	Tota	l			5 5				

Summary for Subcatchment \$43: Subcatchment 43

Runoff = 23.05 cfs @ 12.38 hrs, Volume= 2.776 af, Depth= 2.69"

	Area	(sf)	CN	De	scription					
*	232,	052	98							
*	36,	376	98	Wo	ater Surfac	ce				
	122,	740	49	50-	-75% Gras	s cover, Fair	r, HSG A			
	38,	160	79	50-	-75% Gras	s cover, Fair	r, HSG C			
	8,	153	69	50-	-75% Gras	s cover, Fair	r, HSG B			
	86,	000	36	Wo	oods, Fair,	HSG A				
	5,	923	60	Wo	oods, Fair,	HSG B				
	10,	549	77	Wo	ods, Poo	r, HSG C				
	539,953 74 Weighted Average									
	271,	525	50.29% Pervious Area							
	268,	428		49.71% Impervious Area						
	Tc Le	ength	Slop	ре	Velocity	Capacity	Description			
(m	nin) (feet)	(ft/	ft)	(ft/sec)	(cfs)				
1	0.6	75	0.010	00	0.12		Sheet Flow, A to B			
							Grass: Short n= 0.150 P2= 3.00"			
1	6.3	488	0.010	00	0.50		Shallow Concentrated Flow, B to C			
							Woodland Kv= 5.0 fps			
2	6.9	563	Tota	l						

Summary for Subcatchment S50: Subcatchment 50

Runoff = 3.31 cfs @ 12.09 hrs, Volume= 0.240 af, Depth= 3.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Α	rea (sf)	CN	Descriptio	n								
-	*	10,546	98										
		4,431	61	>75% Gras	>75% Grass cover, Good, HSG B								
		24,859	74	>75% Gras	75% Grass cover, Good, HSG C								
_		39,836 79 Weighted Average											
		29,290		73.53% Pervious Area									
		10,546		26.47% lm	pervious Are	а							
	Tc	Length		•	ty Capacity	•							
	(min)	(feet)	(ft,	/ft) (ft/sec	c) (cfs)								
	6.0					Direct Entry							

Summary for Subcatchment S60: Subcatchment 60

Runoff = 16.23 cfs @ 13.74 hrs, Volume= 5.582 af, Depth= 0.92"

	Ar	ea (sf)	CN	De	scription						
*	1	68,411	98								
	3	49	50-75% Grass cover, Fair, HSG A								
	1,9	36	Wo	Woods, Fair, HSG A							
	1	35,432	73	Woods, Fair, HSG C							
	6	17,169	79	Wo	ods, Fair,	s, Fair, HSG D					
	3,155,574		51	We	Weighted Average						
	2,9		94.	.66% Pervi	ous Area						
	1		5.34% Impervious Area								
	Tc	Length	Slop	е	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)					
	23.2	75	0.010	00	0.05		Sheet Flow, A to B				
							Woods: Light underbrush n= 0.400 P2= 3.00"				
	83.4	1,640	0.00	43	0.33		Shallow Concentrated Flow, B to C				
							Woodland Kv= 5.0 fps				
	1.1	330	0.010	00	5.10	122.30	Channel Flow, C to D				
							Area= 24.0 sf Perim= 23.0' r= 1.04'				
_							n= 0.030 Earth, grassed & winding				
	107.7	2,045	Tota	l							

<u>HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC</u>

Page 39

Summary for Reach R1: Portion of Davis Brook

Inflow Area = 27.238 ac, 66.69% Impervious, Inflow Depth > 1.43" for 25-Yr event

Inflow = 31.35 cfs @ 12.14 hrs, Volume= 3.235 af

Outflow = 17.56 cfs @ 12.57 hrs, Volume= 3.227 af, Atten= 44%, Lag= 26.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.62 fps, Min. Travel Time= 16.5 min Avg. Velocity = 0.36 fps, Avg. Travel Time= 75.0 min

Peak Storage= 17,396 cf @ 12.29 hrs Average Depth at Peak Storage= 1.06'

Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 176.49 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 1,600.0' Slope= 0.0031 '/'

Inlet Invert= 221.00', Outlet Invert= 216.00'



Summary for Reach R2: Portion of Davis Brook

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.75" for 25-Yr event

Inflow = 3.52 cfs @ 13.12 hrs, Volume= 1.284 af

Outflow = 3.44 cfs @ 13.44 hrs, Volume= 1.282 af, Atten= 2%, Lag= 19.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.05 fps, Min. Travel Time= 9.5 min Avg. Velocity = 0.31 fps, Avg. Travel Time= 32.4 min

Peak Storage= 1,971 cf @ 13.28 hrs Average Depth at Peak Storage= 0.45'

Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 182.28 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 600.0' Slope= 0.0033 '/'

Inlet Invert= 223.00', Outlet Invert= 221.00'



HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 40

Summary for Reach R3: Natural Ditch

20.495 ac, 62.54% Impervious, Inflow Depth > 0.75" for 25-Yr event Inflow Area =

Inflow 3.53 cfs @ 13.05 hrs. Volume= 1.284 af

3.52 cfs @ 13.12 hrs, Volume= Outflow 1.284 af, Atten= 0%, Lag= 3.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.31 fps, Min. Travel Time= 2.2 min Avg. Velocity = 0.97 fps, Avg. Travel Time= 5.2 min

Peak Storage= 457 cf @ 13.08 hrs Average Depth at Peak Storage= 0.14'

Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 998.06 cfs

10.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 5.0 '/' Top Width= 40.00'

Length= 300.0' Slope= 0.0433 '/'

Inlet Invert= 236.00', Outlet Invert= 223.00'



Summary for Reach R4: Existing Ditch

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 0.00" for 25-Yr event

Inflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

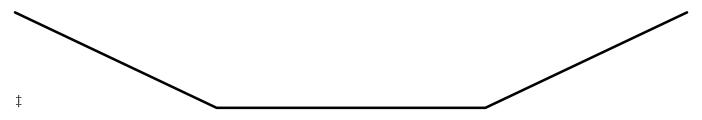
Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 168.09 cfs

8.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 20.00'

Length= 190.0' Slope= 0.0053 '/'

Inlet Invert= 244.00', Outlet Invert= 243.00'



HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Summary for Reach R5: Natural Ditch

Inflow Area = 6.743 ac, 79.30% Impervious, Inflow Depth > 3.47" for 25-Yr event

Inflow = 40.34 cfs @ 12.05 hrs, Volume= 1.952 af

Outflow = 31.35 cfs @ 12.14 hrs, Volume= 1.952 af, Atten= 22%, Lag= 5.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.40 fps, Min. Travel Time= 1.2 min Avg. Velocity = 0.77 fps, Avg. Travel Time= 5.4 min

Peak Storage= 2,390 cf @ 12.11 hrs Average Depth at Peak Storage= 0.34'

Bank-Full Depth= 3.00' Flow Area= 165.0 sf, Capacity= 1,945.52 cfs

25.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 10.0 '/' Top Width= 85.00'

Length= 250.0' Slope= 0.0320 '/'

Inlet Invert= 229.00', Outlet Invert= 221.00'



Summary for Pond 30A: Grassed Underdrained Soil Filter A

Inflow Area = 3.277 ac, 81.25% Impervious, Inflow Depth = 4.70" for 25-Yr event

Inflow = 16.32 cfs @ 12.09 hrs, Volume= 1.284 af

Primary = 16.32 cfs @ 12.09 hrs, Volume= 1.284 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 30B: Grassed Underdrained Soil Filter B

Inflow Area = 4.812 ac, 94.45% Impervious, Inflow Depth = 4.82" for 25-Yr event

Inflow = 24.25 cfs @ 12.09 hrs, Volume= 1.931 af

Primary = 24.25 cfs @ 12.09 hrs, Volume= 1.931 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 101: Pond #1 (plus wetland storage)

Inflow Area = 35.428 ac, 69.71% Impervious, Inflow Depth = 3.32" for 25-Yr event

Inflow = 50.23 cfs @ 12.09 hrs, Volume= 9.787 af

Outflow = 6.99 cfs @ 15.07 hrs, Volume= 7.871 af, Atten= 86%, Lag= 178.4 min

Primary = 6.99 cfs @ 15.07 hrs, Volume= 7.871 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 42

Peak Elev= 242.17' @ 15.07 hrs Surf.Area= 236,476 sf Storage= 275,795 cf

Plug-Flow detention time= 652.8 min calculated for 7.871 af (80% of inflow)

Center-of-Mass det. time=576.1 min (1,406.8 - 830.7)

Volume	Invert	Avail.Stor	age Storaç	ge Description	
#1	240.00'	835,45	8 cf Custo	m Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet		.Area (sq-ft) (cı	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	
240.00) 2	2,686	0	0	
241.00) 11	5,923	69,305	69,305	
242.00) 22	1,289	168,606	237,911	
243.00	31	3,044	267,167	505,077	
244.00	34	7,717	330,381	835,458	
Device	Routing	Invert	Outlet De	vices	
#1	Primary	237.64'	24.0" Roui	nd Culvert	
#2	Device 1	241.00'	L= 87.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 237.64' / 237.30' S= 0.0039'/' Cc= 0.90 n= 0.025 Corrugated metal, Flow Area= 3.14 sf 20' 24.0" Vert. Orifice/Grate C= 0.600		
πΖ	DC VICE I	241.00	27.0 V CII.	Crince/Grale	C- 0.000

Primary OutFlow Max=6.98 cfs @ 15.07 hrs HW=242.17' (Free Discharge) 1=Culvert (Passes 6.98 cfs of 17.56 cfs potential flow)

2=Orifice/Grate (Orifice Controls 6.98 cfs @ 3.68 fps)

Summary for Pond 102: Pond #2

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 2.69" for 25-Yr event

Inflow = 23.05 cfs @ 12.38 hrs, Volume= 2.776 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 245.34' @ 25.60 hrs Surf.Area= 81,374 sf Storage= 120,862 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	242.00'	190,120 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.00	25,000	0	0
244.68	36,400	82,276	82,276
246.00	127,000	107,844	190,120

50465_predev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 43

Device	Routing	Invert	Outlet Devices
#1	Primary	246.53'	18.0" Round Culvert
			L= 165.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 246.53' / 244.00' S= 0.0153'/' Cc= 0.900 n= 0.025 Corrugated metal, Flow Area= 1.77 sf
#2	Device 1	248.05'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=242.00' (Free Discharge)

-1=Culvert (Controls 0.00 cfs)

2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 103: Pond #3

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth = 1.73" for 25-Yr event

Inflow = 33.51 cfs @ 12.15 hrs, Volume= 2.950 af

Outflow = 3.53 cfs @ 13.05 hrs, Volume= 1.284 af, Atten= 89%, Lag= 54.4 min

Primary = 3.53 cfs @ 13.05 hrs, Volume= 1.284 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 241.60' @ 13.05 hrs Surf.Area= 25,745 sf Storage= 83,192 cf

Plug-Flow detention time= 426.6 min calculated for 1.283 af (43% of inflow)

Center-of-Mass det. time=305.3 min (1,094.1 - 788.8)

Volume	Invert	Avail.Storage	Storage Description
#1	234.00'	94,130 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
234.00	4,000	0	0
236.00	7,000	11,000	11,000
238.92	11,139	26,483	37,483
240.00	14,287	13,730	51,213
242.00	28,630	42,917	94,130

Device	Routing	Invert	Outlet Devices			
#1	Primary	234.22'	24.0" Round Culvert			
	•		L= 95.0' CMP, projecting, no headwall, Ke= 0.900			
			Inlet / Outlet Invert= 234.22' / 233.90' S= 0.0034 '/' Cc= 0.900			
			n=0.025 Corrugated metal, Flow Area=3.14sf			
#2	Device 1	241.29'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads			
#3	Device 1	236.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600			

Primary OutFlow Max=3.53 cfs @ 13.05 hrs HW=241.60' (Free Discharge)

-1=Culvert (Passes 3.53 cfs of 24.02 cfs potential flow)

2=Orifice/Grate (Weir Controls 3.51 cfs @ 1.81 fps)

3=Orifice/Grate (Orifice Controls 0.02 cfs @ 11.37 fps)

Printed 2/2/2018

Page 44

Summary for Pond 104: Pond #4

Inflow Area = 6.743 ac, 79.30% Impervious, Inflow Depth = 4.82" for 25-Yr event

Inflow = 33.08 cfs @ 12.10 hrs, Volume= 2.706 af

Outflow = 40.34 cfs @ 12.05 hrs, Volume= 1.952 af, Atten= 0%, Lag= 0.0 min

Primary = 40.34 cfs @ 12.05 hrs, Volume= 1.952 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 246.59' @ 12.05 hrs Surf.Area= 16,786 sf Storage= 36,018 cf

Plug-Flow detention time= 239.6 min calculated for 1.952 af (72% of inflow)

Center-of-Mass det. time= 150.0 min (917.3 - 767.3)

Volume	Invert	Avail.Storage	Storage Description
#1	230.00'	36,018 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

E	Elevation	Surf.Area	Inc.Store	Cum.Store
_	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
	230.00	620	0	0
	232.00	3,391	4,011	4,011
	234.05	7,519	11,183	15,194
	235.00	9,087	7,888	23,082
	236.00	16,786	12,937	36,018

Device	Routing	Invert	Outlet Devices
#1	Primary	230.62'	24.0" Round Culvert
			L= 96.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 230.62' / 228.40' S= 0.0231 '/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=3.14sf
#2	Device 1	237.61'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	231.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=40.34 cfs @ 12.05 hrs HW=246.59' (Free Discharge)

_1=Culvert (Barrel Controls 40.34 cfs @ 12.84 fps)

2=Orifice/Grate (Passes < 45.33 cfs potential flow)

-3=Orifice/Grate (Passes < 0.03 cfs potential flow)

Summary for Pond 106: Existing Mining Pit

Inflow Area = 3.738 ac,100.00% Impervious, Inflow Depth = 5.16" for 25-Yr event

Inflow = 19.26 cfs @ 12.09 hrs, Volume= 1.608 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 247.63' @ 24.40 hrs Surf.Area= 54,693 sf Storage= 70,048 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Type III 24-hr 25-Yr Rainfall=5.40" Printed 2/2/2018

50465_predev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 45

Volume	Inve	t Avail.Sto	rage St	orage Des	cription	
#1	245.00)' 215,8	50 cf C (ustom Stag	e Data (F	rismatic) Listed below (Recalc)
Elevation (feet)		urf.Area (sq-ft) (Inc.Sto		m.Store bic-feet)	
245.00		327		0	0	
246.00		18,653	9,49	90	9,490	
247.00		41,546	30,10	00	39,590	
248.00		62,315	51,93	31	91,520	
249.00		73,054	67,68	35	159,205	
249.75		78,000	56,64	45	215,850	
	Routing	Inver		Devices		
#1	Primary	249.75		•		Broad-Crested Rectangular Weir 0 0.80 1.00 1.20 1.40 1.60

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=245.00' (Free Discharge)
1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 301: Point of Analysis 301

Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Inflow Area = 41.560 ac, 60.02% Impervious, Inflow Depth > 2.45" for 25-Yr event

Inflow = 7.66 cfs @ 14.72 hrs, Volume= 8.477 af

Primary = 7.66 cfs @ 14.72 hrs, Volume= 8.477 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 401: Point of Analysis 401

Inflow Area = 61.724 ac, 45.42% Impervious, Inflow Depth > 2.44" for 25-Yr event

Inflow = 88.24 cfs @ 12.41 hrs, Volume= 12.552 af

Primary = 88.24 cfs @ 12.41 hrs, Volume= 12.552 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 501: Point of Analysis 501

Inflow Area = 0.915 ac, 26.47% Impervious, Inflow Depth = 3.15" for 25-Yr event

Inflow = 3.31 cfs @ 12.09 hrs, Volume= 0.240 af

Primary = 3.31 cfs @ 12.09 hrs, Volume= 0.240 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 601: Point of Analysis 601

Inflow Area = 72.442 ac, 5.34% Impervious, Inflow Depth = 0.92" for 25-Yr event

Inflow = 16.23 cfs @ 13.74 hrs, Volume= 5.582 af

Primary = 16.23 cfs @ 13.74 hrs, Volume= 5.582 af, Atten= 0%, Lag= 0.0 min

Prepared by Stantec Consulting Services, Inc.

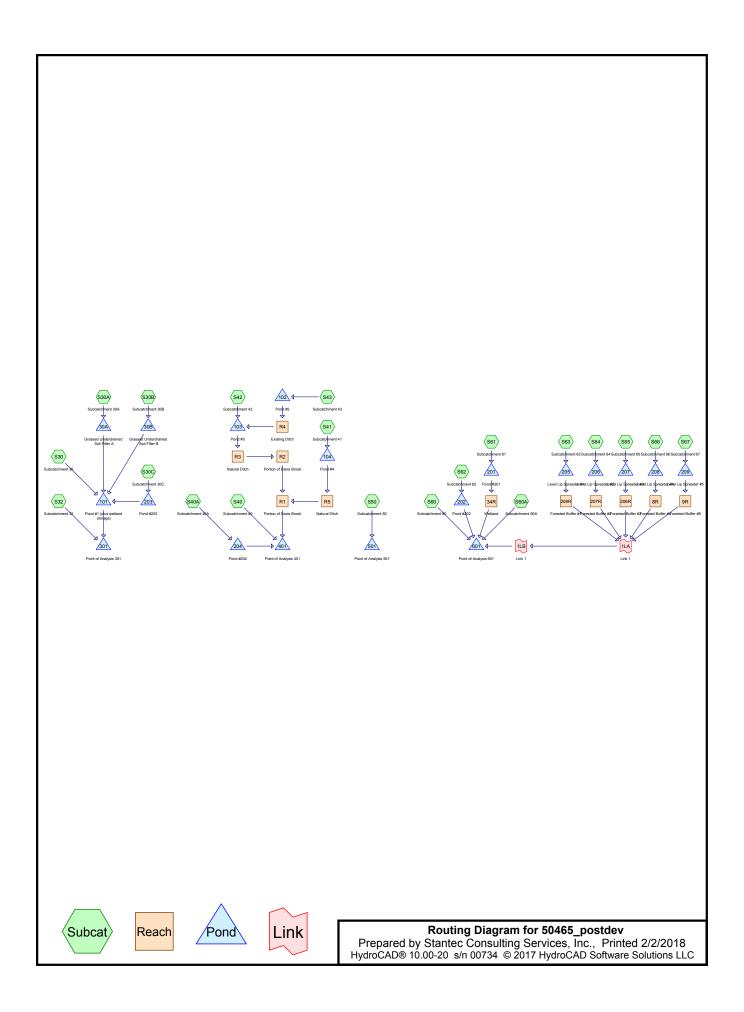
HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 46

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

ATTACHMENT B

POST DEVELOPMENT HYDROCAD COMPUTATIONS



Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
60.504	98	(\$30, \$30A, \$30B, \$30C, \$32, \$40, \$40A, \$41, \$42, \$43, \$50, \$60, \$61, \$62, \$63, \$64, \$65)
4.311	49	50-75% Grass cover, Fair, HSG A (\$30A, \$30B, \$42, \$43, \$61, \$62)
0.860	69	50-75% Grass cover, Fair, HSG B (\$42, \$43)
2.132	79	50-75% Grass cover, Fair, HSG C (S30A, S30B, S41, S42, S43)
1.338	84	50-75% Grass cover, Fair, HSG D (S41, S62)
0.033	86	<50% Grass cover, Poor, HSG C (S61)
7.484	39	>75% Grass cover, Good, HSG A (\$30, \$30C, \$32, \$40, \$40A, \$60, \$60A, \$63, \$64, \$65,
		S66, S67)
2.345	61	>75% Grass cover, Good, HSG B (S40, S50)
8.983	74	>75% Grass cover, Good, HSG C (\$30, \$30C, \$32, \$40, \$40A, \$50, \$60A)
7.572	80	>75% Grass cover, Good, HSG D (\$32, \$40, \$40A, \$60, \$66, \$67)
2.907	98	Water Surface (S30, S30C, S40A, S41, S42, S43, S61, S62)
1.974	36	Woods, Fair, HSG A (S43)
0.136	60	Woods, Fair, HSG B (S43)
40.765	30	Woods, Good, HSG A (\$30, \$32, \$40, \$60, \$60A)
0.728	55	Woods, Good, HSG B (S40)
10.227	70	Woods, Good, HSG C (S30, S32, S40, S60, S60A)
13.309	77	Woods, Good, HSG D (S32, S40, S60, S60A)
0.242	77	Woods, Poor, HSG C (S43)
165.848	70	TOTAL AREA

Printed 2/2/2018 Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
54.534	HSG A	\$30, \$30A, \$30B, \$30C, \$32, \$40, \$40A, \$42, \$43, \$60, \$60A, \$61, \$62, \$63, \$64, \$65, \$66, \$67
4.069	HSG B	S40, S42, S43, S50
21.617	HSG C	S30, S30A, S30B, S30C, S32, S40, S40A, S41, S42, S43, S50, S60, S60A, S61
22.219	HSG D	S32, S40, S40A, S41, S60, S60A, S62, S66, S67
63.410	Other	\$30, \$30A, \$30B, \$30C, \$32, \$40, \$40A, \$41, \$42, \$43, \$50, \$60, \$61, \$62, \$63, \$64, \$65
165.848		TOTAL AREA

Printed 2/2/2018 Page 4

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	0.000	60.504	60.504		\$30, \$30A, \$30B, \$30C, \$32, \$40,
							\$40A, \$41, \$42,
							\$43, \$50, \$60,
							\$61, \$62, \$63,
							S64, S65
4.311	0.860	2.132	1.338	0.000	8.641	50-75% Grass cover, Fair	\$30A, \$30B, \$41,
							\$42, \$43, \$61,
							\$62
0.000	0.000	0.033	0.000	0.000	0.033	<50% Grass cover, Poor	\$61
7.484	2.345	8.983	7.572	0.000	26.383	>75% Grass cover, Good	
							\$40, \$40A, \$50,
							\$60, \$60A, \$63,
							\$64, \$65, \$66,
0.000	0.000	0.000	0.000	2.907	2.907	Water Surface	\$67
0.000	0.000	0.000	0.000	2.907	2.907	water surface	\$30, \$30C,
							\$40A, \$41, \$42,
1.974	0.136	0.000	0.000	0.000	2.110	Woods, Fair	\$43, \$61, \$62 \$43
40.765	0.728	10.227	13.309	0.000	65.028	Woods, Good	\$30, \$32, \$40,
0.000	0.000	0.040	0.000	0.000	0.040	Manda Dani	\$60, \$60A
0.000	0.000	0.242	0.000	0.000	0.242	Woods, Poor	\$43
54.534	4.069	21.617	22.219	63.410	165.848	TOTAL AREA	

Summary for Subcatchment S30: Subcatchment 30

Runoff = 7.70 cfs @ 12.82 hrs, Volume= 1.447 af, Depth= 0.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Αı	rea (sf)	CN	De	scription		
*	3	79,829	98				
	1	51,673	30	Wo	ods, God	od, HSG A	
	1	29,474	70	Wo	oods, Goo	od, HSG C	
*		22,394	98	Wo	ater Surfac	ce	
		99,405	39			cover, Good	
_		99,405	74	>7	5% Grass o	cover, Good	d, HSG C
		82,180	73		eighted Av	_	
	4	79,957		54.	.41% Pervi	ous Area	
	4	02,223		45.	.59% Impe	rvious Area	
						_	
	Tc	Length	-			Capacity	Description
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)	
	8.0	75	0.040	00	1.66		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	4.9	481	0.010	04	1.64		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	49.2	1,094	0.003	55	0.37		Shallow Concentrated Flow, C to D
_							Woodland Kv= 5.0 fps
	54 9	1.650	Tota	1			

Summary for Subcatchment S30A: Subcatchment 30A

Runoff = 8.49 cfs @ 12.09 hrs, Volume= 0.642 af, Depth= 2.35"

	۸.	/ ()	C) I	Б-			
_	Ar	ea (sf)	CN	De	escription		
*	1	15,960	98				
		24,290	79	50-	-75% Gras	s cover, Fair	r, HSG C
		2,477	49	50-	-75% Gras	s cover, Fair	r, HSG A
	1	42,727	94	We	eighted A	verage	
		26,767		18.	.75% Pervi	ous Area	
	1	15,960		81.	.25% Impe	rvious Area	
	Tc	Length	Slop	ое	Velocity	Capacity	Description
	(min)	(feet)	(ft/	/ft)	(ft/sec)	(cfs)	
	1.1	75	0.01	50	1.12		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	2.8	333	0.01	50	1.97		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	2.0	400	Toto	ءا اد	acroacod	to minimum	$T_0 = /0$ min

Summary for Subcatchment S30B: Subcatchment 30B

Runoff = 12.82 cfs @ 12.09 hrs, Volume= 0.982 af, Depth= 2.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Ar	ea (sf)	CN	De	escription		
*	1	97,982	98				
		947	79	50-	-75% Gras	s cover, Fair	r, HSG C
		10,676	49	50-	-75% Gras	s cover, Fair	r, HSG A
	2	09,605	95	We	eighted A	verage	
		11,623		5.5	55% Pervio	us Area	
	1	97,982		94	.45% Impe	rvious Area	
	TC	Length	Slop	эе	Velocity	Capacity	Description
(min)	(feet)	(ft/	′f†)	(ft/sec)	(cfs)	
	1.0	75	0.02	00	1.26		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	1.9	312	0.03	00	2.79		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	2.9	387	Tota	al, Ir	ncreased	to minimum	TC = 6.0 min

Summary for Subcatchment S30C: Subcatchment 30C

Runoff = 22.69 cfs @ 12.13 hrs, Volume= 1.878 af, Depth= 2.25"

	Area (sf)	CN	Description
*	360,000	98	
	24,598	39	>75% Grass cover, Good, HSG A
	34,063	74	>75% Grass cover, Good, HSG C
*	16,903	98	Water Surface
	435,564	93	Weighted Average
	58,661		13.47% Pervious Area
	376,903		86.53% Impervious Area

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC Page 7

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.0	75	0.0200	1.26	•	Sheet Flow, A to B
					Smooth surfaces n= 0.011 P2= 3.00"
2.2	300	0.0200	2.28		Shallow Concentrated Flow, B to C
					Unpaved Kv= 16.1 fps
1.8	200	0.0150	1.84		Shallow Concentrated Flow, C to D
					Grassed Waterway Kv= 15.0 fps
0.6	30	0.0100	0.79		Sheet Flow, D to E
					Smooth surfaces n= 0.011 P2= 3.00"
3.5	46	0.0600	0.22		Sheet Flow, E to F
					Grass: Short n= 0.150 P2= 3.00"
9.1	651	Total			

Summary for Subcatchment S32: Subcatchment 32

Runoff = 0.47 cfs @ 12.64 hrs, Volume= 0.113 af, Depth= 0.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Area (s	sf)	CN	Description	า	
*	19,70)6	98			
	30,61	3	74	>75% Gras	s cover, Goo	d, HSG C
	59,97		39	>75% Gras	s cover, Goo	d, HSG A
	46,84		80	>75% Gras	s cover, Goo	d, HSG D
	46,89		30	Woods, Go	ood, HSG A	
	4,05			Woods, Go	ood, HSG C	
	8,27	7 4	77	<u>Woods, Go</u>	ood, HSG D	
	216,36	55	58	Weighted	Average	
	196,65	59		90.89% Per	vious Area	
	19,70)6		9.11% Imp	ervious Area	
			0.1			
	Tc Len	_	Slop		y Capacity	Description
(m	in) (te	et)	(ft/f	t) (ft/sec	(cfs)	
16	5.9	75	0.008	0.0	7	Sheet Flow, A to B
						Grass: Dense n= 0.240 P2= 3.00"
12	2.1	360	0.005	0 0.4	9	Shallow Concentrated Flow, B to C
						Short Grass Pasture Kv= 7.0 fps
29	2.0	435	Total			

Summary for Subcatchment \$40: Subcatchment 40

Runoff = 18.72 cfs @ 12.32 hrs, Volume= 2.141 af, Depth= 1.02"

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 8

	rea (sf)	CN I	Description		
*	158,513	98			
	31,707	55	Woods, God	od, HSG B	
:	217,840	70	Woods, God	od, HSG C	
	189,606	77 '	Woods, God	od, HSG D	
	950		Woods, Goo	od, HSG A	
	26,357		>75% Grass (
	97,705		>75% Grass (
	190,000		>75% Grass (
	190,000	80 :	>75% Grass (cover, Goo	d, HSG D
1,	102,678	76	Weighted A	verage	
•	944,165	8	85.62% Pervi	ous Area	
	158,513		14.38% Impe	ervious Area	
Tc	_	Slope	•	• •	Description
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)	
1.1	75	0.015	0 1.12		Sheet Flow, A to B
					Smooth surfaces n= 0.011 P2= 3.00"
4.4	550	0.016	4 2.06		Shallow Concentrated Flow, B to C
					Unpaved Kv= 16.1 fps
1.6	325	0.050	0 3.35		Shallow Concentrated Flow, C to D
					Grassed Waterway Kv= 15.0 fps
14.3	2,100	0.003	0 2.45	73.44	
					Area= 30.0 sf Perim= 35.0' r= 0.86'
					n= 0.030 Earth, grassed & winding
21.4	3,050	Total			

Summary for Subcatchment \$40A: Subcatchment 40A

Runoff = 16.55 cfs @ 12.18 hrs, Volume= 1.586 af, Depth= 2.45"

	Area (sf)	CN	Description
*	275,000	98	
	4,986	39	>75% Grass cover, Good, HSG A
	43,462	80	>75% Grass cover, Good, HSG D
	3,315	74	>75% Grass cover, Good, HSG C
*	11,757	98	Water Surface
	338,520	95	Weighted Average
	51,763		15.29% Pervious Area
	286,757		84.71% Impervious Area

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Pa	ge	9

T (mir	c Length	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.	1 75	0.0150	1.12	, ,	Sheet Flow, A to B
					Smooth surfaces n=0.011 P2=3.00"
1.	7 204	0.0150	1.97		Shallow Concentrated Flow, B to C
					Unpaved Kv= 16.1 fps
7.	0 629	0.0100	1.50		Shallow Concentrated Flow, C to D
					Grassed Waterway Kv= 15.0 fps
0.	2 148	0.0400	14.40	45.24	• • • • • • • • • • • • • • • • • • • •
					24.0" Round Area = 3.1 sf Perim = 6.3' r = 0.50'
	0 07	0.0010	0.00		n= 0.013 Corrugated PE, smooth interior
1.	9 37	0.0010	0.33		Sheet Flow, E to F
1	0 20	0.1700	0.20		Smooth surfaces n= 0.011 P2= 3.00"
1.	8 32	0.1600	0.30		Sheet Flow, F to G
					Grass: Short n= 0.150 P2= 3.00"
13.	7 1,125	Total			

Summary for Subcatchment S41: Subcatchment 41

Runoff = 14.77 cfs @ 12.10 hrs, Volume= 1.148 af, Depth= 2.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Α	rea (sf)	CN	De	escription							
*		187,081	98									
		22,827	79	50-	1-75% Grass cover, Fair, HSG C							
		37,988	84	50-	-75% Grass cover, Fair, HSG D							
*	:	7,519	519 98 Water Surface									
	2	255,415	94	We	eighted A	verage						
		60,815		23.	.81% Pervi	ous Area						
		194,600		76.	.19% Impe	rvious Area						
	Tc		-		•		Description					
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)						
	1.3	75	0.010	OC	0.95		Sheet Flow, A to B					
							Smooth surfaces n= 0.011 P2= 3.00"					
	3.3	383	0.014	43	1.93		Shallow Concentrated Flow, B to C					
							Unpaved Kv= 16.1 fps					
	2.5	243	0.053	35	1.62		Shallow Concentrated Flow, C to D					
_							Short Grass Pasture Kv= 7.0 fps					
	7.1	701	Tota									

Summary for Subcatchment \$42: Subcatchment 42

Runoff = 16.43 cfs @ 12.15 hrs, Volume= 1.398 af, Depth= 2.07"

Prepared by Stantec Consulting Services, Inc.
HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

<u>Page 10</u>

	Ar	ea (sf)	CN	De	escription				
*	2	78,726	98						
*		11,139	98	W	ater Surfac	ce			
		26,977	49	50-	-75% Gras	s cover, Fair	r, HSG A		
		29,325	69	50-	-75% Gras	s cover, Fair	r, HSG B		
_		6,630	79	50-	-75% Gras	s cover, Fair	r, HSG C		
	·		91	Weighted Average					
			17.84% Pervious Area						
	289,865			82.16% Impervious Area					
	Tc	Length	-		·	Capacity	Description		
_	(min)	(feet)	(ft/	'f†)	(ft/sec)	(cfs)			
	1.3	75	0.01	00	0.95		Sheet Flow, A to B		
							Smooth surfaces n= 0.011 P2= 3.00"		
	6.1	700	0.01	40	1.90		Shallow Concentrated Flow, B to C		
							Unpaved Kv= 16.1 fps		
	3.4	225	0.02	50	1.11		Shallow Concentrated Flow, C to D		
_							Short Grass Pasture Kv= 7.0 fps		
	10.8	1,000	Tota	ıl					

Summary for Subcatchment \$43: Subcatchment 43

Runoff = 7.28 cfs @ 12.41 hrs, Volume= 0.938 af, Depth= 0.91"

	Area (sf) CN Description							
*	2	32,052	98					
*		36,376	98	W	ater Surfac	ce		
	1	22,740	49	50	-75% Grass	s cover, Fair	r, HSG A	
		38,160	79	50	-75% Grass	s cover, Fair	r, HSG C	
		8,153	69	50	-75% Grass	s cover, Fair	r, HSG B	
		86,000	36	W	oods, Fair,	HSG A		
		5,923	60	W	oods, Fair,	HSG B		
_		10,549	77	Wo	oods, Poor	r, HSG C		
	539,953 74 Weighted Average					verage		
	2	71,525		50	.29% Pervi	ous Area		
	2	68,428		49	.71% Impe	rvious Area		
	Tc	Length	Slop		Velocity	Capacity	Description	
_	(min)	(feet)	(ft/	′ft)	(ft/sec)	(cfs)		
	10.6	75	0.01	00	0.12		Sheet Flow, A to B	
							Grass: Short n= 0.150 P2= 3.00"	
	16.3 488		0.01	0.0100 0.50			Shallow Concentrated Flow, B to C	
_							Woodland Kv= 5.0 fps	
	26.9	563	Toto	ıl				

<u>Page 11</u>

Summary for Subcatchment S50: Subcatchment 50

Runoff = 1.23 cfs @ 12.10 hrs, Volume= 0.091 af, Depth= 1.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Area (sf)	CN	Description
*	10,546	98	
	4,431	61	>75% Grass cover, Good, HSG B
	24,859	74	>75% Grass cover, Good, HSG C
	39,836	79	Weighted Average
	29,290		73.53% Pervious Area
	10,546		26.47% Impervious Area
(m	Tc Length nin) (feet)		pe Velocity Capacity Description /ft) (ft/sec) (cfs)
	6.0		Direct Entry,

Summary for Subcatchment S60: Subcatchment 60

Runoff = 0.12 cfs @ 16.21 hrs, Volume= 0.086 af, Depth= 0.03"

	Ar	ea (sf)	CN	De	scription		
	8	41,049	30	Wc	ods, God	od, HSG A	
	3.	44,078	77	Wc	ods, God	od, HSG D	
		22,871	70	Wc	ods, God	od, HSG C	
	;	37,335	39	>75	5% Grass o	cover, Good	d, HSG A
	;	32,830	80	>7	5% Grass o	cover, Good	d, HSG D
*	⁴ 17,570 98						
	15,415 77 Woods, Good, HSG D						
	1,3	11,148	46	We	eighted A	verage	
	1,2	93,578		98.	66% Pervi	ous Area	
		17,570		1.3	4% Imper	vious Area	
	Tc	Length	Slop				Description
(n	nin)	(feet)	(ft/:	ft)	(ft/sec)	(cfs)	
3	80.7	75	0.020	00	0.04		Sheet Flow, A to B
							Woods: Dense underbrush n= 0.800 P2= 3.00"
1	4.4	530	0.015	50	0.61		Shallow Concentrated Flow, B to C
							Woodland Kv= 5.0 fps
	1.4	430	0.010	00	5.10	122.30	Channel Flow, C to D
							Area= 24.0 sf Perim= 23.0' r= 1.04'
							n= 0.030 Earth, grassed & winding
4	6.5	1,035	Tota				

Summary for Subcatchment S60A: Subcatchment 60A

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Ar	ea (sf)	(sf) CN Description							
735,140 30 Woods, Good, HSG A										
		71,251 70 Woods, Good, HSG C								
22,354 77 Woods, Good, HSG D										
		26,632	39	>7	>75% Grass cover, Good, HSG A					
		9,029	74	>7	5% Grass of	cover, Good	d, HSG C			
	8	64,406	35	5 Weighted Average						
864,406				100.00% Pervious Area						
(n	Tc nin)	Length (feet)	Slo _l		Velocity (ft/sec)	Capacity (cfs)	Description			
3	30.7	75	0.02	200	0.04		Sheet Flow, A to B			
7	71.5	1,175	0.00	30	0.27		Woods: Dense underbrush n= 0.800 P2= 3.00" Shallow Concentrated Flow, B to C Woodland Kv= 5.0 fps			
10)2.2	1,250	Toto	 k						

Summary for Subcatchment S61: Subcatchment 61

Runoff = 8.25 cfs @ 12.09 hrs, Volume= 0.612 af, Depth= 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Ar	ea (sf)	CN	Descr	ription		
1,448 86 <50% Grass cover, Poor, HSG C							
		18,168	49	50-75	% Grass	cover, Fair	, HSG A
*		8,570	98	Wate	r Surfac	e	
*	1	19,736	98				
	1	47,922	92	Weigl	hted Av	/erage	
		19,616		13.269	% Pervi	ous Area	
	1	28,306		86.749	% Impe	rvious Area	
	Tc	Length	Slop	e Ve	elocity	Capacity	Description
_	(min)	(feet)	(ft/	ft) (f	ft/sec)	(cfs)	
	1.0	75	0.020	00	1.26		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	3.7	500	0.020	00	2.28		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	1.2	100	0.008	30	1.34		Shallow Concentrated Flow, C to D
							Grassed Waterway Kv= 15.0 fps
	5.9	675	Tota	Incr	eased :	to minimum	$T_C = 6.0 \text{ min}$

5.9 675 Total, Increased to minimum Tc = 6.0 min

Printed 2/2/2018 Page 13

Summary for Subcatchment S62: Subcatchment 62

Runoff = 17.26 cfs @ 12.09 hrs, Volume= 1.343 af, Depth= 2.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Ar	ea (sf)	sf) CN Description								
*		11,953	98	Wc	Vater Surface						
*	2	36,076	98								
		20,275	84	50-	-75% Grass cover, Fair, HSG D						
_		6,756	49 50-75% Grass cover, Fair, HSG A								
	2										
		27,031		9.8	3% Pervio	us Area					
	2	48,029		90.	17% Impe	rvious Area					
	Tc	Length	Slop		•	Capacity	Description				
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)					
	1.1	75	0.013	50	1.12		Sheet Flow, A to B				
							Smooth surfaces n= 0.011 P2= 3.00"				
	1.3	150	0.013	50	1.97		Shallow Concentrated Flow, B to C				
							Unpaved Kv= 16.1 fps				
	2.5	600	0.030	00	4.05	24.32	Channel Flow, C to D				
							Area= 6.0 sf Perim= 12.0' r= 0.50'				
_							n= 0.040 Earth, cobble bottom, clean sides				
	1 Q	825	Tota	l In	creased	to minimum	$T_{C} = A \cap min$				

⁸²⁵ Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment S63: Subcatchment 63

Runoff = 0.80 cfs @ 12.09 hrs, Volume= 0.058 af, Depth= 1.59"

_	Area (sf)	CN	Description							
*	15,000	98								
_	4,049	39	>75% Grass of	cover, Good	d, HSG A					
	19,049									
	4,049	21.26% Pervious Area								
	15,000									
	Tc Length		·	Capacity	Description					
_	(min) (feet)	(††,	/ft) (ft/sec)	(cfs)						
	6.0				Direct Entry,					

Summary for Subcatchment S64: Subcatchment 64

Runoff = 0.65 cfs @ 12.09 hrs, Volume= 0.047 af, Depth= 1.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

	Aı	rea (sf)	CN	Description							
-	*	11,520	98								
		2,572	39	>75% Grass	75% Grass cover, Good, HSG A						
		14,092	87	Weighted A	verage						
		2,572 18.25% Pervious Area									
		11,520		81.75% Impe	ervious Area						
	-		0.1								
	Tc	Length		be Velocity	. ,	Description					
	(min)	(feet)	(ft/	ft) (ft/sec)	(cfs)						
	6.0					Direct Entry					

Summary for Subcatchment S65: Subcatchment 65

Runoff = 0.56 cfs @ 12.11 hrs, Volume= 0.049 af, Depth= 0.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

_	Are	a (sf)	CN	Description	1		
*	20	0,240	98				
_	20	0,692	39	>75% Gras	cover, Goo	d, HSG A	
	40	0,932	68	Weighted	Average		
	20	0,692	2 50.55% Pervious Area				
	20	0,240	49.45% Impervious Area			I	
	Tc L (min)	ength. (feet)			y Capacity) (cfs)	Description	
	6.0					Direct Entry,	

Summary for Subcatchment S66: Subcatchment 66

Runoff = 0.08 cfs @ 12.16 hrs, Volume= 0.013 af, Depth= 0.33"

 Area (sf)	CN	Description
9,622	39	>75% Grass cover, Good, HSG A
 10,094	80	>75% Grass cover, Good, HSG D
19,716	60	Weighted Average
19,716		100.00% Pervious Area

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 15

(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Direct Entry,

Summary for Subcatchment S67: Subcatchment 67

0.03 cfs @ 12.35 hrs, Volume= 0.007 af, Depth= 0.22" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Yr Rainfall=3.00"

A	rea (sf)	sf) CN Description					
	9,767	7 39 >75% Grass cover, Good, HSG A					
	6,617	80	>75% Grass cover, Good, HSG D				
	16,384 56 Weighted Average						
	16,384	,384 100.00% Pervious Area					
_							
Tc	Length	Slop	oe Velocity	Capacity	Description		
(min)	(feet)	(ft/	ft) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Reach 8R: Forested Buffer #4

0.453 ac, 0.00% Impervious, Inflow Depth = 0.21" for 2-Yr event Inflow Area =

0.008 af 0.03 cfs @ 13.35 hrs, Volume= Inflow

Outflow 0.01 cfs @ 17.14 hrs, Volume= 0.008 af, Atten= 56%, Lag= 227.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.03 fps, Min. Travel Time= 93.4 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 318.4 min

Peak Storage= 73 cf @ 15.58 hrs

Average Depth at Peak Storage= 0.02'

Bank-Full Depth= 0.10' Flow Area= 2.5 sf, Capacity= 0.20 cfs

25.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 150.0' Slope= 0.0400 '/'

Inlet Invert= 249.00', Outlet Invert= 243.00'

Prepared by Stantec Consulting Services, Inc.
HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 16

Summary for Reach 9R: Forested Buffer #5

Inflow Area = 0.376 ac, 0.00% Impervious, Inflow Depth = 0.07" for 2-Yr event

Inflow = 0.01 cfs @ 17.00 hrs, Volume= 0.002 af

Outflow = 0.00 cfs @ 23.94 hrs, Volume= 0.002 af, Atten= 62%, Lag= 416.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.02 fps, Min. Travel Time= 149.5 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 408.1 min

Peak Storage= 32 cf @ 21.45 hrs Average Depth at Peak Storage= 0.01' Bank-Full Depth= 0.10' Flow Area= 2.5 sf, Capacity= 0.21 cfs

 $25.00' \times 0.10'$ deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 150.0' Slope= 0.0467 '/'

Inlet Invert= 249.00', Outlet Invert= 242.00'

Summary for Reach 34R: Wetland

Inflow Area = 3.396 ac, 86.74% Impervious, Inflow Depth > 2.09" for 2-Yr event

Inflow = 0.13 cfs @ 19.54 hrs, Volume= 0.591 af

Outflow = 0.13 cfs @ 39.59 hrs, Volume= 0.493 af, Atten= 3%, Lag= 1,202.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.03 fps, Min. Travel Time= 425.9 min Avg. Velocity = 0.03 fps, Avg. Travel Time= 500.7 min

Peak Storage= 3,294 cf @ 32.49 hrs Average Depth at Peak Storage= 0.04'

Bank-Full Depth= 0.10' Flow Area= 10.0 sf, Capacity= 0.57 cfs

100.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush

Length= 800.0' Slope= 0.0050 '/'

Inlet Invert= 245.00', Outlet Invert= 241.00'

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Summary for Reach 206R: Forested Buffer #3

Inflow Area = 0.940 ac, 49.45% Impervious, Inflow Depth = 0.63" for 2-Yr event

Inflow = 0.56 cfs @ 12.11 hrs, Volume= 0.049 af

Outflow = 0.22 cfs @ 13.09 hrs, Volume= 0.049 af, Atten= 62%, Lag= 59.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.04 fps, Min. Travel Time= 37.0 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 233.5 min

Peak Storage= 478 cf @ 12.48 hrs Average Depth at Peak Storage= 0.09'

Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.28 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 86.0' Slope= 0.0116 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Summary for Reach 207R: Forested Buffer #2

Inflow Area = 0.324 ac, 81.75% Impervious, Inflow Depth = 1.74" for 2-Yr event

Inflow = 0.65 cfs @ 12.09 hrs, Volume= 0.047 af

Outflow = 0.23 cfs @ 13.20 hrs, Volume= 0.047 af, Atten= 64%, Lag= 66.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.04 fps, Min. Travel Time= 48.8 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 327.3 min

Peak Storage= 681 cf @ 12.38 hrs Average Depth at Peak Storage= 0.10'

Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.25 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 109.0' Slope= 0.0092 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Prepared by Stantec Consulting Services, Inc.
HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

<u> Page 18</u>

Summary for Reach 208R: Forested Buffer #1

Inflow Area = 0.437 ac, 78.74% Impervious, Inflow Depth = 1.59" for 2-Yr event

Inflow = 0.80 cfs @ 12.09 hrs, Volume= 0.058 af

Outflow = 0.24 cfs @ 13.49 hrs, Volume= 0.058 af, Atten= 70%, Lag= 84.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.04 fps, Min. Travel Time= 62.8 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 386.0 min

Peak Storage= 913 cf @ 12.45 hrs Average Depth at Peak Storage= 0.10' Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.22 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 134.0' Slope= 0.0075 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Summary for Reach R1: Portion of Davis Brook

Inflow Area = 26.358 ac, 65.57% Impervious, Inflow Depth > 0.22" for 2-Yr event

Inflow = 1.01 cfs @ 13.77 hrs, Volume= 0.478 af

Outflow = 0.71 cfs @ 15.61 hrs, Volume= 0.470 af, Atten= 29%, Lag= 110.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.62 fps, Min. Travel Time= 42.7 min Avg. Velocity = 0.26 fps, Avg. Travel Time= 102.1 min

Peak Storage= 1,827 cf @ 14.90 hrs Average Depth at Peak Storage= 0.19'

Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 176.49 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 1,600.0' Slope= 0.0031 '/'

Inlet Invert= 221.00', Outlet Invert= 216.00'



Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 19

Summary for Reach R2: Portion of Davis Brook

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.05" for 2-Yr event

Inflow = 0.02 cfs @ 24.55 hrs, Volume= 0.086 af

Outflow = 0.02 cfs @ 27.25 hrs, Volume= 0.084 af, Atten= 0%, Lag= 162.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.20 fps, Min. Travel Time= 49.3 min Avg. Velocity = 0.20 fps, Avg. Travel Time= 49.3 min

Peak Storage= 53 cf @ 26.43 hrs Average Depth at Peak Storage= 0.02' Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 182.28 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 600.0' Slope= 0.0033 '/'

Inlet Invert= 223.00', Outlet Invert= 221.00'



Summary for Reach R3: Natural Ditch

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.05" for 2-Yr event

Inflow = 0.02 cfs @ 24.29 hrs, Volume= 0.086 af

Outflow = 0.02 cfs @ 24.55 hrs, Volume= 0.086 af, Atten= 0%, Lag= 15.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.84 fps, Min. Travel Time= 5.9 min

Avg. Velocity = 0.84 fps, Avg. Travel Time= 5.9 min

Peak Storage= 6 cf @ 24.45 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 998.06 cfs

10.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 5.0 '/' Top Width= 40.00'

Length= 300.0' Slope= 0.0433 '/'

Inlet Invert= 236.00', Outlet Invert= 223.00'



Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 20

Summary for Reach R4: Existing Ditch

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 0.00" for 2-Yr event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 168.09 cfs

8.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 20.00'

Length= 190.0' Slope= 0.0053 '/'

Inlet Invert= 244.00', Outlet Invert= 243.00'



Summary for Reach R5: Natural Ditch

Inflow Area = 5.864 ac, 76.19% Impervious, Inflow Depth > 0.81" for 2-Yr event

Inflow = 1.37 cfs @ 13.50 hrs, Volume= 0.395 af

Outflow = 1.00 cfs @ 13.77 hrs, Volume= 0.394 af, Atten= 27%, Lag= 16.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.96 fps, Min. Travel Time= 4.4 min

Avg. Velocity = 0.73 fps, Avg. Travel Time= 5.7 min

Peak Storage= 262 cf @ 13.69 hrs

Average Depth at Peak Storage= 0.04'

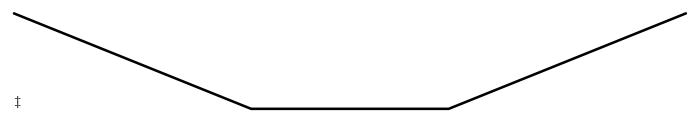
Bank-Full Depth= 3.00' Flow Area= 165.0 sf, Capacity= 1,945.52 cfs

25.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 10.0 '/' Top Width= 85.00'

Length= 250.0' Slope= 0.0320 '/'

Inlet Invert= 229.00', Outlet Invert= 221.00'



Volume

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 21

Summary for Pond 30A: Grassed Underdrained Soil Filter A

Inflow Area = 3.277 ac, 81.25% Impervious, Inflow Depth = 2.35" for 2-Yr event

Inflow = 8.49 cfs @ 12.09 hrs, Volume= 0.642 af

Primary = 8.49 cfs @ 12.09 hrs, Volume= 0.642 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 30B: Grassed Underdrained Soil Filter B

Inflow Area = 4.812 ac, 94.45% Impervious, Inflow Depth = 2.45" for 2-Yr event

Inflow = 12.82 cfs @ 12.09 hrs, Volume= 0.982 af

Primary = 12.82 cfs @ 12.09 hrs, Volume= 0.982 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 101: Pond #1 (plus wetland storage)

Inflow Area = 38.340 ac, 65.45% Impervious, Inflow Depth > 1.35" for 2-Yr event

Inflow = 22.22 cfs @ 12.09 hrs, Volume= 4.312 af

Outflow = 0.89 cfs @ 23.99 hrs, Volume= 2.194 af, Atten= 96%, Lag= 714.2 min

Primary = 0.89 cfs @ 23.99 hrs, Volume= 2.194 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 241.38' @ 23.99 hrs Surf.Area= 156,316 sf Storage= 121,487 cf

Plug-Flow detention time= 1,437.5 min calculated for 2.194 af (51% of inflow)

Avail Storage Storage Description

Center-of-Mass det. time=925.3 min (2,178.8 - 1,253.5)

Invert

#1	240.00'	835,458 cf	Custom S	tage Data	(Prismatic) Listed below (Recalc)	
Elevation	Surf.Ar	ea Inc	Store	Cum.Store		
(feet)	(sq	-ft) (cubic	-feet) (c	cubic-feet)		
240.00	22,6	686	0	0		
241.00	115,9	923 6	9,305	69,305		
242.00	221,2	289 16	8,606	237,911		
243.00	313,0)44 26	7,167	505,077		
244.00	347,7	717 33	0,381	835,458		

Device	Routing	Invert	Outlet Devices
#1	Primary	237.64'	24.0" Round Culvert
			L= 87.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 237.64' / 237.30' S= 0.0039 '/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=3.14 sf
#2	Device 1	241 00'	24.0" Vert. Orifice/Grate. C= 0.600

Primary OutFlow Max=0.89 cfs @ 23.99 hrs HW=241.38' (Free Discharge)

1=Culvert (Passes 0.89 cfs of 14.97 cfs potential flow)

T_2=Orifice/Grate (Orifice Controls 0.89 cfs @ 2.11 fps)

Printed 2/2/2018

Page 22

Summary for Pond 102: Pond #2

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 0.91" for 2-Yr event

Inflow = 7.28 cfs @ 12.41 hrs, Volume= 0.938 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 243.45' @ 25.60 hrs Surf.Area= 31,188 sf Storage= 40,866 cf

Plua-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	242.00'	190,120 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.00	25,000	0	0
244.68	36,400	82,276	82,276
246.00	127,000	107,844	190,120

Device	Routing	Invert	Outlet Devices
#1	Primary	246.53'	18.0" Round Culvert
	•		L= 165.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 246.53' / 244.00' S= 0.0153 '/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=1.77 sf
#2	Device 1	248.05'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=242.00' (Free Discharge)

1=Culvert (Controls 0.00 cfs)

2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 103: Pond #3

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth = 0.82" for 2-Yr event

Inflow = 16.43 cfs @ 12.15 hrs, Volume= 1.398 af

Outflow = 0.02 cfs @ 24.29 hrs, Volume= 0.086 af, Atten= 100%, Lag= 728.1 min

Primary = 0.02 cfs @ 24.29 hrs, Volume= 0.086 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 240.55' @ 24.29 hrs Surf.Area= 18,229 sf Storage= 60,150 cf

Plug-Flow detention time= 1,976.8 min calculated for 0.086 af (6% of inflow)

Center-of-Mass det. time=1,727.2 min (2,536.5 - 809.4)

Volume	Invert	Avail.Storage	Storage Description
#1	234.00'	94,130 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 23

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
234.00	4,000	0	0
236.00	7,000	11,000	11,000
238.92	11,139	26,483	37,483
240.00	14,287	13,730	51,213
242.00	28,630	42,917	94,130

Device	Routing	Invert	Outlet Devices
#1	Primary	234.22'	24.0" Round Culvert
	•		L= 95.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 234.22' / 233.90' S= 0.0034 '/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=3.14sf
#2	Device 1	241.29'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	236.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600

Primary Outflow Max=0.02 cfs @ 24.29 hrs HW=240.55' (Free Discharge)

1=Culvert (Passes 0.02 cfs of 21.70 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

3=Orifice/Grate (Orifice Controls 0.02 cfs @ 10.25 fps)

Summary for Pond 104: Pond #4

Inflow Area = 5.864 ac, 76.19% Impervious, Inflow Depth = 2.35" for 2-Yr event

Inflow = 14.77 cfs @ 12.10 hrs, Volume= 1.148 af

Outflow = 1.37 cfs @ 13.50 hrs, Volume= 0.395 af, Atten= 91%, Lag= 83.9 min

Primary = 1.37 cfs @ 13.50 hrs, Volume= 0.395 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 237.77' @ 13.50 hrs Surf.Area= 16,786 sf Storage= 36,018 cf

Plug-Flow detention time=710.0 min calculated for 0.395 af (34% of inflow)

Avail Storage Storage Description

Center-of-Mass det. time=573.1 min (1,363.3 - 790.3)

Invert

Volume

VOIDITIE	1111	1 Avaii.3101	age siolag	ge Description	
#1	230.00)' 36,01	8 cf Custor	m Stage Data (P	rismatic) Listed below (Recalc)
Elevation	n Su	ırf.Area	Inc.Store	Cum.Store	
(feet	·)	(sq-ft) (c	ubic-feet)	(cubic-feet)	
230.00	0	620	0	0	
232.0	0	3,391	4,011	4,011	
234.0	5	7,519	11,183	15,194	
235.0	0	9,087	7,888	23,082	
236.0	0	16,786	12,937	36,018	
Device	Routing	Invert	Outlet Dev	vices	
#1	Primary	230.62'	24.0" Rour	nd Culvert	
	•		L= 96.0' C	MP, projecting,	no headwall, Ke= 0.900
			Inlet / Outl	et Invert= 230.6	2' / 228.40' S= 0.0231 '/' Cc= 0.900
			n= 0.025 C	Corrugated met	al, Flow Area= 3.14 sf
#2	Device 1	237.61'	24.0" Horiz.	. Orifice/Grate	C= 0.600 Limited to weir flow at low heads

#3 Device 1 231.00' **0.5" W x 0.5" H Vert. Orifice/Grate** C= 0.600

Primary OutFlow Max=1.36 cfs @ 13.50 hrs HW=237.77' (Free Discharge)

1=Culvert (Passes 1.36 cfs of 27.22 cfs potential flow)

2=Orifice/Grate (Weir Controls 1.33 cfs @ 1.31 fps)

-3=Orifice/Grate (Orifice Controls 0.02 cfs @ 12.51 fps)

Summary for Pond 201: Pond #201

Inflow Area = 3.396 ac, 86.74% Impervious, Inflow Depth = 2.16" for 2-Yr event

Inflow = 8.25 cfs @ 12.09 hrs, Volume= 0.612 af

Outflow = 0.13 cfs @ 19.54 hrs, Volume= 0.591 af, Atten= 98%, Lag= 447.3 min

Primary = 0.13 cfs @ 19.54 hrs, Volume= 0.591 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 249.51' @ 19.54 hrs Surf.Area= 16,785 sf Storage= 20,503 cf

Plug-Flow detention time= 1,513.3 min calculated for 0.591 af (97% of inflow)

Center-of-Mass det. time=1,494.1 min (2,294.2 - 800.1)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1	248.00'	47,142 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
248.00	8,570	0	0
249.00	15,880	12,225	12,225
250.00	17,665	16,773	28,998
251.00	18,624	18,145	47,142

Device	Routing	Invert	Outlet Devices	
#1	Primary	250.00'	20.0' long x 10.0' breadth Broad-Crested Rectangular Weir	
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60	
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64	
#2	Primary	245.40'	12.0" Round Culvert L= 29.0' CPP, projecting, no headwall, Ke= 0.900	
			Inlet / Outlet Invert= 245.40' / 245.00' S= 0.0138'/' Cc= 0.900	
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	
#3	Device 2	249.65'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads	
#4	Device 2	245.50'	1.6" Vert. Orifice/Grate C= 0.600	
#5	Device 4	248.00'	2.000 in/hr Exfiltration over Surface area above 248.00'	
			Excluded Surface area = 8,570 sf	

Primary OutFlow Max=0.13 cfs @ 19.54 hrs HW=249.51' (Free Discharge)

—1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Passes 0.13 cfs of 5.67 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.13 cfs @ 9.56 fps)

5=Exfiltration (Passes 0.13 cfs of 0.38 cfs potential flow)

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 25

Summary for Pond 202: Pond #202

Inflow Area = 6.315 ac, 90.17% Impervious, Inflow Depth = 2.55" for 2-Yr event

Inflow = 17.26 cfs @ 12.09 hrs, Volume= 1.343 af

Outflow = 0.33 cfs @ 17.76 hrs, Volume= 1.280 af, Atten= 98%, Lag= 340.2 min

Primary = 0.33 cfs @ 17.76 hrs, Volume= 1.280 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 243.41'@ 17.76 hrs Surf.Area= 23,594 sf Storage= 44,520 cf

Plug-Flow detention time= 1,373.1 min calculated for 1.280 af (95% of inflow)

Center-of-Mass det. time=1,346.2 min (2,122.1 - 775.9)

Volume	Invert	Avail.Storage	Storage Description
#1	241.00'	92,645 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
241.00	12,606	0	0
242.00	18,880	15,743	15,743
243.00	20,722	19,801	35,544
244.00	27,811	24,267	59,811
245.00	37,857	32,834	92,645

Device	Routing	Invert	Outlet Devices	
#1	Primary	244.00'	25.0' long x 10.0' breadth Broad-Crested Rectangular Weir	
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60	
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64	
#2	Primary	238.40'	12.0" Round Culvert	
			L= 115.0' CPP, projecting, no headwall, Ke= 0.900	
			Inlet / Outlet Invert= 238.40' / 237.75' S= 0.0057 '/' Cc= 0.900	
			n=0.013 Corrugated PE, smooth interior, Flow Area=0.79 sf	
#3	Device 2	243.50'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads	
#4	Device 2	238.50'	2.4" Vert. Orifice/Grate C= 0.600	
#5	Device 4	241.00'	2.000 in/hr Exfiltration over Surface area above 241.00'	
			Excluded Surface area = 12,606 sf	

Primary OutFlow Max=0.33 cfs @ 17.76 hrs HW=243.41' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Passes 0.33 cfs of 5.79 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

4=Orifice/Grate (Orifice Controls 0.33 cfs @ 10.55 fps)

5=Exfiltration (Passes 0.33 cfs of 0.51 cfs potential flow)

Summary for Pond 203: Pond #203

Inflow Area = 9.999 ac, 86.53% Impervious, Inflow Depth = 2.25" for 2-Yr event

Inflow = 22.69 cfs @ 12.13 hrs, Volume= 1.878 af

Outflow = 0.34 cfs @ 21.29 hrs, Volume= 1.241 af, Atten= 99%, Lag= 549.6 min

Primary = 0.34 cfs @ 21.29 hrs, Volume= 1.241 af

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 26

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 248.34' @ 21.29 hrs Surf.Area= 32,639 sf Storage= 67,832 cf

Plug-Flow detention time= 1,558.1 min calculated for 1.241 af (66% of inflow) Center-of-Mass det. time= 1,462.4 min (2,260.2 - 797.8)

Volume	Inver	t Avail.Stor	age Storag	ge Description
#1	246.00)' 177,12	3 cf Custor	om Stage Data (Prismatic) Listed below (Recalc)
Elevation	n Su	rf.Area	Inc.Store	Cum.Store
(feet	-)	(sq-ft) (ci	ubic-feet)	(cubic-feet)
246.0	0	25,338	0	0
247.0	0	28,403	26,871	26,871
248.0	0	31,540	29,972	56,842
249.0	0	34,748	33,144	89,986
250.0		38,027	36,388	126,374
251.0	0	63,471	50,749	177,123
Б.	D !:		0 11 1 5	
Device	Routing	Invert	Outlet Dev	
#1	Primary	249.75'	_	x 8.0' breadth Broad-Crested Rectangular Weir
			•	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
				4.00 4.50 5.00 5.50
				glish) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64
				2.66 2.66 2.68 2.70 2.74
#2	Primary	243.40'		nd Culvert L= 65.0' CPP, projecting, no headwall, Ke= 0.900
				tlet Invert= 243.40' / 243.00' S= 0.0062 '/' Cc= 0.900
				Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	249.25'		z. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2			Orifice/Grate C= 0.600
#5	Device 4	246.00'	•	r Exfiltration over Surface area above 246.00'
			Excluded S	Surface area = 25,338 sf

Primary OutFlow Max=0.34 cfs @ 21.29 hrs HW=248.34' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Passes 0.34 cfs of 6.29 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Passes 0.34 cfs of 0.42 cfs potential flow)

5=Exfiltration (Exfiltration Controls 0.34 cfs)

Summary for Pond 204: Pond #204

Inflow Area = 7.771 ac, 84.71% Impervious, Inflow Depth = 2.45" for 2-Yr event

Inflow = 16.55 cfs @ 12.18 hrs, Volume= 1.586 af

Outflow = 0.35 cfs @ 18.80 hrs, Volume= 1.488 af, Atten= 98%, Lag= 396.7 min

Primary = 0.35 cfs @ 18.80 hrs, Volume= 1.488 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 237.83' @ 18.80 hrs Surf.Area= 22,686 sf Storage= 53,466 cf

Plug-Flow detention time= 1,515.2 min calculated for 1.488 af (94% of inflow) Center-of-Mass det. time= 1,481.6 min (2,271.7 - 790.1)

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 27

<u>Volume</u>	Inver	t Avail.Stord	age Storaç	ge Description
#1	235.00	106,90	3 cf Custo	om Stage Data (Prismatic) Listed below (Recalc)
Elevatio	n Su	rf.Area	Inc.Store	Cum.Store
(feet	t)	(sq-ft) (cu	ubic-feet)	(cubic-feet)
235.0	0	11,757	0	0
236.0	0	18,909	15,333	15,333
237.0	0	20,939	19,924	35,257
238.0	0	23,032	21,986	57,243
239.0	0	25,210	24,121	81,364
240.0	0	25,868	25,539	106,903
5 .	D !!		0 11 15	
<u>Device</u>	Routing	Invert	Outlet Dev	
#1	Primary	239.00'	_	x 12.0' breadth Broad-Crested Rectangular Weir
			•	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			, ,	glish) 2.57 2.62 2.70 2.67 2.66 2.67 2.66 2.64
#2	Primary	232.40'		nd Culvert L= 93.0' CPP, projecting, no headwall, Ke= 0.900
			-	tlet Invert= 232.40' / 232.00' S= 0.0043'/' Cc= 0.900
	D : 0	000 501		Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	232.50'		Orifice/Grate C= 0.600
#4	Device 2			z. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 3	235.00'	2.000 in/hr	r Exfiltration over Surface area above 235.00'
			Excluded S	Surface area = 11,757 sf

Primary OutFlow Max=0.35 cfs @ 18.80 hrs HW=237.83' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Passes 0.35 cfs of 6.31 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.35 cfs @ 11.02 fps)

5=Exfiltration (Passes 0.35 cfs of 0.51 cfs potential flow)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 205: Level Lip Spreader #1

Inflow Area = 0.437 ac, 78.74% Impervious, Inflow Depth = 1.59" for 2-Yr event

Inflow = 0.80 cfs @ 12.09 hrs, Volume= 0.058 af

Outflow = 0.80 cfs @ 12.09 hrs, Volume= 0.058 af, Atten= 0%, Lag= 0.1 min

Primary = 0.80 cfs @ 12.09 hrs, Volume= 0.058 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 242.53' @ 12.09 hrs Surf.Area= 148 sf Storage= 4 cf

Plug-Flow detention time=0.1 min calculated for 0.058 af (100% of inflow)

Center-of-Mass det. time=0.1 min (829.1 - 829.0)

Volume	Invert	Avail.Storage	Storage Description
#1	242.50'	306 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 28

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.50	139	0	0
243.00	292	108	108
243.50	500	198	306

Device Routing Invert Outlet Devices

#1 Primary 242.50' 65.0' long x 2.0' breadth Broad-Crested Rectangular Weir

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.78 cfs @ 12.09 hrs HW=242.53' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 0.78 cfs @ 0.43 fps)

Summary for Pond 206: Level Lip Spreader #2

Inflow Area = 0.324 ac, 81.75% Impervious, Inflow Depth = 1.74" for 2-Yr event

Inflow = 0.65 cfs @ 12.09 hrs, Volume= 0.047 af

Outflow = 0.65 cfs @ 12.09 hrs, Volume= 0.047 af, Atten= 0%, Lag= 0.1 min

Primary = 0.65 cfs @ 12.09 hrs, Volume= 0.047 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.52' @ 12.09 hrs Surf.Area= 147 sf Storage= 4 cf

Plug-Flow detention time=0.1 min calculated for 0.047 af (100% of inflow)

Center-of-Mass det. time=0.1 min (821.8 - 821.6)

<u>Volume</u>	Invert	Avail.Storag	<u>e Storaç</u>	ge Description	
#1	242.50'	306 (cf Custo	m Stage Data (P	'rismatic) Listed below (Recalc)
Elevatior (feet			ic.Store ic-feet)	Cum.Store (cubic-feet)	
242.50 243.00 243.50		139 292 500	0 108 198	0 108 306	
Device	Routing	Invert (Dutlet De	vices	
#1	#1 Primary 242.50' 65.0' lor Head (f 3.00 3.5 Coef. (E		Head (fee 3.00 3.50	t) 0.20 0.40 0.6 glish) 2.54 2.61	road-Crested Rectangular Weir 0 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

Primary OutFlow Max=0.63 cfs @ 12.09 hrs HW=242.52' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 0.63 cfs @ 0.40 fps)

Summary for Pond 207: Level Lip Spreader #3

Inflow Area = 0.940 ac, 49.45% Impervious, Inflow Depth = 0.63" for 2-Yr event

Inflow = 0.56 cfs @ 12.11 hrs, Volume= 0.049 af

Outflow = 0.56 cfs @ 12.11 hrs, Volume= 0.049 af, Atten= 0%, Lag= 0.1 min

Primary = 0.56 cfs @ 12.11 hrs, Volume= 0.049 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.52' @ 12.11 hrs Surf.Area= 146 sf Storage= 3 cf

Plug-Flow detention time=0.1 min calculated for 0.049 af (100% of inflow)

Center-of-Mass det. time=0.1 min (888.9 - 888.7)

Volume	Invert	Avail.Storage	Storage Description
#1	242.50'	306 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surt.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.50	139	0	0
243.00	292	108	108
243.50	500	198	306

Device Routing Invert Outlet Devices

#1 Primary 242.50' 65.0' long x 2.0' breadth Broad-Crested Rectangular Weir

 $\mbox{Head (feet)} \ \ 0.20 \ \ 0.40 \ \ 0.60 \ \ 0.80 \ \ 1.00 \ \ 1.20 \ \ 1.40 \ \ 1.60 \ \ 1.80 \ \ 2.00 \ \ 2.50$

3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

3.07 3.20 3.32

Primary OutFlow Max=0.54 cfs @ 12.11 hrs HW=242.52' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 0.54 cfs @ 0.38 fps)

Summary for Pond 208: Level Lip Spreader #4

Inflow Area = 0.453 ac, 0.00% Impervious, Inflow Depth = 0.33" for 2-Yr event

Inflow = 0.08 cfs @ 12.16 hrs, Volume= 0.013 af

Outflow = 0.03 cfs @ 13.35 hrs, Volume= 0.008 af, Atten= 65%, Lag= 71.4 min

Primary = 0.03 cfs @ 13.35 hrs, Volume= 0.008 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.00' @ 13.35 hrs Surf.Area= 300 sf Storage= 200 cf

Plug-Flow detention time= 233.5 min calculated for 0.008 af (64% of inflow)

Center-of-Mass det. time= 103.8 min (1,033.7 - 929.9)

Volume	Invert	Avail.Storage	Storage Description	
#1	248.00'	200 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 30

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
248.00	100	0	0
248.50	200	75	75
249.00	300	125	200

Device Routing Invert Outlet Devices

#1 Primary 249.00' 25.0' long x 2.0' breadth Broad-Crested Rectangular Weir

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50

3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

3.07 3.20 3.32

Primary OutFlow Max=0.02 cfs @ 13.35 hrs HW=249.00' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 0.02 cfs @ 0.17 fps)

Summary for Pond 209: Level Lip Spreader #5

Inflow Area = 0.376 ac, 0.00% Impervious, Inflow Depth = 0.22" for 2-Yr event

Inflow = 0.03 cfs @ 12.35 hrs, Volume= 0.007 af

Outflow = 0.01 cfs @ 17.00 hrs, Volume= 0.002 af, Atten= 71%, Lag= 279.1 min

Primary = 0.01 cfs @ 17.00 hrs, Volume= 0.002 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.00' @ 17.00 hrs Surf.Area= 300 sf Storage= 200 cf

Plug-Flow detention time= 443.3 min calculated for 0.002 af (33% of inflow)

Center-of-Mass det. time=248.3 min (1,208.9 - 960.6)

Volume	Invert	Avail	.Storage	Storag	ge Description	
#1	248.00'		200 cf	Custo	m Stage Data (F	Prismatic) Listed below (Recalc)
Elevation (feet)		Area sq-ft)	lnc.: (cubic-	Store feet)	Cum.Store (cubic-feet)	
248.00		100		0	0	
248.50		200		75	75	
249.00		300		125	200	

Primary OutFlow Max=0.00 cfs @ 17.00 hrs HW=249.00' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.10 fps)

Prepared by Stantec Consulting Services, Inc.
HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

<u>Page 31</u>

Summary for Pond 301: Point of Analysis 301

Inflow Area = 43.307 ac, 58.99% Impervious, Inflow Depth > 0.64" for 2-Yr event

Inflow = 0.93 cfs @ 23.44 hrs, Volume= 2.308 af

Primary = 0.93 cfs @ 23.44 hrs, Volume= 2.308 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 401: Point of Analysis 401

Inflow Area = 59.444 ac, 46.27% Impervious, Inflow Depth > 0.83" for 2-Yr event

Inflow = 19.03 cfs @ 12.32 hrs, Volume= 4.100 af

Primary = 19.03 cfs @ 12.32 hrs, Volume= 4.100 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 501: Point of Analysis 501

Inflow Area = 0.915 ac, 26.47% Impervious, Inflow Depth = 1.19" for 2-Yr event

Inflow = 1.23 cfs @ 12.10 hrs, Volume= 0.091 af

Primary = 1.23 cfs @ 12.10 hrs, Volume= 0.091 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 601: Point of Analysis 601

Inflow Area = 62.183 ac, 16.27% Impervious, Inflow Depth > 0.39" for 2-Yr event

Inflow = 0.95 cfs @ 13.30 hrs, Volume= 2.023 af

Primary = 0.95 cfs @ 13.30 hrs, Volume= 2.023 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link 1LA: Link 1

Inflow Area = 2.529 ac, 42.44% Impervious, Inflow Depth > 0.78" for 2-Yr event

Inflow = 0.63 cfs @ 13.30 hrs, Volume= 0.164 af

Primary = 0.63 cfs @ 13.30 hrs, Volume= 0.164 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link 1LB: Link 1

Inflow Area = 2.529 ac, 42.44% Impervious, Inflow Depth > 0.78" for 2-Yr event

Inflow = 0.63 cfs @ 13.30 hrs, Volume= 0.164 af

Primary = 0.63 cfs @ 13.30 hrs, Volume= 0.164 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Subcatchment S30: Subcatchment 30

Runoff = 16.79 cfs @ 12.77 hrs, Volume= 2.947 af, Depth= 1.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Αı	rea (sf)	CN	De	scription		
*	3	79,829	98				
	1	51,673	30	Wo	ods, God	od, HSG A	
	1	29,474	70	Wo	oods, Goo	od, HSG C	
*		22,394	98	Wo	ater Surfac	ce	
		99,405	39			cover, Good	
_		99,405	74	>7	5% Grass o	cover, Good	d, HSG C
		82,180	73		eighted Av	_	
	4	79,957		54.	.41% Pervi	ous Area	
	4	02,223		45.	.59% Impe	rvious Area	
						_	
	Tc	Length	-			Capacity	Description
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)	
	8.0	75	0.040	00	1.66		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	4.9	481	0.010	04	1.64		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	49.2	1,094	0.003	55	0.37		Shallow Concentrated Flow, C to D
_							Woodland Kv= 5.0 fps
	54 9	1.650	Tota	1			

Summary for Subcatchment S30A: Subcatchment 30A

Runoff = 12.75 cfs @ 12.09 hrs, Volume= 0.988 af, Depth= 3.62"

_	Ar	rea (sf)	CN	Descri	iption				
*	1	15,960	98						
		24,290	79	50-759	% Grass	s cover, Fair	, HSG C		
_		2,477	49	50-759	% Grass	s cover, Fair	, HSG A		
	1	42,727	94	Weigh	nted Av	verage			
26,767 18.75% Pe						ous Area			
	1	15,960		81.25% Impervious Area					
	Tc (min)	Length (feet)	Slop (ft/		elocity t/sec)	Capacity (cfs)	Description		
	1.1	75	0.01	50	1.12		Sheet Flow, A to B		
	2.8	333	0.01	50	1.97		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C Unpaved Kv= 16.1 fps		
	3 9	408	Tota	Incre	ansed t	to minimum	$T_{C} = 6.0 \text{ min}$		

Summary for Subcatchment S30B: Subcatchment 30B

Runoff = 19.04 cfs @ 12.09 hrs, Volume= 1.495 af, Depth= 3.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Ar	ea (sf)	CN	De	escription		
*	1	97,982	98				
		947	79	50-	-75% Gras	s cover, Fair	, HSG C
		10,676	49	50-	-75% Gras	s cover, Fair	r, HSG A
	2	09,605	95	We	eighted A	verage	
		11,623		5.5	55% Pervio	us Area	
	1	97,982		94	.45% Impe	rvious Area	
	Tc	Length	Slop		Velocity	Capacity	Description
_	(min)	(feet)	(ft/	'ft)	(ft/sec)	(cfs)	
	1.0	75	0.02	00	1.26		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	1.9	312	0.03	00	2.79		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	2.9	387	Tota	ıl, İr	ncreased	to minimum	Tc = 6.0 min

Summary for Subcatchment S30C: Subcatchment 30C

Runoff = 34.76 cfs @ 12.12 hrs, Volume= 2.927 af, Depth= 3.51"

	Area (sf)	CN	Description
*	360,000	98	
	24,598	39	>75% Grass cover, Good, HSG A
	34,063	74	>75% Grass cover, Good, HSG C
*	16,903	98	Water Surface
	435,564	93	Weighted Average
	58,661		13.47% Pervious Area
	376,903		86.53% Impervious Area

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	1.0	75	0.0200	1.26	(013)	Sheet Flow, A to B
		. •	0.0200	.,_0		Smooth surfaces n= 0.011 P2= 3.00"
	2.2	300	0.0200	2.28		Shallow Concentrated Flow, B to C
						Unpaved Kv= 16.1 fps
	1.8	200	0.0150	1.84		Shallow Concentrated Flow, C to D
						Grassed Waterway Kv= 15.0 fps
	0.6	30	0.0100	0.79		Sheet Flow, D to E
						Smooth surfaces n= 0.011 P2= 3.00"
	3.5	46	0.0600	0.22		Sheet Flow, E to F
_						Grass: Short n= 0.150 P2= 3.00"
	9.1	651	Total			

Summary for Subcatchment S32: Subcatchment 32

Runoff = 2.14 cfs @ 12.50 hrs, Volume= 0.334 af, Depth= 0.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Area (sf)	CN	De	escription		
*	19,706	98				
	30,613	74	>7	5% Grass	cover, Good	d, HSG C
	59,971	39	>7	'5% Grass (cover, Good	d, HSG A
	46,847	80	>7	'5% Grass (cover, Goo	d, HSG D
	46,898	30	W	oods, God	od, HSG A	
	4,056	70		oods, God	•	
	8,274	77	W	<u>oods, Goc</u>	od, HSG D	
	216,365	58	W	eighted A	verage	
	196,659 90.89% Pervious Area					
	19,706		9.	11% Imper	vious Area	
_		0.1				~
	ic Length		•		Capacity	Description
(mi	n) (feet)	(††,	/ft)	(ft/sec)	(cfs)	
16	.9 75	0.00	080	0.07		Sheet Flow, A to B
						Grass: Dense n= 0.240 P2= 3.00"
12	.1 360	0.00)50	0.49		Shallow Concentrated Flow, B to C
						Short Grass Pasture Kv= 7.0 fps
29	.0 435	Tota	lc			

Summary for Subcatchment \$40: Subcatchment 40

Runoff = 37.88 cfs @ 12.31 hrs, Volume= 4.159 af, Depth= 1.97"

A	rea (sf)	CN	Description			
*	158,513	98				
	31,707	55	Woods, God	od, HSG B		
4	217,840		Woods, God			
	189,606		Woods, Goo			
	950		Woods, God			
	26,357		>75% Grass (
	97,705	61	>75% Grass (cover, Goo	d, HSG B	
	190,000		>75% Grass (
	190,000	80	>75% Grass (cover, Goo	d, HSG D	
1,	102,678	76	Weighted A	verage		
Ç	944,165		85.62% Pervi	ous Area		
•	158,513		14.38% Impe	ervious Area		
Tc		Slop		•	Description	
(min)	(feet)	(ft/f	, , , , , ,	(cfs)		
1.1	75	0.015	0 1.12		Sheet Flow, A to B	
					Smooth surfaces n= 0.011 P2= 3.00"	
4.4	550	0.016	4 2.06		Shallow Concentrated Flow, B to C	
					Unpaved Kv= 16.1 fps	
1.6	325	0.050	0 3.35		Shallow Concentrated Flow, C to D	
					Grassed Waterway Kv= 15.0 fps	
14.3	2,100	0.003	0 2.45	73.44		
					Area= 30.0 sf Perim= 35.0' r= 0.86'	
					n= 0.030 Earth, grassed & winding	
21.4	3,050	Total				

Summary for Subcatchment S40A: Subcatchment 40A

Runoff = 24.61 cfs @ 12.18 hrs, Volume= 2.414 af, Depth= 3.73"

	Area (sf)	CN	Description
*	275,000	98	
	4,986	39	>75% Grass cover, Good, HSG A
	43,462	80	>75% Grass cover, Good, HSG D
	3,315	74	>75% Grass cover, Good, HSG C
*	11,757	98	Water Surface
	338,520	95	Weighted Average
	51,763		15.29% Pervious Area
	286,757		84.71% Impervious Area

<u>Page 36</u>

(r	Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	1.1	75	0.0150	1.12		Sheet Flow, A to B
		00.4	0.0150	1.07		Smooth surfaces n= 0.011 P2= 3.00"
	1.7	204	0.0150	1.97		Shallow Concentrated Flow, B to C Unpaved Kv= 16.1 fps
	7.0	629	0.0100	1.50		Shallow Concentrated Flow, C to D
	, .0	027	0.0100	1.00		Grassed Waterway Kv= 15.0 fps
	0.2	148	0.0400	14.40	45.24	
						24.0" Round Area = 3.1 sf Perim = 6.3' r = 0.50'
	1.9	37	0.0010	0.33		n= 0.013 Corrugated PE, smooth interior Sheet Flow, E to F
	1.7	3/	0.0010	0.55		Smooth surfaces n= 0.011 P2= 3.00"
	1.8	32	0.1600	0.30		Sheet Flow, F to G
						Grass: Short n= 0.150 P2= 3.00"
	13.7	1,125	Total			

Summary for Subcatchment S41: Subcatchment 41

Runoff = 22.20 cfs @ 12.10 hrs, Volume= 1.768 af, Depth= 3.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Ar	ea (sf)	CN	Description					
*	1	87,081	98						
	22,827 79 50-75% Grass cover, Fair,					s cover, Fair	, HSG C		
37,988 84 50-75% Grass cover, Fair, HSG D						, HSG D			
*		7,519 98 Water Surface							
	255,415		94	Weighted Average					
			23.81% Pervious Area						
	194,600 76.19% Impervi			.19% Impe	rvious Area				
	Tc	Length	Slop		•	Capacity	Description		
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)			
	1.3	75	0.010	00	0.95		Sheet Flow, A to B		
							Smooth surfaces n= 0.011 P2= 3.00"		
	3.3	383	0.01	43	1.93		Shallow Concentrated Flow, B to C		
							Unpaved Kv= 16.1 fps		
	2.5	243	0.05	35	1.62		Shallow Concentrated Flow, C to D		
_							Short Grass Pasture Kv= 7.0 fps		
	7.1	701	Tota						

Summary for Subcatchment \$42: Subcatchment 42

Runoff = 25.71 cfs @ 12.15 hrs, Volume= 2.231 af, Depth= 3.31"

	Ar	ea (sf)	CN	De	scription				
*	2	78,726	98		·				
*		11,139	98	Wc	iter Surfac	ce			
		26,977	49	50-	75% Grass	s cover, Fair	r, HSG A		
		29,325	69	50-	75% Grass	s cover, Fair	r, HSG B		
_		6,630	79	50-	75% Gras:	s cover, Fair	r, HSG C		
	352,797 91 Weighte			ighted A	verage				
	62,932			17.84% Pervious Area					
	289,865			82.	16% Impe	rvious Area			
	Tc	Length	Slop			Capacity	Description		
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)			
	1.3	75	0.010	OC	0.95		Sheet Flow, A to B		
							Smooth surfaces n= 0.011 P2= 3.00"		
	6.1	700	0.014	40	1.90		Shallow Concentrated Flow, B to C		
							Unpaved Kv= 16.1 fps		
	3.4	225	0.025	50	1.11		Shallow Concentrated Flow, C to D		
_							Short Grass Pasture Kv= 7.0 fps		
	10.8	1,000	Tota						

Summary for Subcatchment \$43: Subcatchment 43

Runoff = 15.40 cfs @ 12.39 hrs, Volume= 1.880 af, Depth= 1.82"

	Ar	ea (sf)	CN	De	Description							
*	2	32,052	98									
*		36,376	98	W	ater Surfac	ce						
	1	22,740	49	50	-75% Gras	s cover, Fair	r, HSG A					
		38,160	79	50	-75% Gras	s cover, Fair	r, HSG C					
		8,153	69	50	-75% Gras	s cover, Fair	r, HSG B					
		86,000	36	W	oods, Fair,	HSG A						
		5,923	60	W	oods, Fair,	HSG B						
		10,549	77	W	oods, Poo	r, HSG C						
539,953 74 Weighted Average						verage						
	2	71,525		50	.29% Pervi	ous Area						
	2	68,428		49	.71% Impe	rvious Area						
	Tc	Length	Slo		•	Capacity	Description					
(min)	(feet)	(ft <i>)</i>	/ft)	(ft/sec)	(cfs)						
	10.6	75	0.01	00	0.12		Sheet Flow, A to B					
							Grass: Short n= 0.150 P2= 3.00"					
	16.3	488	0.01	00	0.50		Shallow Concentrated Flow, B to C					
							Woodland Kv= 5.0 fps					
	26.9	563	Toto	lr								

<u>Page 38</u>

Summary for Subcatchment S50: Subcatchment 50

Runoff = 2.32 cfs @ 12.09 hrs, Volume= 0.168 af, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Area (sf)	CN	Description							
*	10,546	98								
	4,431	61	>75% Grass cover, Good, HSG B							
	24,859	74	>75% Grass cover, Good, HSG C							
	39,836	79	Weighted Average							
	29,290		73.53% Pervious Area							
	10,546		26.47% Impervious Area							
	Tc Length		ppe Velocity Capacity Description							
(m	iin) (feet)	(ft,	r/ft) (ft/sec) (cfs)							
(6.0		Direct Entry,							

Summary for Subcatchment S60: Subcatchment 60

Runoff = 1.75 cfs @ 13.03 hrs, Volume= 0.698 af, Depth= 0.28"

_	Ar	ea (sf)	CN	Description			
	8	41,049	30	Woods, Go	od, HSG A		
	3	44,078	77	Woods, Go	od, HSG D		
		22,871	70	Woods, Go	od, HSG C		
		37,335	39	>75% Grass	cover, Goo	d, HSG A	
	32,830 80 >75% Grass cover, Good, HSG D						
*		17,570	98				
_		15,415	77	Woods, Go	od, HSG D		
	1,3						
	1,2	93,578		98.66% Perv	ious Area		
	17,570 1.34% Impervious Area						
	Tc	Length	Slop	•	•	Description	
_	(min)	(feet)	(ft/1	ft) (ft/sec)	(cfs)		
	30.7	75	0.020	0.04		Sheet Flow, A to B	
						Woods: Dense underbrush n= 0.800 P2= 3.00"	
	14.4	530	0.015	0.61		Shallow Concentrated Flow, B to C	
						Woodland Kv= 5.0 fps	
	1.4	430	0.010	00 5.10	122.30	Channel Flow, C to D	
						Area= 24.0 sf Perim= 23.0' r= 1.04'	
_						n= 0.030 Earth, grassed & winding	
	46.5	1,035	Total				

Summary for Subcatchment S60A: Subcatchment 60A

Runoff = 0.05 cfs @ 23.05 hrs, Volume= 0.030 af, Depth= 0.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

Are	Area (sf) CN Description								
73	35,140	30	Wo	ods, God	d, HSG A				
71,251 70 Woods, Good, HSG C									
22,354 77 Woods, Good, HSG D									
	26,632	39	>75	% Grass o	cover, Good	d, HSG A			
	9,029	74	>75	% Grass o	cover, Good	d, HSG C			
86	34,406	35	We	ighted A	verage				
86	54,406		100.00% Pervious Area						
Tc	Length	Slop	oe '	Velocity	Capacity	Description			
(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)				
30.7	75	0.02	00	0.04		Sheet Flow, A to B			
						Woods: Dense underbrush n= 0.800 P2= 3.00"			
71.5	1,175	0.00	30	0.27		Shallow Concentrated Flow, B to C			
						Woodland Kv= 5.0 fps			
102.2	1,250	Tota	1						

Summary for Subcatchment S61: Subcatchment 61

Runoff = 12.71 cfs @ 12.09 hrs, Volume= 0.964 af, Depth= 3.41"

	Ar	ea (sf)	CN	Descrip	tion				
		1,448	86	<50% G	rass (cover, Poor,	HSG C		
		18,168	49	50-75%	Gras	s cover, Fair	r, HSG A		
*		8,570	98	Water S	urfa	ce			
*	1	19,736	98						
	147,922 92 Weighted Average								
19,616 13.26% Pervious Area									
	1	28,306		86.74%	Impe	ervious Area			
	Tc	Length	Slop	e Velc	city	Capacity	Description		
_	(min)	(feet)	(ft/f	t) (ft/s	sec)	(cfs)			
	1.0	75	0.020	00	1.26		Sheet Flow, A to B		
							Smooth surfaces n=0.011 P2=3.00"		
	3.7	500	0.020	00	2.28		Shallow Concentrated Flow, B to C		
							Unpaved Kv= 16.1 fps		
	1.2	100	0.008	30	1.34		Shallow Concentrated Flow, C to D		
_							Grassed Waterway Kv= 15.0 fps		
	5.9	675	Total	, Incred	sed	to minimum	Tc = 6.0 min		

Printed 2/2/2018

Page 40

Summary for Subcatchment S62: Subcatchment 62

Runoff = 25.35 cfs @ 12.09 hrs, Volume= 2.019 af, Depth= 3.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Ar	ea (sf)	CN	Descript	ion							
*		11,953	98	Water Si	urfa	ce						
*	2	36,076	98									
		20,275	84	50-75% (Gras	s cover, Fair	r, HSG D					
_		6,756	49	50-75% (<u> Gras</u>	s cover, Fair	r, HSG A					
275,060 96 Weighted Average												
		27,031		9.83% Pe	ervic	ous Area						
	248,029			90.17% Impervious Area								
	_		0.1				- · · ·					
	TC	Length	Slop		•	Capacity	Description					
_	(min)	(feet)	(ft/	t) (ft/s	ec)	(cfs)						
	1.1	75	0.015	50 1	.12		Sheet Flow, A to B					
							Smooth surfaces n= 0.011 P2= 3.00"					
	1.3	150	0.015	50 1	.97		Shallow Concentrated Flow, B to C					
							Unpaved Kv= 16.1 fps					
	2.5	600	0.030	00 4	1.05	24.32						
							Area= 6.0 sf Perim= 12.0' r= 0.50'					
_							n= 0.040 Earth, cobble bottom, clean sides					
	10	925	Tata	Increa	2	to minimum	$T_0 = 4.0 \text{ min}$					

^{4.9 825} Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment S63: Subcatchment 63

Runoff = 1.36 cfs @ 12.09 hrs, Volume= 0.099 af, Depth= 2.73"

_	Area (sf)	CN	Description								
*	15,000	98									
_	4,049	39	>75% Grass of	cover, Good	d, HSG A						
	19,049	85	Weighted Av	/erage							
	4,049		21.26% Pervi	1.26% Pervious Area							
	15,000		78.74% Impe	rvious Area							
	Tc Length		·	Capacity	Description						
_	(min) (feet)	(††,	/ft) (ft/sec)	(cfs)							
	6.0				Direct Entry,						

Summary for Subcatchment S64: Subcatchment 64

Runoff = 1.07 cfs @ 12.09 hrs, Volume= 0.079 af, Depth= 2.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Aı	rea (sf)	CN	Description								
-	*	11,520	98									
		2,572	39	>75% Grass	cover, Goo	d, HSG A						
		14,092										
		2,572		18.25% Pervi	8.25% Pervious Area							
		11,520		81.75% Impe	ervious Area							
	-		0.1									
	Tc	Length		be Velocity	. ,	Description						
	(min)	(feet)	(ft/	ft) (ft/sec)	(cfs)							
	6.0					Direct Entry						

Summary for Subcatchment S65: Subcatchment 65

Runoff = 1.44 cfs @ 12.10 hrs, Volume= 0.110 af, Depth= 1.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

	Aı	rea (sf)	CN	De	escription							
*		20,240	98									
		20,692	39	>7	5% Grass of	cover, Good	od, HSG A					
		40,932	68	We	eighted A	verage						
		20,692	50.55% Pervious Area									
		20,240		49	.45% Impe	rvious Area	a					
	Тс	Length	Slo	ре	Velocity	Capacity	Description					
(min)	(feet)										
	6.0						Direct Entry,					

Summary for Subcatchment S66: Subcatchment 66

Runoff = 0.40 cfs @ 12.11 hrs, Volume= 0.034 af, Depth= 0.91"

 Area (sf)	CN	Description
9,622	39	>75% Grass cover, Good, HSG A
 10,094	80	>75% Grass cover, Good, HSG D
19,716	60	Weighted Average
19,716		100.00% Pervious Area

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 42

Tc (min)	_	Slope (ft/ft)	,	Capacity (cfs)	Description
6.0	, ,		, , ,	, ,	Direct Entry,

Summary for Subcatchment S67: Subcatchment 67

0.022 af, Depth= 0.70" Runoff 0.22 cfs @ 12.12 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Yr Rainfall=4.30"

A	rea (sf)	CN	Des	Description				
	9,767	39	>75	>75% Grass cover, Good, HSG A				
	6,617	80	>75	>75% Grass cover, Good, HSG D				
	16,384	56	We	eighted Av	/erage			
	16,384		100	100.00% Pervious Area				
_								
Tc	Length	Slop	ре	Velocity	Capacity	Description		
(min)	(feet)	(ft/	/ft)	(ft/sec)	(cfs)			
6.0						Direct Entry,		

Summary for Reach 8R: Forested Buffer #4

0.453 ac, 0.00% Impervious, Inflow Depth = 0.79" for 10-Yr event Inflow Area =

0.030 af 0.39 cfs @ 12.16 hrs, Volume= Inflow

Outflow 0.10 cfs @ 13.26 hrs, Volume= 0.030 af, Atten= 74%, Lag= 66.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.06 fps, Min. Travel Time= 41.0 min

Avg. Velocity = 0.01 fps, Avg. Travel Time= 227.6 min

Peak Storage= 251 cf @ 12.57 hrs

Average Depth at Peak Storage= 0.07'

Bank-Full Depth= 0.10' Flow Area= 2.5 sf, Capacity= 0.20 cfs

25.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 150.0' Slope= 0.0400 '/'

Inlet Invert= 249.00', Outlet Invert= 243.00'

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 43

Summary for Reach 9R: Forested Buffer #5

Inflow Area = 0.376 ac, 0.00% Impervious, Inflow Depth = 0.56" for 10-Yr event

Inflow = 0.22 cfs @ 12.35 hrs, Volume= 0.018 af

Outflow = 0.04 cfs @ 14.25 hrs, Volume= 0.018 af, Atten= 83%, Lag= 114.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.04 fps, Min. Travel Time= 58.9 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 247.2 min

Peak Storage= 130 cf @ 13.29 hrs Average Depth at Peak Storage= 0.03'

Bank-Full Depth= 0.10' Flow Area= 2.5 sf, Capacity= 0.21 cfs

25.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 150.0' Slope= 0.0467 '/'

Inlet Invert= 249.00', Outlet Invert= 242.00'

Summary for Reach 34R: Wetland

Inflow Area = 3.396 ac, 86.74% Impervious, Inflow Depth > 3.25" for 10-Yr event

Inflow = 1.19 cfs @ 12.96 hrs, Volume= 0.920 af

Outflow = 0.50 cfs @ 19.94 hrs, Volume= 0.818 af, Atten= 58%, Lag= 419.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.05 fps, Min. Travel Time= 247.0 min Avg. Velocity = 0.03 fps, Avg. Travel Time= 431.3 min

Peak Storage= 7,466 cf @ 15.83 hrs Average Depth at Peak Storage= 0.09'

Bank-Full Depth= 0.10' Flow Area= 10.0 sf, Capacity= 0.57 cfs

100.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush

Length= 800.0' Slope= 0.0050 '/'

Inlet Invert= 245.00', Outlet Invert= 241.00'

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC Page 44

Summary for Reach 206R: Forested Buffer #3

Inflow Area = 0.940 ac, 49.45% Impervious, Inflow Depth = 1.40" for 10-Yr event

Inflow = 1.44 cfs @ 12.10 hrs, Volume= 0.110 af

Outflow = 0.67 cfs @ 12.77 hrs, Volume= 0.110 af, Atten= 53%, Lag= 39.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.06 fps, Min. Travel Time= 25.6 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 190.2 min

Peak Storage= 1,033 cf @ 12.34 hrs Average Depth at Peak Storage= 0.18'

Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.28 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 86.0' Slope= 0.0116 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Summary for Reach 207R: Forested Buffer #2

Inflow Area = 0.324 ac, 81.75% Impervious, Inflow Depth = 2.91" for 10-Yr event

Inflow = 1.07 cfs @ 12.09 hrs, Volume= 0.079 af

Outflow = 0.44 cfs @ 12.97 hrs, Volume= 0.079 af, Atten= 59%, Lag= 52.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.05 fps, Min. Travel Time= 39.3 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 282.1 min

Peak Storage= 1,038 cf @ 12.32 hrs Average Depth at Peak Storage= 0.15'

Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.25 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 109.0' Slope= 0.0092 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 45

Summary for Reach 208R: Forested Buffer #1

Inflow Area = 0.437 ac, 78.74% Impervious, Inflow Depth = 2.73" for 10-Yr event

Inflow = 1.36 cfs @ 12.09 hrs, Volume= 0.099 af

Outflow = 0.48 cfs @ 13.24 hrs, Volume= 0.099 af, Atten= 65%, Lag= 68.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.04 fps, Min. Travel Time= 51.1 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 332.0 min

Peak Storage= 1,462 cf @ 12.38 hrs Average Depth at Peak Storage= 0.17

Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.22 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 134.0' Slope= 0.0075 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Summary for Reach R1: Portion of Davis Brook

Inflow Area = 26.358 ac, 65.57% Impervious, Inflow Depth > 0.72" for 10-Yr event

Inflow = 15.19 cfs @ 12.26 hrs, Volume= 1.577 af

Outflow = 5.56 cfs @ 12.92 hrs, Volume= 1.570 af, Atten= 63%, Lag= 39.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.18 fps, Min. Travel Time= 22.6 min Avg. Velocity = 0.33 fps, Avg. Travel Time= 81.6 min

Peak Storage= 7,562 cf @ 12.54 hrs Average Depth at Peak Storage= 0.59'

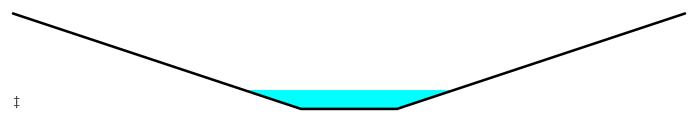
Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 176.49 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 1,600.0' Slope= 0.0031 '/'

Inlet Invert= 221.00', Outlet Invert= 216.00'



Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20, s/n 00734 © 2017 HydroCAD Soft

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC Page 46

Summary for Reach R2: Portion of Davis Brook

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.33" for 10-Yr event

Inflow = 0.99 cfs @ 15.95 hrs, Volume= 0.565 af

Outflow = 0.98 cfs @ 16.37 hrs, Volume= 0.563 af, Atten= 1%, Lag= 25.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.71 fps, Min. Travel Time= 14.1 min Avg. Velocity = 0.27 fps, Avg. Travel Time= 37.0 min

Peak Storage= 831 cf @ 16.13 hrs Average Depth at Peak Storage= 0.23' Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 182.28 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 600.0' Slope= 0.0033 '/'

Inlet Invert= 223.00', Outlet Invert= 221.00'



Summary for Reach R3: Natural Ditch

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.33" for 10-Yr event

Inflow = 0.99 cfs @ 15.85 hrs, Volume= 0.565 af

Outflow = 0.99 cfs @ 15.95 hrs, Volume= 0.565 af, Atten= 0%, Lag= 5.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.44 fps, Min. Travel Time= 3.5 min Avg. Velocity = 0.90 fps, Avg. Travel Time= 5.6 min

Peak Storage= 206 cf @ 15.89 hrs

Average Depth at Peak Storage= 0.07'

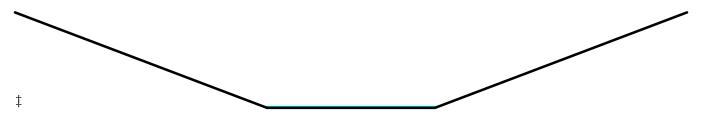
Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 998.06 cfs

10.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 5.0 '/' Top Width= 40.00'

Length= 300.0' Slope= 0.0433 '/'

Inlet Invert= 236.00', Outlet Invert= 223.00'



HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 47

Summary for Reach R4: Existing Ditch

12.396 ac, 49.71% Impervious, Inflow Depth = 0.00" for 10-Yr event Inflow Area =

Inflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

0.00 cfs @ 0.00 hrs, Volume= Outflow 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 168.09 cfs

8.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 20.00'

Length= 190.0' Slope= 0.0053 '/'

Inlet Invert= 244.00', Outlet Invert= 243.00'



Summary for Reach R5: Natural Ditch

Inflow Area = 5.864 ac, 76.19% Impervious, Inflow Depth > 2.08" for 10-Yr event

Inflow 30.34 cfs @ 12.20 hrs, Volume= 1.015 af

Outflow 15.19 cfs @ 12.26 hrs, Volume= 1.014 af, Atten= 50%, Lag= 3.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.60 fps, Min. Travel Time= 1.6 min

Avg. Velocity = 0.75 fps, Avg. Travel Time= 5.5 min

Peak Storage= 1,519 cf @ 12.23 hrs

Average Depth at Peak Storage= 0.22'

Bank-Full Depth= 3.00' Flow Area= 165.0 sf, Capacity= 1,945.52 cfs

25.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 10.0 '/' Top Width= 85.00'

Length= 250.0' Slope= 0.0320 '/'

Inlet Invert= 229.00', Outlet Invert= 221.00'



HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC Page 48

Summary for Pond 30A: Grassed Underdrained Soil Filter A

3.277 ac, 81.25% Impervious, Inflow Depth = 3.62" for 10-Yr event Inflow Area =

Inflow 12.75 cfs @ 12.09 hrs. Volume= 0.988 af

12.75 cfs @ 12.09 hrs, Volume= 0.988 af, Atten= 0%, Lag= 0.0 min Primary

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 30B: Grassed Underdrained Soil Filter B

Inflow Area = 4.812 ac, 94.45% Impervious, Inflow Depth = 3.73" for 10-Yr event

19.04 cfs @ 12.09 hrs. Volume= 1.495 af Inflow

19.04 cfs @ 12.09 hrs, Volume= 1.495 af, Atten= 0%, Lag= 0.0 min Primary

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 101: Pond #1 (plus wetland storage)

38.340 ac, 65.45% Impervious, Inflow Depth > 2.31" for 10-Yr event Inflow Area =

34.99 cfs @ 12.09 hrs, Volume= Inflow 7.396 af

Outflow 2.86 cfs @ 17.74 hrs, Volume= 5.166 af, Atten= 92%, Lag= 339.1 min =

Primary 2.86 cfs @ 17.74 hrs, Volume= 5.166 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 241.71'@ 17.74 hrs Surf.Area= 190,596 sf Storage= 177,920 cf

Plua-Flow detention time= 1,036.2 min calculated for 5.166 af (70% of inflow)

Center-of-Mass det. time=639.9 min (1,829.4 - 1,189.5)

Volume	Invert	Avail.Storage	Storage Description	
#1	240.00'	835,458 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
	0 (

E	Elevation	Surf.Area	Inc.Store	Cum.Store
	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
	240.00	22,686	0	0
	241.00	115,923	69,305	69,305
	242.00	221,289	168,606	237,911
	243.00	313,044	267,167	505,077
	244.00	347,717	330,381	835,458

Device	Routing	Invert	Outlet Devices
#1	Primary	237.64'	24.0" Round Culvert
			L= 87.0' CMP, projecting, no headwall, Ke= 0.900

Inlet / Outlet Invert= 237.64' / 237.30' S= 0.0039 '/' Cc= 0.900

n=0.025 Corrugated metal, Flow Area=3.14 sf

Device 1 241.00' **24.0" Vert. Orifice/Grate** C= 0.600

Primary OutFlow Max=2.86 cfs @ 17.74 hrs HW=241.71' (Free Discharge)

-1=Culvert (Passes 2.86 cfs of 16.10 cfs potential flow)

2=Orifice/Grate (Orifice Controls 2.86 cfs @ 2.87 fps)

Printed 2/2/2018

<u>Page 49</u>

Summary for Pond 102: Pond #2

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 1.82" for 10-Yr event

Inflow = 15.40 cfs @ 12.39 hrs, Volume= 1.880 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 244.67' @ 25.60 hrs Surf.Area= 36,354 sf Storage= 81,885 cf

Plua-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	242.00'	190,120 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.00	25,000	0	0
244.68	36,400	82,276	82,276
246.00	127,000	107,844	190,120

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 246.53'
 18.0" Round Culvert

 L= 165.0'
 CMP, projecting, no headwall, Ke= 0.900

 Inlet / Outlet Invert= 246.53' / 244.00'
 S= 0.0153'/'
 Cc= 0.900

 n= 0.025
 Corrugated metal, Flow Area= 1.77 sf

 #2
 Device 1
 248.05'
 24.0" Vert. Orifice/Grate
 C= 0.600

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=242.00' (Free Discharge)

□1=Culvert (Controls 0.00 cfs)

2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 103: Pond #3

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth = 1.31" for 10-Yr event

Inflow = 25.71 cfs @ 12.15 hrs, Volume= 2.231 af

Outflow = 0.99 cfs @ 15.85 hrs, Volume= 0.565 af, Atten= 96%, Lag= 222.3 min

Primary = 0.99 cfs @ 15.85 hrs, Volume= 0.565 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 241.42' @ 15.85 hrs Surf.Area= 24,457 sf Storage= 78,686 cf

Plug-Flow detention time=725.6 min calculated for 0.565 af (25% of inflow)

Center-of-Mass det. time=567.8 min (1,364.2 - 796.3)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1	234.00'	94,130 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 50

	Elevation	Surf.Area	Inc.Store	Cum.Store
_	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
	234.00	4,000	0	0
	236.00	7,000	11,000	11,000
	238.92	11,139	26,483	37,483
	240.00	14,287	13,730	51,213
	242.00	28.630	42.917	94.130

Device	Routing	Invert	Outlet Devices			
#1	Primary	234.22'	24.0" Round Culvert			
	,		L= 95.0' CMP, projecting, no headwall, Ke= 0.900			
			Inlet / Outlet Invert= 234.22' / 233.90' S= 0.0034 '/' Cc= 0.900			
			n=0.025 Corrugated metal, Flow Area=3.14sf			
#2	Device 1	241.29'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads			
#3	Device 1	236.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600			

Primary OutFlow Max=0.96 cfs @ 15.85 hrs HW=241.42' (Free Discharge)

-1=Culvert (Passes 0.96 cfs of 23.64 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.94 cfs @ 1.17 fps)

-3=Orifice/Grate (Orifice Controls 0.02 cfs @ 11.19 fps)

Summary for Pond 104: Pond #4

Inflow Area = 5.864 ac, 76.19% Impervious, Inflow Depth = 3.62" for 10-Yr event

22.20 cfs @ 12.10 hrs, Volume= 1.768 af Inflow =

Outflow 30.34 cfs @ 12.20 hrs, Volume= 1.015 af, Atten= 0%, Lag= 6.0 min

Primary 30.34 cfs @ 12.20 hrs, Volume= 1.015 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 241.63' @ 12.20 hrs Surf.Area= 16,786 sf Storage= 36,018 cf

Plug-Flow detention time= 354.0 min calculated for 1.014 af (57% of inflow)

Center-of-Mass det. time=249.4 min (1,028.3 - 778.9)

Volume	Inver	t Avail.Stor	age Storc	age Description	
#1	230.00)' 36,01	8 cf Cust	om Stage Data (Pi	rismatic) Listed below (Recalc)
Elevation (feet			Inc.Store ubic-feet)	Cum.Store (cubic-feet)	
230.0	0	620	0	0	
232.0	0	3,391	4,011	4,011	
234.0	5	7,519	11,183	15,194	
235.0	0	9,087	7,888	23,082	
236.0	0	16,786	12,937	36,018	
Device	Routing	Invert	Outlet De	evices	
#1	Primary	230.62'	24.0" Rou	und Culvert	
			L= 96.0'	CMP, projecting,	no headwall, Ke= 0.900
			Inlet / Ou	itlet Invert= 230.62	2' / 228.40' S= 0.0231 '/' Cc= 0.900
			n = 0.025	Corrugated meta	al, Flow Area= 3.14 sf
#2	Device 1	237.61'	24.0" Hor	iz. Orifice/Grate	C= 0.600 Limited to weir flow at low heads

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 51

#3 Device 1 231.00' **0.5" W x 0.5" H Vert. Orifice/Grate** C= 0.600

Primary OutFlow Max=30.34 cfs @ 12.20 hrs HW=241.63' (Free Discharge)

-1=Culvert (Passes 30.34 cfs of 33.59 cfs potential flow)
-2=Orifice/Grate (Orifice Controls 30.32 cfs @ 9.65 fps)

-3=Orifice/Grate (Orifice Controls 0.03 cfs @ 15.68 fps)

Summary for Pond 201: Pond #201

Inflow Area = 3.396 ac, 86.74% Impervious, Inflow Depth = 3.41" for 10-Yr event

Inflow = 12.71 cfs @ 12.09 hrs, Volume= 0.964 af

Outflow = 1.19 cfs @ 12.96 hrs, Volume= 0.920 af, Atten= 91%, Lag= 52.3 min

Primary = 1.19 cfs @ 12.96 hrs, Volume= 0.920 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.79' @ 12.96 hrs Surf.Area= 17,286 sf Storage= 25,290 cf

Plug-Flow detention time= 1,160.0 min calculated for 0.920 af (95% of inflow)

Center-of-Mass det. time=1,133.6 min (1,921.2 - 787.6)

Volume	Invert	Avail.Storage	Storage Description
#1	248.00'	47,142 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

E	Elevation	Surf.Area	Inc.Store	Cum.Store
_	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
	248.00	8,570	0	0
	249.00	15,880	12,225	12,225
	250.00	17,665	16,773	28,998
	251.00	18,624	18,145	47,142

Device	Routing	Invert	Outlet Devices
#1	Primary	250.00'	20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#2	Primary	245.40'	12.0" Round Culvert L= 29.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 245.40' / 245.00' S= 0.0138'/' Cc= 0.900
			n=0.013 Corrugated PE, smooth interior, Flow Area=0.79 sf
#3	Device 2	249.65'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	245.50'	1.6" Vert. Orifice/Grate C= 0.600
#5	Device 4	248.00'	2.000 in/hr Exfiltration over Surface area above 248.00'
			Excluded Surface area = 8,570 sf

Primary OutFlow Max=1.19 cfs @ 12.96 hrs HW=249.79' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Passes 1.19 cfs of 5.89 cfs potential flow)

-3=Orifice/Grate (Weir Controls 1.05 cfs @ 1.21 fps)
-4=Orifice/Grate (Orifice Controls 0.14 cfs @ 9.89 fps)

5=Exfiltration (Passes 0.14 cfs of 0.40 cfs potential flow)

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 52

Summary for Pond 202: Pond #202

Inflow Area = 6.315 ac, 90.17% Impervious, Inflow Depth = 3.84" for 10-Yr event

Inflow = 25.35 cfs @ 12.09 hrs, Volume= 2.019 af

Outflow = 2.98 cfs @ 12.70 hrs, Volume= 1.936 af, Atten= 88%, Lag= 36.9 min

Primary = 2.98 cfs @ 12.70 hrs, Volume= 1.936 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 243.75' @ 12.70 hrs Surf.Area= 26,067 sf Storage= 53,184 cf

Plug-Flow detention time= 1,045.8 min calculated for 1.936 af (96% of inflow)

Center-of-Mass det. time=1,021.6 min (1,787.7 - 766.1)

Volume	Invert	Avail.Storage	Storage Description
#1	241.00'	92,645 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
241.00	12,606	0	0
242.00	18,880	15,743	15,743
243.00	20,722	19,801	35,544
244.00	27,811	24,267	59,811
245.00	37,857	32,834	92,645

Device	Routing	Invert	Outlet Devices
#1	Primary	244.00'	25.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#2	Primary	238.40'	12.0" Round Culvert
			L= 115.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 238.40' / 237.75' S= 0.0057 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	243.50'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	238.50'	2.4" Vert. Orifice/Grate C= 0.600
#5	Device 4	241.00'	2.000 in/hr Exfiltration over Surface area above 241.00'
			Excluded Surface area = 12,606 sf

Primary OutFlow Max=2.97 cfs @ 12.70 hrs HW=243.75' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Passes 2.97 cfs of 6.01 cfs potential flow)

3=Orifice/Grate (Weir Controls 2.63 cfs @ 1.65 fps)

4=Orifice/Grate (Orifice Controls 0.34 cfs @ 10.93 fps)

5=Exfiltration (Passes 0.34 cfs of 0.62 cfs potential flow)

Summary for Pond 203: Pond #203

Inflow Area = 9.999 ac, 86.53% Impervious, Inflow Depth = 3.51" for 10-Yr event

Inflow = 34.76 cfs @ 12.12 hrs, Volume= 2.927 af

Outflow = 0.97 cfs @ 16.73 hrs, Volume= 1.966 af, Atten= 97%, Lag= 276.6 min

Primary = 0.97 cfs @ 16.73 hrs, Volume= 1.966 af

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 53

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.33' @ 16.73 hrs Surf.Area= 35,842 sf Storage= 101,763 cf

Plug-Flow detention time= 1,467.4 min calculated for 1.966 af (67% of inflow) Center-of-Mass det. time= 1,371.9 min (2,157.7 - 785.8)

Volume	Inver	t Avail.Stor	age Storaç	ge Description
#1	246.00)' 177,12	3 cf Custo	om Stage Data (Prismatic) Listed below (Recalc)
Elevation	n Su	ırf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft) (c	ubic-feet)	(cubic-feet)
246.00)	25,338	0	0
247.00)	28,403	26,871	26,871
248.00)	31,540	29,972	56,842
249.00)	34,748	33,144	89,986
250.00		38,027	36,388	126,374
251.00)	63,471	50,749	177,123
<u>Device</u>	Routing	Invert	Outlet Dev	evices
#1	Primary	249.75'	•	x 8.0' breadth Broad-Crested Rectangular Weir
			•	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
				4.00 4.50 5.00 5.50
				glish) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64
				2.66 2.66 2.68 2.70 2.74
#2	Primary	243.40'		and Culvert L= 65.0' CPP, projecting, no headwall, Ke= 0.900
				tlet Invert= 243.40' / 243.00' S= 0.0062'/' Cc= 0.900
				Corrugated PE, smooth interior, Flow Area = 0.79 sf
#3	Device 2			z. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2			Orifice/Grate C= 0.600
#5	Device 4	246.00'	•	r Exfiltration over Surface area above 246.00'
			Excluded S	Surface area = 25,338 sf

Primary OutFlow Max=0.96 cfs @ 16.73 hrs HW=249.33' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Passes 0.96 cfs of 6.96 cfs potential flow)

3=Orifice/Grate (Weir Controls 0.50 cfs @ 0.95 fps)

4=Orifice/Grate (Orifice Controls 0.46 cfs @ 11.52 fps)

5=Exfiltration (Passes 0.46 cfs of 0.49 cfs potential flow)

Summary for Pond 204: Pond #204

Inflow Area = 7.771 ac, 84.71% Impervious, Inflow Depth = 3.73" for 10-Yr event

Inflow = 24.61 cfs @ 12.18 hrs, Volume= 2.414 af

Outflow = 1.17 cfs @ 15.41 hrs, Volume= 2.004 af, Atten= 95%, Lag= 193.7 min

Primary = 1.17 cfs @ 15.41 hrs, Volume= 2.004 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 238.86' @ 15.41 hrs Surf.Area= 24,911 sf Storage= 77,925 cf

Plug-Flow detention time= 1,479.6 min calculated for 2.003 af (83% of inflow) Center-of-Mass det. time= 1,412.2 min (2,191.6 - 779.4)

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 54

<u>Volume</u>	Inver	t Avail.Stord	age Storag	ge Description
#1	235.00	106,90	3 cf Custor	m Stage Data (Prismatic) Listed below (Recalc)
Elevatio	n Su	rf.Area	Inc.Store	Cum.Store
(feet	-)	(sq-ft) (cı	ubic-feet)	(cubic-feet)
235.0	0	11,757	0	0
236.0	0	18,909	15,333	15,333
237.0	0	20,939	19,924	35,257
238.0	0	23,032	21,986	57,243
239.0	0	25,210	24,121	81,364
240.0	0	25,868	25,539	106,903
Device	Routing	Invert	Outlet Dev	vices
#1	Primary	239.00'	25.0' long	x 12.0' breadth Broad-Crested Rectangular Weir
			Head (fee	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (Eng	glish) 2.57 2.62 2.70 2.67 2.66 2.67 2.66 2.64
#2	Primary	232.40'		nd Culvert L= 93.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outl	let Invert= 232.40' / 232.00' S= 0.0043 '/' Cc= 0.900
				Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	232.50'	2.4" Vert. C	Orifice/Grate C= 0.600
#4	Device 2	238.75'	24.0" Horiz.	r. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 3	235.00'	2.000 in/hr	r Exfiltration over Surface area above 235.00'
			Excluded S	Surface area = 11,757 sf

Primary OutFlow Max=1.16 cfs @ 15.41 hrs HW=238.86' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Passes 1.16 cfs of 6.95 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.38 cfs @ 12.05 fps)

5=Exfiltration (Passes 0.38 cfs of 0.61 cfs potential flow)

-4=Orifice/Grate (Weir Controls 0.78 cfs @ 1.10 fps)

Summary for Pond 205: Level Lip Spreader #1

Inflow Area = 0.437 ac, 78.74% Impervious, Inflow Depth = 2.73" for 10-Yr event

Inflow = 1.36 cfs @ 12.09 hrs, Volume= 0.099 af

Outflow = 1.36 cfs @ 12.09 hrs, Volume= 0.099 af, Atten= 0%, Lag= 0.1 min

Primary = 1.36 cfs @ 12.09 hrs, Volume= 0.099 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.54' @ 12.09 hrs Surf.Area= 151 sf Storage= 6 cf

Plug-Flow detention time=0.1 min calculated for 0.099 af (100% of inflow)

Center-of-Mass det. time=0.1 min (813.6 - 813.5)

Volume	Invert	Avail.Storage	Storage Description
#1	242.50'	306 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 55

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.50	139	0	0
243.00	292	108	108
243.50	500	198	306

Device	Routing	Invert	Outlet Devices
#1	Primary	242.50'	65.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=1.33 cfs @ 12.09 hrs HW=242.54' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 1.33 cfs @ 0.51 fps)

Summary for Pond 206: Level Lip Spreader #2

Inflow Area = 0.324 ac, 81.75% Impervious, Inflow Depth = 2.91" for 10-Yr event

Inflow = 1.07 cfs @ 12.09 hrs, Volume= 0.079 af

Outflow = 1.07 cfs @ 12.09 hrs, Volume= 0.079 af, Atten= 0%, Lag= 0.1 min

Primary = 1.07 cfs @ 12.09 hrs, Volume= 0.079 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.53' @ 12.09 hrs Surf.Area= 150 sf Storage= 5 cf

Plug-Flow detention time=0.1 min calculated for 0.078 af (100% of inflow)

Center-of-Mass det. time=0.1 min (807.1 - 806.9)

Volume	Invert	Avail.Storc	ige Stora	ge Description	
#1	242.50'	306	of Custo	om Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)			Inc.Store (bic-feet)	Cum.Store (cubic-feet)	
242.50		139	0	0	
243.00		292	108	108	
243.50		500	198	306	
Device R	Routing	Invert	Outlet De	evices	
#1 P	Primary	242.50'	Head (fee 3.00 3.50	et) 0.20 0.40 0.40 glish) 2.54 2.61	Broad-Crested Rectangular Weir 60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

Primary OutFlow Max=1.04 cfs @ 12.09 hrs HW=242.53' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 1.04 cfs @ 0.47 fps)

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 56

Summary for Pond 207: Level Lip Spreader #3

Inflow Area = 0.940 ac, 49.45% Impervious, Inflow Depth = 1.40" for 10-Yr event

Inflow = 1.44 cfs @ 12.10 hrs, Volume= 0.110 af

Outflow = 1.44 cfs @ 12.10 hrs, Volume= 0.110 af, Atten= 0%, Lag= 0.1 min

Primary = 1.44 cfs @ 12.10 hrs, Volume= 0.110 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.54' @ 12.10 hrs Surf.Area= 152 sf Storage= 6 cf

Plug-Flow detention time=0.1 min calculated for 0.109 af (100% of inflow)

Center-of-Mass det. time=0.1 min (862.0 - 861.9)

Volume	Invert	Avail.Storage	Storage Description
#1	242.50'	306 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surt.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.50	139	0	0
243.00	292	108	108
243.50	500	198	306

Device Routing Invert Outlet Devices

#1 Primary 242.50' 65.0' long x 2.0' breadth Broad-Crested Rectangular Weir

 $\mbox{Head (feet)} \ \ 0.20 \ \ 0.40 \ \ 0.60 \ \ 0.80 \ \ 1.00 \ \ 1.20 \ \ 1.40 \ \ 1.60 \ \ 1.80 \ \ 2.00 \ \ 2.50$

3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

3.07 3.20 3.32

Primary OutFlow Max=1.43 cfs @ 12.10 hrs HW=242.54' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 1.43 cfs @ 0.52 fps)

Summary for Pond 208: Level Lip Spreader #4

Inflow Area = 0.453 ac, 0.00% Impervious, Inflow Depth = 0.91" for 10-Yr event

Inflow = 0.40 cfs @ 12.11 hrs, Volume= 0.034 af

Outflow = 0.39 cfs @ 12.16 hrs, Volume= 0.030 af, Atten= 2%, Lag= 3.3 min

Primary = 0.39 cfs @ 12.16 hrs, Volume= 0.030 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.03' @ 12.15 hrs Surf.Area= 300 sf Storage= 200 cf

Plug-Flow detention time=87.0 min calculated for 0.030 af (87% of inflow)

Center-of-Mass det. time=26.0 min (913.7 - 887.7)

Volume	Invert	Avail.Storage	Storage Description
#1	248.00'	200 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 57

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
248.00	100	0	0
248.50	200	75	75
249.00	300	125	200

Device Routing Invert Outlet Devices #1 Primary 249.00' 25.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50

3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

3.07 3.20 3.32

Primary OutFlow Max=0.34 cfs @ 12.16 hrs HW=249.03' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 0.34 cfs @ 0.44 fps)

Summary for Pond 209: Level Lip Spreader #5

0.376 ac, 0.00% Impervious, Inflow Depth = 0.70" for 10-Yr event Inflow Area =

0.22 cfs @ 12.12 hrs, Volume= Inflow 0.022 af

0.22 cfs @ 12.35 hrs, Volume= Outflow 0.018 af, Atten= 0%, Laa= 14.1 min =

Primary 0.22 cfs @ 12.35 hrs, Volume= 0.018 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.02' @ 12.35 hrs Surf.Area= 300 sf Storage= 200 cf

Plua-Flow detention time= 129.8 min calculated for 0.018 af (80% of inflow)

Center-of-Mass det. time=45.4 min (949.5 - 904.0)

Volume	Invert	Avail.Store	age Storag	e Description	
#1	248.00'	200	Ocf Custor	n Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet		.Area (sq-ft) (cu	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	
248.00)	100	0	0	
248.50	C	200	75	75	
249.00)	300	125	200	
Device	Routing	Invert	Outlet Dev	rices	
#1	Primary	249.00'	Head (feet 3.00 3.50	t) 0.20 0.40 0.4 lish) 2.54 2.61	Broad-Crested Rectangular Weir 60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

Primary OutFlow Max=0.21 cfs @ 12.35 hrs HW=249.02' (Free Discharge) T-1=Broad-Crested Rectangular Weir (Weir Controls 0.21 cfs @ 0.38 fps)

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20, s/n 00734 © 2017 HydroCAD Soft

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

<u>Page 58</u>

Summary for Pond 301: Point of Analysis 301

Inflow Area = 43.307 ac, 58.99% Impervious, Inflow Depth > 1.52" for 10-Yr event

Inflow = 3.06 cfs @ 17.26 hrs, Volume= 5.499 af

Primary = 3.06 cfs @ 17.26 hrs, Volume= 5.499 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 401: Point of Analysis 401

Inflow Area = 59.444 ac, 46.27% Impervious, Inflow Depth > 1.56" for 10-Yr event

Inflow = 38.23 cfs @ 12.31 hrs, Volume= 7.733 af

Primary = 38.23 cfs @ 12.31 hrs, Volume= 7.733 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 501: Point of Analysis 501

Inflow Area = 0.915 ac, 26.47% Impervious, Inflow Depth = 2.21" for 10-Yr event

Inflow = 2.32 cfs @ 12.09 hrs, Volume= 0.168 af

Primary = 2.32 cfs @ 12.09 hrs, Volume= 0.168 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 601: Point of Analysis 601

Inflow Area = 62.183 ac, 16.27% Impervious, Inflow Depth > 0.74" for 10-Yr event

Inflow = 5.98 cfs @ 12.99 hrs, Volume= 3.817 af

Primary = 5.98 cfs @ 12.99 hrs, Volume= 3.817 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link 1LA: Link 1

Inflow Area = 2.529 ac, 42.44% Impervious, Inflow Depth = 1.59" for 10-Yr event

Inflow = 1.47 cfs @ 13.05 hrs, Volume= 0.335 af

Primary = 1.47 cfs @ 13.05 hrs, Volume= 0.335 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link 1LB: Link 1

Inflow Area = 2.529 ac, 42.44% Impervious, Inflow Depth = 1.59" for 10-Yr event

Inflow = 1.47 cfs @ 13.05 hrs, Volume= 0.335 af

Primary = 1.47 cfs @ 13.05 hrs, Volume= 0.335 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Subcatchment \$30: Subcatchment 30

Runoff = 25.43 cfs @ 12.76 hrs, Volume= 4.385 af, Depth= 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

_	Αı	rea (sf)	CN	De	scription		
*	3	79,829	98				
	1	51,673	30	Wo	ods, God	od, HSG A	
	1	29,474	70	Wo	ods, God	od, HSG C	
*		22,394	98	Wo	ater Surfac	ce	
		99,405	39	>7.	5% Grass o	cover, Good	d, HSG A
_		99,405	74	>7.	5% Grass o	cover, Goo	d, HSG C
	8	82,180	73	We	eighted A	verage	
	4	79,957		54.	.41% Pervi	ous Area	
	4	02,223		45.	.59% Impe	rvious Area	
	Tc	Length	Slop	е	•	Capacity	Description
_	(min)	(feet)	(ft/f	†)	(ft/sec)	(cfs)	
	0.8	75	0.040	00	1.66		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	4.9	481	0.010)4	1.64		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	49.2	1,094	0.005	55	0.37		Shallow Concentrated Flow, C to D
_							Woodland Kv= 5.0 fps
	54.9	1,650	Total				

Summary for Subcatchment S30A: Subcatchment 30A

Runoff = 16.32 cfs @ 12.09 hrs, Volume= 1.284 af, Depth= 4.70"

_	Ar	ea (sf)	CN	Description		
*	1	15,960	98			
		24,290	79	50-75% Gras	s cover, Fair	r, HSG C
		2,477	49	50-75% Gras	s cover, Fair	r, HSG A
	1	42,727	94	Weighted A	verage	
		26,767		18.75% Perv	ious Area	
	1	15,960		81.25% Impe	ervious Area	
	Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description
	1.1	75	0.015	50 1.12		Sheet Flow, A to B
	2.8	333	0.015	50 1.97		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C
_						Unpaved Kv= 16.1 fps
	39	408	Total	Increased	to minimum	$T_{C} = 6.0 \text{ min}$

Summary for Subcatchment S30B: Subcatchment 30B

Runoff = 24.25 cfs @ 12.09 hrs, Volume= 1.931 af, Depth= 4.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Ar	ea (sf)	CN	De	escription		
*	1	97,982	98				
		947	79	50-	-75% Gras	s cover, Fair	r, HSG C
		10,676	49	50-	-75% Gras	s cover, Fair	r, HSG A
	2	09,605	95	We	eighted A	verage	
		11,623		5.5	55% Pervio	us Area	
	1	97,982		94.	.45% Impe	rvious Area	
	Tc	Length	Slop		Velocity	' '	Description
	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)	
	1.0	75	0.020	00	1.26		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	1.9	312	0.030	00	2.79		Shallow Concentrated Flow, B to C
_							Unpaved Kv= 16.1 fps
	2.9	387	Tota	l, Ir	ncreased	to minimum	TC = 6.0 min

Summary for Subcatchment S30C: Subcatchment 30C

Runoff = 44.76 cfs @ 12.12 hrs, Volume= 3.826 af, Depth= 4.59"

	Area (sf)	CN	Description
*	360,000	98	
	24,598	39	>75% Grass cover, Good, HSG A
	34,063	74	>75% Grass cover, Good, HSG C
*	16,903	98	Water Surface
	435,564	93	Weighted Average
	58,661		13.47% Pervious Area
	376,903		86.53% Impervious Area

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	1.0	75	0.0200	1.26	, ,	Sheet Flow, A to B
						Smooth surfaces n= 0.011 P2= 3.00"
	2.2	300	0.0200	2.28		Shallow Concentrated Flow, B to C
						Unpaved Kv= 16.1 fps
	1.8	200	0.0150	1.84		Shallow Concentrated Flow, C to D
						Grassed Waterway Kv= 15.0 fps
	0.6	30	0.0100	0.79		Sheet Flow, D to E
						Smooth surfaces n= 0.011 P2= 3.00"
	3.5	46	0.0600	0.22		Sheet Flow, E to F
_						Grass: Short n= 0.150 P2= 3.00"
	9.1	651	Total			

Summary for Subcatchment S32: Subcatchment 32

Runoff = 4.17 cfs @ 12.46 hrs, Volume= 0.577 af, Depth= 1.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Area (sf)	CN	De	escription		
*	19,706	98				
	30,613	74	>7	'5% Grass o	cover, Good	d, HSG C
	59,971	39	>7	'5% Grass o	cover, Good	d, HSG A
	46,847	80	>7	'5% Grass o	cover, Good	d, HSG D
	46,898	30	W	oods, God	od, HSG A	
	4,056	70	W	oods, God	od, HSG C	
	8,274	77	W	oods, God	od, HSG D	
	216,365	58	W	eighted A	verage	
	196,659		90	.89% Pervi	ous Area	
	19,706		9.	11% Imper	vious Area	
1	c Length			Velocity	. ,	Description
(mi	<u>n) (feet)</u>	(ft/	′ft)	(ft/sec)	(cfs)	
16	.9 75	0.00	80	0.07		Sheet Flow, A to B
						Grass: Dense n= 0.240 P2= 3.00"
12	.1 360	0.00	50	0.49		Shallow Concentrated Flow, B to C
						Short Grass Pasture Kv= 7.0 fps
29	.0 435	Toto	ıl			

Summary for Subcatchment \$40: Subcatchment 40

Runoff = 55.60 cfs @ 12.30 hrs, Volume= 6.051 af, Depth= 2.87"

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Paae	6

	Area (sf)	CN	De	escription		
*	158,513	98				
	31,707	55	W	oods, God	od, HSG B	
	217,840	70	W	oods, God	od, HSG C	
	189,606	77		oods, God		
	950	30		oods, God		
	26,357	39			cover, Goo	
	97,705	61			cover, Goo	
	190,000	74			cover, Goo	
	190,000	80	>7	5% Grass of	cover, Goo	d, HSG D
	1,102,678	76		eighted A	•	
	944,165			.62% Pervi		
	158,513		14	.38% Impe	rvious Area	
_		01				
	c Length		•	•	•	Description
<u>(mi</u>			/ft)	(ft/sec)	(cfs)	
1	.1 75	0.0	150	1.12		Sheet Flow, A to B
				0.04		Smooth surfaces n= 0.011 P2= 3.00"
4	.4 550	0.0	164	2.06		Shallow Concentrated Flow, B to C
,	, , ,	- 00		0.05		Unpaved Kv= 16.1 fps
I	.6 325	0.0	500	3.35		Shallow Concentrated Flow, C to D
1.4	0 0 100		220	0.45	70.44	Grassed Waterway Kv= 15.0 fps
14	.3 2,100	0.0	J30	2.45	73.44	•
						Area = 30.0 sf Perim = 35.0' r = 0.86'
	4 005					n= 0.030 Earth, grassed & winding
21	.4 3,050) Tot	al			

Summary for Subcatchment S40A: Subcatchment 40A

Runoff = 31.36 cfs @ 12.18 hrs, Volume= 3.119 af, Depth= 4.82"

	Area (sf)	CN	Description
*	275,000	98	
	4,986	39	>75% Grass cover, Good, HSG A
	43,462	80	>75% Grass cover, Good, HSG D
	3,315	74	>75% Grass cover, Good, HSG C
*	11,757	98	Water Surface
	338,520	95	Weighted Average
	51,763		15.29% Pervious Area
	286,757		84.71% Impervious Area

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 63

T (mir	c Length	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.	1 75	0.0150	1.12	, ,	Sheet Flow, A to B
					Smooth surfaces n=0.011 P2=3.00"
1.	7 204	0.0150	1.97		Shallow Concentrated Flow, B to C
					Unpaved Kv= 16.1 fps
7.	0 629	0.0100	1.50		Shallow Concentrated Flow, C to D
					Grassed Waterway Kv= 15.0 fps
0.	2 148	0.0400	14.40	45.24	• • • • • • • • • • • • • • • • • • • •
					24.0" Round Area = 3.1 sf Perim = 6.3' r = 0.50'
	0 07	0.0010	0.00		n= 0.013 Corrugated PE, smooth interior
1.	9 37	0.0010	0.33		Sheet Flow, E to F
1	0 20	0.1700	0.20		Smooth surfaces n= 0.011 P2= 3.00"
1.	8 32	0.1600	0.30		Sheet Flow, F to G
					Grass: Short n= 0.150 P2= 3.00"
13.	7 1,125	Total			

Summary for Subcatchment S41: Subcatchment 41

Runoff = 28.43 cfs @ 12.10 hrs, Volume= 2.298 af, Depth= 4.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Ar	ea (sf)	CN	De	scription							
*	1	87,081	98									
		22,827	79	50-	0-75% Grass cover, Fair, HSG C							
		37,988	84	50-	0-75% Grass cover, Fair, HSG D							
*		7,519	98	Wo	ater Surfac	ce						
	255,415 94 Weighted Average					verage						
	60,815 23.81% Pervious Area											
	194,600 76.19% Impervious Area											
	Tc	Length			•	Capacity	Description					
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)						
	1.3	75	0.01	00	0.95		Sheet Flow, A to B					
							Smooth surfaces n= 0.011 P2= 3.00"					
	3.3	383	0.01	43	1.93		Shallow Concentrated Flow, B to C					
							Unpaved Kv= 16.1 fps					
	2.5	243	0.05	35	1.62		Shallow Concentrated Flow, C to D					
_							Short Grass Pasture Kv= 7.0 fps					
	7.1	701	Tota	ı								

Summary for Subcatchment \$42: Subcatchment 42

Runoff = 33.51 cfs @ 12.15 hrs, Volume= 2.950 af, Depth= 4.37"

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page	64

	Ar	ea (sf)	CN	De	scription							
*	2	78,726	98									
*		11,139	98	W	ater Surfac	ce						
		26,977	49	50-	-75% Gras	s cover, Fair	r, HSG A					
		29,325	69	50-	-75% Gras	s cover, Fair	r, HSG B					
_	6,630 79 50-75% Grass cover, Fair,						r, HSG C					
	352,797 91 Weighted Average				eighted A	verage						
	62,932 17.84% Pervious Area											
	289,865			82.16% Impervious Area								
	Тс	Length	Slop	ре	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)						
	1.3	75	0.010	00	0.95		Sheet Flow, A to B					
							Smooth surfaces n= 0.011 P2= 3.00"					
	6.1	700	0.01	40	1.90		Shallow Concentrated Flow, B to C					
							Unpaved Kv= 16.1 fps					
	3.4	225	0.02	50	1.11		Shallow Concentrated Flow, C to D					
_							Short Grass Pasture Kv= 7.0 fps					
	10.8	1,000	Tota	I								

Summary for Subcatchment \$43: Subcatchment 43

Runoff = 23.05 cfs @ 12.38 hrs, Volume= 2.776 af, Depth= 2.69"

	Ar	ea (sf)	CN	De	escription		
*	2	32,052	98				
*		36,376	98	W	ater Surfac	ce	
	1	22,740	49	50	-75% Gras	s cover, Fair	r, HSG A
		38,160	79	50	-75% Gras	s cover, Fair	r, HSG C
		8,153	69	50	-75% Gras	s cover, Fair	r, HSG B
		000,68	36	W	oods, Fair,	HSG A	
		5,923	60		oods, Fair,		
		10,549	77	Wo	oods, Poo	r, HSG C	
	539,953 74 Weighted Average						
	2	71,525		50	.29% Pervi	ous Area	
	2	68,428		49	.71% Impe	rvious Area	
	Tc	Length			Velocity	•	Description
(min)	(feet)	(ft/	/ft)	(ft/sec)	(cfs)	
	10.6	75	0.01	00	0.12		Sheet Flow, A to B
							Grass: Short n= 0.150 P2= 3.00"
	16.3	488	0.01	00	0.50		Shallow Concentrated Flow, B to C
							Woodland Kv= 5.0 fps
	26.9	563	Tota	lr			

Summary for Subcatchment S50: Subcatchment 50

Runoff = 3.31 cfs @ 12.09 hrs, Volume= 0.240 af, Depth= 3.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Area (sf)	CN	Description
*	10,546	98	
	4,431	61	>75% Grass cover, Good, HSG B
	24,859	74	>75% Grass cover, Good, HSG C
	39,836	79	Weighted Average
	29,290		73.53% Pervious Area
	10,546		26.47% Impervious Area
(m	Tc Length nin) (feet)		pe Velocity Capacity Description /ft) (ft/sec) (cfs)
	6.0		Direct Entry,

Summary for Subcatchment S60: Subcatchment 60

Runoff = 6.24 cfs @ 12.85 hrs, Volume= 1.580 af, Depth= 0.63"

	Ar	ea (sf)	CN	De	escription				
	8	41,049	30	W	oods, God	od, HSG A			
	344,078 77 Woods, Good, HSG D								
	22,871 70 Woods, Good, HSG C								
	37,335 39 >75% Grass cover, Good, HSG A								
		32,830	80	>7	5% Grass of	cover, Good	d, HSG D		
*		17,570	98						
_		15,415	77	W	oods, God	od, HSG D			
	1,311,148 46 Weighted Average								
	1,2	93,578		98	.66% Pervi	ous Area			
	17,570 1.34% Impervious Area								
		Length	Slop		-		Description		
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)			
	30.7	75	0.020	00	0.04		Sheet Flow, A to B		
							Woods: Dense underbrush n= 0.800 P2= 3.00"		
	14.4	530	0.013	50	0.61		Shallow Concentrated Flow, B to C		
							Woodland Kv= 5.0 fps		
	1.4	430	0.010	00	5.10	122.30	·		
							Area= 24.0 sf Perim= 23.0' r= 1.04'		
_							n= 0.030 Earth, grassed & winding		
	46.5	1.035	Tota	l					

Summary for Subcatchment S60A: Subcatchment 60A

Runoff = 0.35 cfs @ 16.20 hrs, Volume= 0.232 af, Depth= 0.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

Are	ea (sf)	CN	Des	scription						
73	5,140 30 Woods, Good, HSG A									
7	71,251	70	70 Woods, Good, HSG C							
	22,354	77	Wo	ods, Goo	d, HSG D					
	26,632	39	>75	% Grass o	cover, Good	d, HSG A				
	9,029	74	>75	% Grass o	cover, Good	d, HSG C				
864,406 35 Weighted Average										
86	54,406		100.00% Pervious Area							
Tc	Length	Slop	oe '	Velocity	Capacity	Description				
(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)					
30.7	75	0.02	00	0.04		Sheet Flow, A to B				
						Woods: Dense underbrush n= 0.800 P2= 3.00"				
71.5	1,175	0.00	30	0.27		Shallow Concentrated Flow, B to C				
						Woodland Kv= 5.0 fps				
102.2	1,250	Tota	1							

Summary for Subcatchment S61: Subcatchment 61

Runoff = 16.46 cfs @ 12.09 hrs, Volume= 1.268 af, Depth= 4.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Ar	ea (sf)	CN	Description						
	1,448 86 <50% Grass cover, Poor, HSG C									
		18,168	3 49 50-75% Grass cover, Fair, HSG A							
*		8,570	98	Water Surfc	/ater Surface					
*	1	19,736	98							
	1	47,922	92	Weighted A	Average					
		19,616		13.26% Per	vious Area					
	1	28,306		86.74% Imp	ervious Area					
	Tc	Length	Slop	e Velocity	Capacity	Description				
_	(min)	(feet)	(ft/	t) (ft/sec	(cfs)					
	1.0	75	0.020	00 1.26)	Sheet Flow, A to B				
						Smooth surfaces n= 0.011 P2= 3.00"				
	3.7	500	0.020	00 2.28	}	Shallow Concentrated Flow, B to C				
						Unpaved Kv= 16.1 fps				
	1.2	100	0.008	30 1.34		Shallow Concentrated Flow, C to D				
						Grassed Waterway Kv= 15.0 fps				
_	5.9	475	Tota	Increased	l to minimum	$\Delta T_{\rm C} = 6.0 \text{min}$				

5.9 675 Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment S62: Subcatchment 62

Runoff = 32.14 cfs @ 12.09 hrs, Volume= 2.594 af, Depth= 4.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Ar	ea (sf)	CN	De	scription					
*		11,953	98	W	Water Surface					
*	2	36,076	98							
		20,275	84	50-	0-75% Grass cover, Fair, HSG D					
_		6,756	49	9 50-75% Grass cover, Fair, HSG A						
	275,060 96 Weighted Average									
		27,031		9.8	3% Pervio	us Area				
248,029 90.17% Impervious Area										
	TC	Length			•	Capacity	Description			
	(min)	(feet)	(ft/	††)	(ft/sec)	(cfs)				
	1.1	75	0.01	50	1.12		Sheet Flow, A to B			
							Smooth surfaces n= 0.011 P2= 3.00"			
	1.3	150	0.01	50	1.97		Shallow Concentrated Flow, B to C			
							Unpaved Kv= 16.1 fps			
	2.5	600	0.03	00	4.05	24.32	Channel Flow, C to D			
							Area= 6.0 sf Perim= 12.0' r= 0.50'			
							n= 0.040 Earth, cobble bottom, clean sides			
	4.9	825	Tota	otal, Increased to minimum Tc = 6.0 min						

Runoff = 1.85 cfs @ 12.09 hrs, Volume= 0.136 af, Depth= 3.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

_	Area (sf)	CN	Description							
*	15,000	98								
_	4,049	39	>75% Grass (cover, Good	d, HSG A					
	19,049									
	4,049		21.26% Pervious Area							
	15,000		78.74% Impe	rvious Area						
	Ta lavasida	. CI.		C = = = :h .	Description					
	Tc Length		. ,	Capacity	Description					
_	(min) (feet)) (11	/ft) (ft/sec)	(cfs)						
	6.0				Direct Entry,					

Summary for Subcatchment S63: Subcatchment 63

Summary for Subcatchment S64: Subcatchment 64

Runoff = 1.43 cfs @ 12.09 hrs, Volume= 0.106 af, Depth= 3.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Aı	rea (sf)	CN	Description					
-	*	11,520	98						
		2,572	39	>75% Grass	cover, Goo	d, HSG A			
		14,092	87	Weighted A	verage				
		2,572 18.25% Pervious Area							
		11,520		81.75% Impe	ervious Area				
	-		0.1						
	Tc	Length		be Velocity	. ,	Description			
	(min)	(feet)	(ft/	ft) (ft/sec)	(cfs)				
	6.0					Direct Entry			

Summary for Subcatchment S65: Subcatchment 65

Runoff = 2.30 cfs @ 12.10 hrs, Volume= 0.170 af, Depth= 2.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Are	ea (sf)	CN	De	Description				
*	2	20,240	98						
	4	20,692	39	>75% Grass cover, Good, HSG A					
		40,932	68	We	eighted A	verage			
	20,692 50.55% Pervious Area								
	20,240 49.45% Impervious Area						ea		
1		Length			,	. ,	,		
<u>(r</u>	nin)	(feet)	(ft/	TT)	(ft/sec)	(cfs)			
	6.0						Direct Entry,		

Summary for Subcatchment S66: Subcatchment 66

Runoff = 0.74 cfs @ 12.10 hrs, Volume= 0.058 af, Depth= 1.54"

Area (sf)	CN	Description
9,622	39	>75% Grass cover, Good, HSG A
 10,094	80	>75% Grass cover, Good, HSG D
19,716	60	Weighted Average
19,716		100.00% Pervious Area

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 69

	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
_	6.0 Direct Entry,					Direct Entry,

Summary for Subcatchment S67: Subcatchment 67

0.47 cfs @ 12.11 hrs, Volume= 0.039 af, Depth= 1.25" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Yr Rainfall=5.40"

	Area (sf)	CN	Description				
	9,767	39	>75% Grass cover, Good, HSG A				
	6,617	80	>75% Grass cover, Good, HSG D				
	16,384	56	Weighted A	verage			
	16,384	16,384 100.00% Pervious Area					
To	c Length	Slo	pe Velocity	Capacity	Description		
(mir) (feet)	(ft,	/ft) (ft/sec)	(cfs)			
6.	0				Direct Entry,		

Summary for Reach 8R: Forested Buffer #4

0.453 ac, 0.00% Impervious, Inflow Depth = 1.44" for 25-Yr event Inflow Area =

0.98 cfs @ 12.15 hrs, Volume= 0.054 af Inflow

Outflow 0.29 cfs @ 12.92 hrs, Volume= 0.054 af, Atten= 70%, Lag= 46.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.09 fps, Min. Travel Time= 27.5 min

Avg. Velocity = 0.01 fps, Avg. Travel Time= 196.6 min

Peak Storage= 480 cf @ 12.40 hrs

Average Depth at Peak Storage= 0.13'

Bank-Full Depth= 0.10' Flow Area= 2.5 sf, Capacity= 0.20 cfs

25.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 150.0' Slope= 0.0400 '/'

Inlet Invert= 249.00', Outlet Invert= 243.00'

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

<u>Page 70</u>

Summary for Reach 9R: Forested Buffer #5

Inflow Area = 0.376 ac, 0.00% Impervious, Inflow Depth = 1.13" for 25-Yr event

Inflow = 0.71 cfs @ 12.15 hrs, Volume= 0.035 af

Outflow = 0.15 cfs @ 13.07 hrs, Volume= 0.035 af, Atten= 79%, Lag= 55.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.08 fps, Min. Travel Time= 33.3 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 208.8 min

Peak Storage= 306 cf @ 12.52 hrs Average Depth at Peak Storage= 0.08'

Bank-Full Depth= 0.10' Flow Area= 2.5 sf, Capacity= 0.21 cfs

25.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 150.0' Slope= 0.0467 '/'

Inlet Invert= 249.00', Outlet Invert= 242.00'

Summary for Reach 34R: Wetland

Inflow Area = 3.396 ac, 86.74% Impervious, Inflow Depth > 4.32" for 25-Yr event

Inflow = 4.31 cfs @ 12.45 hrs, Volume= 1.221 af

Outflow = 1.07 cfs @ 17.34 hrs, Volume= 1.128 af, Atten= 75%, Lag= 293.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.07 fps, Min. Travel Time= 191.5 min Avg. Velocity = 0.03 fps, Avg. Travel Time= 403.4 min

Peak Storage= 12,342 cf @ 14.15 hrs Average Depth at Peak Storage= 0.15'

Bank-Full Depth= 0.10' Flow Area= 10.0 sf, Capacity= 0.57 cfs

100.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush

Length= 800.0' Slope= 0.0050 '/'

Inlet Invert= 245.00', Outlet Invert= 241.00'

Prepared by Stantec Consulting Services, Inc.
HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

<u>Page 71</u>

Summary for Reach 206R: Forested Buffer #3

Inflow Area = 0.940 ac, 49.45% Impervious, Inflow Depth = 2.17" for 25-Yr event

Inflow = 2.30 cfs @ 12.10 hrs, Volume= 0.170 af

Outflow = 1.13 cfs @ 12.69 hrs, Volume= 0.170 af, Atten= 51%, Lag= 35.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.06 fps, Min. Travel Time= 23.3 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 168.9 min

Peak Storage= 1,582 cf @ 12.30 hrs Average Depth at Peak Storage= 0.28'

Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.28 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 86.0' Slope= 0.0116 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Summary for Reach 207R: Forested Buffer #2

Inflow Area = 0.324 ac, 81.75% Impervious, Inflow Depth = 3.95" for 25-Yr event

Inflow = 1.43 cfs @ 12.09 hrs, Volume= 0.106 af

Outflow = 0.61 cfs @ 12.91 hrs, Volume= 0.106 af, Atten= 57%, Lag= 48.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.05 fps, Min. Travel Time= 36.3 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 257.4 min

Peak Storage= 1,332 cf @ 12.30 hrs Average Depth at Peak Storage= 0.19'

Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.25 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 109.0' Slope= 0.0092 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 72

Summary for Reach 208R: Forested Buffer #1

Inflow Area = 0.437 ac, 78.74% Impervious, Inflow Depth = 3.74" for 25-Yr event

Inflow = 1.85 cfs @ 12.09 hrs, Volume= 0.136 af

Outflow = 0.67 cfs @ 13.16 hrs, Volume= 0.136 af, Atten= 64%, Lag= 63.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.05 fps, Min. Travel Time= 47.6 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 303.0 min

Peak Storage= 1,922 cf @ 12.36 hrs Average Depth at Peak Storage= 0.22'

Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.22 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 134.0' Slope= 0.0075 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Summary for Reach R1: Portion of Davis Brook

Inflow Area = 26.358 ac, 65.57% Impervious, Inflow Depth > 1.29" for 25-Yr event

Inflow = 27.39 cfs @ 12.15 hrs, Volume= 2.826 af

Outflow = 12.13 cfs @ 12.66 hrs, Volume= 2.819 af, Atten= 56%, Lag= 30.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.47 fps, Min. Travel Time= 18.2 min Avg. Velocity = 0.35 fps, Avg. Travel Time= 75.9 min

Peak Storage= 13,260 cf @ 12.36 hrs Average Depth at Peak Storage= 0.88'

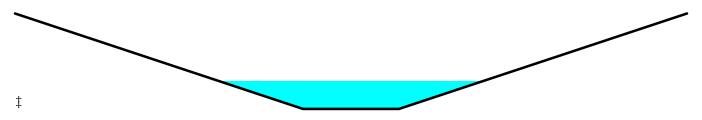
Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 176.49 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 1,600.0' Slope= 0.0031 '/'

Inlet Invert= 221.00', Outlet Invert= 216.00'



Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC Page 73

Summary for Reach R2: Portion of Davis Brook

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.75" for 25-Yr event

Inflow 3.52 cfs @ 13.12 hrs. Volume= 1.284 af

3.44 cfs @ 13.44 hrs, Volume= Outflow 1.282 af, Atten= 2%, Lag= 19.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.05 fps, Min. Travel Time= 9.5 min Avg. Velocity = 0.31 fps, Avg. Travel Time= 32.4 min

Peak Storage= 1,971 cf @ 13.28 hrs Average Depth at Peak Storage= 0.45'

Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 182.28 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 600.0' Slope= 0.0033 '/'

Inlet Invert= 223.00', Outlet Invert= 221.00'



Summary for Reach R3: Natural Ditch

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 0.75" for 25-Yr event

Inflow 3.53 cfs @ 13.05 hrs, Volume= 1.284 af

Outflow 3.52 cfs @ 13.12 hrs, Volume= 1.284 af, Atten= 0%, Lag= 3.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.31 fps, Min. Travel Time= 2.2 min

Avg. Velocity = 0.97 fps, Avg. Travel Time= 5.2 min

Peak Storage= 457 cf @ 13.08 hrs

Average Depth at Peak Storage= 0.14'

Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 998.06 cfs

10.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 5.0 '/' Top Width= 40.00'

Length= 300.0' Slope= 0.0433 '/'

Inlet Invert= 236.00', Outlet Invert= 223.00'



Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 74

Summary for Reach R4: Existing Ditch

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 0.00" for 25-Yr event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 168.09 cfs

8.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 20.00'

Length= 190.0' Slope= 0.0053 '/'

Inlet Invert= 244.00', Outlet Invert= 243.00'



Summary for Reach R5: Natural Ditch

Inflow Area = 5.864 ac, 76.19% Impervious, Inflow Depth > 3.16" for 25-Yr event

Inflow = 29.03 cfs @ 12.10 hrs, Volume= 1.544 af

Outflow = 27.39 cfs @ 12.15 hrs, Volume= 1.544 af, Atten= 6%, Lag= 3.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.32 fps, Min. Travel Time= 1.3 min

Avg. Velocity = 0.77 fps, Avg. Travel Time= 5.4 min

Peak Storage= 2,334 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.33'

Bank-Full Depth= 3.00' Flow Area= 165.0 sf, Capacity= 1,945.52 cfs

25.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 10.0 '/' Top Width= 85.00'

Length= 250.0' Slope= 0.0320 '/'

Inlet Invert= 229.00', Outlet Invert= 221.00'



<u>HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC</u>

Page 75

Summary for Pond 30A: Grassed Underdrained Soil Filter A

Inflow Area = 3.277 ac, 81.25% Impervious, Inflow Depth = 4.70" for 25-Yr event

Inflow = 16.32 cfs @ 12.09 hrs, Volume= 1.284 af

Primary = 16.32 cfs @ 12.09 hrs, Volume= 1.284 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 30B: Grassed Underdrained Soil Filter B

Inflow Area = 4.812 ac, 94.45% Impervious, Inflow Depth = 4.82" for 25-Yr event

Inflow = 24.25 cfs @ 12.09 hrs, Volume= 1.931 af

Primary = 24.25 cfs @ 12.09 hrs, Volume= 1.931 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 101: Pond #1 (plus wetland storage)

Inflow Area = 38.340 ac, 65.45% Impervious, Inflow Depth > 3.27" for 25-Yr event

Inflow = 46.16 cfs @ 12.09 hrs, Volume= 10.457 af

Outflow = 5.86 cfs @ 16.14 hrs, Volume= 8.216 af, Atten= 87%, Lag= 242.9 min

Primary = 5.86 cfs @ 16.14 hrs, Volume= 8.216 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 242.05' @ 16.14 hrs Surf.Area= 226,132 sf Storage= 249,717 cf

Plug-Flow detention time=823.0 min calculated for 8.216 af (79% of inflow)

Center-of-Mass det. time=540.2 min (1,629.3 - 1,089.1)

Volume	Invert	Avail.Storage	Storage Description	
#1	240.00'	835,458 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Flevation	Surf	Area Inc.:	Store Cum Store	

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
240.00	22,686	0	0
241.00	115,923	69,305	69,305
242.00	221,289	168,606	237,911
243.00	313,044	267,167	505,077
244.00	347,717	330,381	835,458

Device	Routing	Invert	Outlet Devices	
#1	Primary	237.64'	24.0" Round Culvert	
			L= 87.0' CMP, projecting, no headwall, Ke= 0.900	
			Inlet / Outlet Invert= 237.64' / 237.30' S= 0.0039 '/' Cc= 0.900	
			n=0.025 Corrugated metal, Flow Area=3.14sf	

#2 Device 1 241.00' **24.0" Vert. Orifice/Grate** C= 0.600

Primary OutFlow Max=5.86 cfs @ 16.14 hrs HW=242.05' (Free Discharge)

1=Culvert (Passes 5.86 cfs of 17.21 cfs potential flow)

12=Orifice/Grate (Orifice Controls 5.86 cfs @ 3.49 fps)

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 76

Summary for Pond 102: Pond #2

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 2.69" for 25-Yr event

Inflow = 23.05 cfs @ 12.38 hrs, Volume= 2.776 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 245.34' @ 25.60 hrs Surf.Area= 81,374 sf Storage= 120,862 cf

Plua-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	242.00'	190,120 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.00	25,000	0	0
244.68	36,400	82,276	82,276
246.00	127,000	107,844	190,120

Device	Routing	Invert	Outlet Devices	
#1	Primary	246.53'	18.0" Round Culvert	
			L= 165.0' CMP, projecting, no headwall, Ke= 0.900	
			Inlet / Outlet Invert= 246.53' / 244.00' S= 0.0153 '/' Cc= 0.900	
			n=0.025 Corrugated metal, Flow Area=1.77 sf	
#2	Device 1	248.05'	24.0" Vert. Orifice/Grate C= 0.600	

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=242.00' (Free Discharge)

1=Culvert (Controls 0.00 cfs)

12=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 103: Pond #3

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth = 1.73" for 25-Yr event

Inflow = 33.51 cfs @ 12.15 hrs, Volume= 2.950 af

Outflow = 3.53 cfs @ 13.05 hrs, Volume= 1.284 af, Atten= 89%, Lag= 54.4 min

Primary = 3.53 cfs @ 13.05 hrs, Volume= 1.284 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 241.60' @ 13.05 hrs Surf.Area= 25,745 sf Storage= 83,192 cf

Plug-Flow detention time= 426.6 min calculated for 1.283 af (43% of inflow)

Center-of-Mass det. time=305.3 min (1,094.1 - 788.8)

Volume	Invert	Avail.Storage	Storage Description
#1	234.00'	94,130 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 77

Elevation Surf.Area		Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
234.00	4,000	0	0
236.00	7,000	11,000	11,000
238.92	11,139	26,483	37,483
240.00	14,287	13,730	51,213
242.00	28,630	42,917	94,130

Device	Routing	Invert	Outlet Devices		
#1	Primary	234.22'	24.0" Round Culvert		
	•		L= 95.0' CMP, projecting, no headwall, Ke= 0.900		
			Inlet / Outlet Invert= 234.22' / 233.90' S= 0.0034 '/' Cc= 0.900		
			n=0.025 Corrugated metal, Flow Area=3.14sf		
#2	Device 1	241.29'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#3	Device 1	236.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600		

Primary OutFlow Max=3.53 cfs @ 13.05 hrs HW=241.60' (Free Discharge)

-1=Culvert (Passes 3.53 cfs of 24.02 cfs potential flow)

2=Orifice/Grate (Weir Controls 3.51 cfs @ 1.81 fps)

3=Orifice/Grate (Orifice Controls 0.02 cfs @ 11.37 fps)

Summary for Pond 104: Pond #4

Inflow Area = 5.864 ac, 76.19% Impervious, Inflow Depth = 4.70" for 25-Yr event

Inflow = 28.43 cfs @ 12.10 hrs, Volume= 2.298 af

Outflow = 29.03 cfs @ 12.10 hrs, Volume= 1.544 af, Atten= 0%, Lag= 0.0 min

Primary = 29.03 cfs @ 12.10 hrs, Volume= 1.544 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 241.29' @ 12.10 hrs Surf.Area= 16,786 sf Storage= 36,018 cf

Plug-Flow detention time= 271.3 min calculated for 1.543 af (67% of inflow)

Center-of-Mass det. time= 177.7 min (950.1 - 772.4)

#2

Device 1

Volume	Inve	rt Avail.Sto	rage Stor	orage Description
#1	230.00	0,36	18 cf Cus	stom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet	-)		Inc.Store	(cubic-feet)
230.0	-	620	0	•
232.0	-	3,391	4,011	·
234.0	5	7,519	11,183	3 15,194
235.0	0	9,087	7,888	8 23,082
236.0	0	16,786	12,937	7 36,018
Device	Routing	Inver	t Outlet D	Devices
#1	Primary	230.62	24.0" Rc	ound Culvert
			Inlet / O	CMP, projecting, no headwall, Ke= 0.900 Dutlet Invert= 230.62' / 228.40' S= 0.0231 '/' Cc= 0.900 5 Corrugated metal, Flow Area= 3.14 sf

237.61' **24.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 78

#3 Device 1 231.00' **0.5" W x 0.5" H Vert. Orifice/Grate** C= 0.600

Primary OutFlow Max=29.02 cfs @ 12.10 hrs HW=241.28' (Free Discharge)

-1=Culvert (Passes 29.02 cfs of 33.07 cfs potential flow)

—2=Orifice/Grate (Orifice Controls 28.99 cfs @ 9.23 fps) **—3=Orifice/Grate** (Orifice Controls 0.03 cfs @ 15.42 fps)

Summary for Pond 201: Pond #201

Inflow Area = 3.396 ac, 86.74% Impervious, Inflow Depth = 4.48" for 25-Yr event

Inflow = 16.46 cfs @ 12.09 hrs, Volume= 1.268 af

Outflow = 4.31 cfs @ 12.45 hrs, Volume= 1.221 af, Atten= 74%, Lag= 22.0 min

Primary = 4.31 cfs @ 12.45 hrs, Volume= 1.221 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 250.00' @ 12.45 hrs Surf.Area= 17,657 sf Storage= 28,916 cf

Plug-Flow detention time=892.4 min calculated for 1.221 af (96% of inflow)

Center-of-Mass det. time=872.3 min (1,652.7 - 780.3)

Volume	Invert	Avail.Storage	Storage Description
#1	248.00'	47,142 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
248.00	8,570	0	0
249.00	15,880	12,225	12,225
250.00	17,665	16,773	28,998
251.00	18,624	18,145	47,142

Device	Routing	Invert	Outlet Devices
#1	Primary	250.00'	20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#2	Primary	245.40'	12.0" Round Culvert L= 29.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 245.40' / 245.00' S= 0.0138'/' Cc= 0.900
			n=0.013 Corrugated PE, smooth interior, Flow Area=0.79 sf
#3	Device 2	249.65'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	245.50'	1.6" Vert. Orifice/Grate C= 0.600
#5	Device 4	248.00'	2.000 in/hr Exfiltration over Surface area above 248.00'
			Excluded Surface area = 8,570 sf

Primary OutFlow Max=4.31 cfs @ 12.45 hrs HW=250.00' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Passes 4.31 cfs of 6.04 cfs potential flow)

-3=Orifice/Grate (Weir Controls 4.17 cfs @ 1.92 fps)

4=Orifice/Grate (Orifice Controls 0.14 cfs @ 10.13 fps)

5=Exfiltration (Passes 0.14 cfs of 0.42 cfs potential flow)

Printed 2/2/2018

Page 79

Summary for Pond 202: Pond #202

Inflow Area = 6.315 ac, 90.17% Impervious, Inflow Depth = 4.93" for 25-Yr event

Inflow = 32.14 cfs @ 12.09 hrs, Volume= 2.594 af

Outflow = 7.48 cfs @ 12.48 hrs, Volume= 2.508 af, Atten= 77%, Lag= 23.6 min

Primary = 7.48 cfs @ 12.48 hrs, Volume= 2.508 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 244.07' @ 12.48 hrs Surf.Area= 28,559 sf Storage= 61,910 cf

Plug-Flow detention time=835.4 min calculated for 2.508 af (97% of inflow)

Center-of-Mass det. time=815.1 min (1,575.7 - 760.6)

Volume	Invert	Avail.Storage	Storage Description
#1	241.00'	92,645 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
241.00	12,606	0	0
242.00	18,880	15,743	15,743
243.00	20,722	19,801	35,544
244.00	27,811	24,267	59,811
245.00	37,857	32,834	92,645

Device	Routing	Invert	Outlet Devices
#1	Primary	244.00'	25.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#2	Primary	238.40'	12.0" Round Culvert
			L= 115.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 238.40' / 237.75' S= 0.0057'/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	243.50'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	238.50'	2.4" Vert. Orifice/Grate C= 0.600
#5	Device 4	241.00'	2.000 in/hr Exfiltration over Surface area above 241.00'
			Excluded Surface area = 12,606 sf

Primary OutFlow Max=7.43 cfs @ 12.48 hrs HW=244.07' (Free Discharge)

1=Broad-Crested Rectangular Weir (Weir Controls 1.24 cfs @ 0.67 fps)

-2=Culvert (Barrel Controls 6.19 cfs @ 7.89 fps)

3=Orifice/Grate (Passes < 8.92 cfs potential flow)

4=Orifice/Grate (Passes < 0.35 cfs potential flow)

5=Exfiltration (Passes < 0.74 cfs potential flow)

Summary for Pond 203: Pond #203

Inflow Area = 9.999 ac, 86.53% Impervious, Inflow Depth = 4.59" for 25-Yr event

Inflow = 44.76 cfs @ 12.12 hrs, Volume= 3.826 af

Outflow = 3.79 cfs @ 13.23 hrs, Volume= 2.857 af, Atten= 92%, Lag= 66.1 min

Primary = 3.79 cfs @ 13.23 hrs, Volume= 2.857 af

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 80

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.55' @ 13.23 hrs Surf.Area= 36,541 sf Storage= 109,471 cf

Plug-Flow detention time= 1,075.0 min calculated for 2.857 af (75% of inflow) Center-of-Mass det. time= 989.8 min (1,768.7 - 778.9)

Volume	Inver	t Avail.Stor	age Stora	ge Description
#1	246.00)' 177,12	3 cf Custo	om Stage Data (Prismatic) Listed below (Recalc)
Elevation	n Su	rf.Area	Inc.Store	Cum.Store
(feet		(sq-ft) (c	ubic-feet)	(cubic-feet)
246.00		25,338	0	0
247.00		28,403	26,871	26,871
248.00)	31,540	29,972	56,842
249.00		34,748	33,144	89,986
250.00		38,027	36,388	126,374
251.00)	63,471	50,749	177,123
Device	Routing	Invert	Outlet De	evices
#1	Primary	249.75		x 8.0' breadth Broad-Crested Rectangular Weir
# 1	Tilliary	247./3	_	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			•	4.00 4.50 5.00 5.50
				glish) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64
			•	2.66 2.66 2.68 2.70 2.74
#2	Primary	243.40'		und Culvert L= 65.0' CPP, projecting, no headwall, Ke= 0.900
	, , , , ,			tlet Invert= 243.40' / 243.00' S= 0.0062'/' Cc= 0.900
				Corrugated PE, smooth interior, Flow Area = 0.79 sf
#3	Device 2	249.25'		z. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	243.50'	2.7" Vert.	Orifice/Grate C= 0.600
#5	Device 4	246.00'	2.000 in/h	nr Exfiltration over Surface area above 246.00'
			Excluded	Surface area = 25,338 sf

Primary OutFlow Max=3.79 cfs @ 13.23 hrs HW=249.55' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Passes 3.79 cfs of 7.09 cfs potential flow)

3=Orifice/Grate (Weir Controls 3.32 cfs @ 1.78 fps)

4=Orifice/Grate (Orifice Controls 0.47 cfs @ 11.73 fps)

5=Exfiltration (Passes 0.47 cfs of 0.52 cfs potential flow)

Summary for Pond 204: Pond #204

Inflow Area = 7.771 ac, 84.71% Impervious, Inflow Depth = 4.82" for 25-Yr event

Inflow = 31.36 cfs @ 12.18 hrs, Volume= 3.119 af

Outflow = 5.18 cfs @ 12.80 hrs, Volume= 2.701 af, Atten= 83%, Lag= 37.1 min

Primary = 5.18 cfs @ 12.80 hrs, Volume= 2.701 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 239.07' @ 12.80 hrs Surf.Area= 25,253 sf Storage= 83,027 cf

Plug-Flow detention time= 1,134.8 min calculated for 2.701 af (87% of inflow) Center-of-Mass det. time= 1,075.0 min (1,848.4 - 773.4)

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

<u>Page 81</u>

Volume	Invert	Avail.Stora	ge Storaç	ge Description
#1	235.00'	106,903	cf Custo	om Stage Data (Prismatic) Listed below (Recalc)
Elevation	n Sur	f.Area	Inc.Store	Cum.Store
(feet	.)	(sq-ft) (cu	bic-feet)	(cubic-feet)
235.0	0	11,757	0	0
236.0	0	18,909	15,333	15,333
237.0	0 :	20,939	19,924	35,257
238.0	0 :	23,032	21,986	57,243
239.0	0	25,210	24,121	81,364
240.0	0	25,868	25,539	106,903
<u>Device</u>	Routing	Invert	Outlet Dev	vices
#1	Primary	239.00'	25.0' long	x 12.0' breadth Broad-Crested Rectangular Weir
			Head (fee	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (Eng	glish) 2.57 2.62 2.70 2.67 2.66 2.67 2.66 2.64
#2	Primary	232.40'	12.0" Rour	nd Culvert L= 93.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outl	tlet Invert= 232.40' / 232.00' S= 0.0043 '/' Cc= 0.900
			n = 0.013 (Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	232.50'	2.4" Vert. C	Orifice/Grate C= 0.600
#4	Device 2	238.75'	24.0" Horiz	z. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 3	235.00'	2.000 in/hr	r Exfiltration over Surface area above 235.00'
			Excluded S	Surface area = 11,757 sf

Primary OutFlow Max=5.12 cfs @ 12.80 hrs HW=239.07' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Weir Controls 1.09 cfs @ 0.66 fps)

-2=Culvert (Passes 4.03 cfs of 7.07 cfs potential flow)

3=Orifice/Grate (Orifice Controls 0.38 cfs @ 12.24 fps)

5=Exfiltration (Passes 0.38 cfs of 0.62 cfs potential flow)

-4=Orifice/Grate (Weir Controls 3.65 cfs @ 1.84 fps)

Summary for Pond 205: Level Lip Spreader #1

Inflow Area = 0.437 ac, 78.74% Impervious, Inflow Depth = 3.74" for 25-Yr event

Inflow = 1.85 cfs @ 12.09 hrs, Volume= 0.136 af

Outflow = 1.85 cfs @ 12.09 hrs, Volume= 0.136 af, Atten= 0%, Lag= 0.1 min

Primary = 1.85 cfs @ 12.09 hrs, Volume= 0.136 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 242.55' @ 12.09 hrs Surf.Area= 154 sf Storage= 7 cf

Plug-Flow detention time=0.1 min calculated for 0.136 af (100% of inflow)

Center-of-Mass det. time=0.1 min (804.7 - 804.6)

Volume	Invert	Avail.Storage	Storage Description
#1	242.50'	306 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 82

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.50	139	0	0
243.00	292	108	108
243.50	500	198	306

Device Routing Invert Outlet Devices

#1 Primary 242.50' 65.0' long x 2.0' breadth Broad-Crested Rectangular Weir

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=1.81 cfs @ 12.09 hrs HW=242.55' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 1.81 cfs @ 0.56 fps)

Summary for Pond 206: Level Lip Spreader #2

Inflow Area = 0.324 ac, 81.75% Impervious, Inflow Depth = 3.95" for 25-Yr event

Inflow = 1.43 cfs @ 12.09 hrs, Volume= 0.106 af

Outflow = 1.43 cfs @ 12.09 hrs, Volume= 0.106 af, Atten= 0%, Lag= 0.1 min

Primary = 1.43 cfs @ 12.09 hrs, Volume= 0.106 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.54' @ 12.09 hrs Surf.Area= 152 sf Storage= 6 cf

Plug-Flow detention time=0.1 min calculated for 0.106 af (100% of inflow)

lavant Avail Change a Change Decamation

Center-of-Mass det. time=0.1 min (798.5 - 798.4)

Volume	Inve	rt Avail.Stor	age Storac	ge Description
#1	242.50	0' 30	6 cf Custo	om Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet		urf.Area (sq-ft) (c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)
242.5 243.0	Ö	139 292	0 108	0 108
243.5		500	198	306
<u>Device</u>	Routing	Invert	Outlet Dev	evices
#1	Primary	242.50'	Head (fee 3.00 3.50	x 2.0' breadth Broad-Crested Rectangular Weir et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 glish) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

Primary OutFlow Max=1.40 cfs @ 12.09 hrs HW=242.54' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 1.40 cfs @ 0.52 fps)

3.07 3.20 3.32

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 83

Summary for Pond 207: Level Lip Spreader #3

Inflow Area = 0.940 ac, 49.45% Impervious, Inflow Depth = 2.17" for 25-Yr event

Inflow = 2.30 cfs @ 12.10 hrs, Volume= 0.170 af

Outflow = 2.30 cfs @ 12.10 hrs, Volume= 0.170 af, Atten= 0%, Lag= 0.0 min

Primary = 2.30 cfs @ 12.10 hrs, Volume= 0.170 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.56' @ 12.10 hrs Surf.Area= 157 sf Storage= 9 cf

Plug-Flow detention time=0.1 min calculated for 0.170 af (100% of inflow)

Center-of-Mass det. time=0.1 min (848.7 - 848.6)

Volume	Invert	Avail.Storage	Storage Description
#1	242.50'	306 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.50	139	0	0
243.00	292	108	108
243.50	500	198	306

Device Routing Invert Outlet Devices

#1 Primary 242.50' 65.0' long x 2.0' breadth Broad-Crested Rectangular Weir

 $\mbox{Head (feet)} \ \ 0.20 \ \ 0.40 \ \ 0.60 \ \ 0.80 \ \ 1.00 \ \ 1.20 \ \ 1.40 \ \ 1.60 \ \ 1.80 \ \ 2.00 \ \ 2.50$

3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

3.07 3.20 3.32

Primary OutFlow Max=2.28 cfs @ 12.10 hrs HW=242.56' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 2.28 cfs @ 0.61 fps)

Summary for Pond 208: Level Lip Spreader #4

Inflow Area = 0.453 ac, 0.00% Impervious, Inflow Depth = 1.54" for 25-Yr event

Inflow = 0.74 cfs @ 12.10 hrs, Volume= 0.058 af

Outflow = 0.98 cfs @ 12.15 hrs, Volume= 0.054 af, Atten= 0%, Lag= 2.6 min

Primary = 0.98 cfs @ 12.15 hrs, Volume= 0.054 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.06' @ 12.14 hrs Surf.Area= 300 sf Storage= 200 cf

Plug-Flow detention time= 47.3 min calculated for 0.054 af (93% of inflow)

Center-of-Mass det. time= 13.0 min (882.8 - 869.8)

Volume	Invert	Avail.Storage	Storage Description
#1	248.00'	200 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 84

I	Elevation	Surf.Area	Inc.Store	Cum.Store
	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
	248.00	100	0	0
	248.50	200	75	75
	249.00	300	125	200

Device Routing Invert Outlet Devices

#1 Primary 249.00' 25.0' long x 2.0' breadth Broad-Crested Rectangular Weir

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50

3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

3.07 3.20 3.32

Primary OutFlow Max=0.92 cfs @ 12.15 hrs HW=249.06' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 0.92 cfs @ 0.62 fps)

Summary for Pond 209: Level Lip Spreader #5

Inflow Area = 0.376 ac, 0.00% Impervious, Inflow Depth = 1.25" for 25-Yr event

Inflow = 0.47 cfs @ 12.11 hrs, Volume= 0.039 af

Outflow = 0.71 cfs @ 12.15 hrs, Volume= 0.035 af, Atten= 0%, Lag= 2.4 min

Primary = 0.71 cfs @ 12.15 hrs, Volume= 0.035 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.05' @ 12.15 hrs Surf.Area= 300 sf Storage= 200 cf

Plua-Flow detention time= 68.6 min calculated for 0.035 af (90% of inflow)

Center-of-Mass det. time= 19.9 min (902.2 - 882.3)

Volume	Invert	Avail.Storag	e Storag	e Description	
#1	248.00'	200 c	f Custor	n Stage Data (P	rismatic) Listed below (Recalc)
Elevation	Surf.A	rea In	c.Store	Cum.Store	
(feet)	(sc	q-ft) (cub	ic-feet)	(cubic-feet)	
248.00		100	0	0	
248.50		200	75	75	
249.00	;	300	125	200	
.					
Device R	outina	Invert (Outlet Dev	ices	

Device	Routing	Invert	Outlet Devices
#1	Primary	249.00'	25.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

@ 12 15 hrs HW-249 05' (Froe Discha

Primary OutFlow Max=0.68 cfs @ 12.15 hrs HW=249.05' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 0.68 cfs @ 0.56 fps)

3.07 3.20 3.32

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

lutions LLC Page 85

Summary for Pond 301: Point of Analysis 301

Inflow Area = 43.307 ac, 58.99% Impervious, Inflow Depth > 2.44" for 25-Yr event

Inflow = 6.31 cfs @ 15.69 hrs, Volume= 8.794 af

Primary = 6.31 cfs @ 15.69 hrs, Volume= 8.794 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 401: Point of Analysis 401

Inflow Area = 59.444 ac, 46.27% Impervious, Inflow Depth > 2.34" for 25-Yr event

Inflow = 55.98 cfs @ 12.30 hrs, Volume= 11.571 af

Primary = 55.98 cfs @ 12.30 hrs, Volume= 11.571 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 501: Point of Analysis 501

Inflow Area = 0.915 ac, 26.47% Impervious, Inflow Depth = 3.15" for 25-Yr event

Inflow = 3.31 cfs @ 12.09 hrs, Volume= 0.240 af

Primary = 3.31 cfs @ 12.09 hrs, Volume= 0.240 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 601: Point of Analysis 601

Inflow Area = 62.183 ac, 16.27% Impervious, Inflow Depth > 1.15" for 25-Yr event

Inflow = 14.82 cfs @ 12.86 hrs, Volume= 5.950 af

Primary = 14.82 cfs @ 12.86 hrs, Volume= 5.950 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link 1LA: Link 1

Inflow Area = 2.529 ac, 42.44% Impervious, Inflow Depth = 2.38" for 25-Yr event

Inflow = 2.54 cfs @ 12.95 hrs, Volume= 0.502 af

Primary = 2.54 cfs @ 12.95 hrs, Volume= 0.502 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link 1LB: Link 1

Inflow Area = 2.529 ac, 42.44% Impervious, Inflow Depth = 2.38" for 25-Yr event

Inflow = 2.54 cfs @ 12.95 hrs, Volume= 0.502 af

Primary = 2.54 cfs @ 12.95 hrs, Volume= 0.502 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Subcatchment S30: Subcatchment 30

Runoff = 43.95 cfs @ 12.75 hrs, Volume= 7.522 af, Depth= 4.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

	Ar	ea (sf)	CN	De	escription		
*	3	79,829	98				
	1	51,673	30	Wo	oods, Goo	od, HSG A	
	1	29,474	70	Wo	oods, Goo	od, HSG C	
*		22,394	98	Wo	ater Surfac	ce	
		99,405	39	>7.	5% Grass o	cover, Good	d, HSG A
_		99,405	74	>7.	5% Grass o	cover, Good	d, HSG C
	8	82,180	73	We	eighted A	verage	
		79,957		54.	.41% Pervi	ous Area	
	4	02,223		45.	.59% Impe	rvious Area	
	_						
	TC	Length	Slop		·	Capacity	Description
_	(min)	(feet)	(ft/	††)	(ft/sec)	(cfs)	
	0.8	75	0.04	00	1.66		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	4.9	481	0.01	04	1.64		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	49.2	1,094	0.00	55	0.37		Shallow Concentrated Flow, C to D
_							Woodland Kv= 5.0 fps
	54.9	1,650	Tota				

Summary for Subcatchment S30A: Subcatchment 30A

Runoff = 23.40 cfs @ 12.09 hrs, Volume= 1.880 af, Depth= 6.88"

_	Ar	rea (sf)	CN	Descri	iption		
*	1	15,960	98				
		24,290	79	50-759	% Grass	s cover, Fair	, HSG C
_		2,477	49	50-759	% Grass	s cover, Fair	, HSG A
	1	42,727	94	Weigh	nted Av	verage	
		26,767		18.75%	% Pervi	ous Area	
	1	15,960		81.25%	% Impe	rvious Area	
	Tc (min)	Length (feet)	Slop (ft/		elocity t/sec)	Capacity (cfs)	Description
	1.1	75	0.01	50	1.12		Sheet Flow, A to B
	2.8	333	0.01	50	1.97		Smooth surfaces n= 0.011 P2= 3.00" Shallow Concentrated Flow, B to C Unpaved Kv= 16.1 fps
	3 9	408	Tota	Incre	ansed t	to minimum	$T_{C} = 6.0 \text{ min}$

<u>Page 87</u>

Summary for Subcatchment S30B: Subcatchment 30B

Runoff = 34.59 cfs @ 12.09 hrs, Volume= 2.808 af, Depth= 7.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

	Ar	ea (sf)	CN	De	escription		
*	1	97,982	98				
		947	79	50-	-75% Gras	s cover, Fair	, HSG C
		10,676	49	50-	-75% Gras	s cover, Fair	r, HSG A
	2	09,605	95	We	eighted A	verage	
		11,623		5.5	55% Pervio	us Area	
	1	97,982		94	.45% Impe	rvious Area	
	Tc	Length	Slop		Velocity	Capacity	Description
_	(min)	(feet)	(ft/	'ft)	(ft/sec)	(cfs)	
	1.0	75	0.02	00	1.26		Sheet Flow, A to B
							Smooth surfaces n= 0.011 P2= 3.00"
	1.9	312	0.03	00	2.79		Shallow Concentrated Flow, B to C
							Unpaved Kv= 16.1 fps
	2.9	387	Tota	ıl, İr	ncreased	to minimum	Tc = 6.0 min

Summary for Subcatchment S30C: Subcatchment 30C

Runoff = 64.56 cfs @ 12.12 hrs, Volume= 5.638 af, Depth= 6.77"

	Area (sf)	CN	Description
*	360,000	98	
	24,598	39	>75% Grass cover, Good, HSG A
	34,063	74	>75% Grass cover, Good, HSG C
*	16,903	98	Water Surface
	435,564	93	Weighted Average
	58,661		13.47% Pervious Area
	376,903		86.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.0	75	0.0200	1.26	•	Sheet Flow, A to B
					Smooth surfaces n= 0.011 P2= 3.00"
2.2	300	0.0200	2.28		Shallow Concentrated Flow, B to C
					Unpaved Kv= 16.1 fps
1.8	200	0.0150	1.84		Shallow Concentrated Flow, C to D
					Grassed Waterway Kv= 15.0 fps
0.6	30	0.0100	0.79		Sheet Flow, D to E
					Smooth surfaces n= 0.011 P2= 3.00"
3.5	46	0.0600	0.22		Sheet Flow, E to F
					Grass: Short n= 0.150 P2= 3.00"
9.1	651	Total			

Summary for Subcatchment S32: Subcatchment 32

Runoff = 9.12 cfs @ 12.43 hrs, Volume= 1.170 af, Depth= 2.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

	Area (s	sf)	CN	Description	า					
*	19,70)6	98							
	30,61	3	74	>75% Gras	s cover, Goo	d, HSG C				
	59,97		39	>75% Gras	s cover, Goo	d, HSG A				
	46,84		80	>75% Gras	s cover, Goo	d, HSG D				
	46,89		30	Woods, Go	ood, HSG A					
	4,05			Woods, Go	ood, HSG C					
	8,27	7 4	77	<u>Woods, Go</u>	ood, HSG D					
	216,36	55	58	Weighted	Average					
	196,65	59		90.89% Per	vious Area					
	19,70)6		9.11% Imp	ervious Area					
			0.1							
	Tc Len	_	Slop		y Capacity	Description				
(m	in) (te	et)	(ft/f	t) (ft/sec	(cfs)					
16	5.9	75	0.008	0.0	7	Sheet Flow, A to B				
						Grass: Dense n= 0.240 P2= 3.00"				
12	2.1	360	0.005	0 0.4	9	Shallow Concentrated Flow, B to C				
						Short Grass Pasture Kv= 7.0 fps				
29	2.0	435	Total							

Summary for Subcatchment \$40: Subcatchment 40

Runoff = 92.82 cfs @ 12.29 hrs, Volume= 10.116 af, Depth= 4.80"

_	Ar	ea (sf)	CN	De	scription				
*	1	58,513	98						
		31,707	55	Wo	ods, God	od, HSG B			
	2	17,840	70	Wo	ods, God	od, HSG C			
	1	89,606	77	Wc	ods, God	d, HSG D			
		950	30	Wo	ods, God	od, HSG A			
		26,357	39	>7	5% Grass (cover, Goo	d, HSG A		
		97,705	61	>7	5% Grass (cover, Goo	d, HSG B		
		90,000		>7	5% Grass (cover, Goo	d, HSG C		
_	1	90,000	80	>7	5% Grass o	cover, Goo	d, HSG D		
		02,678	76	We	eighted A	verage			
	9	44,165		85.	85.62% Pervious Area				
	1	58,513		14.	.38% Impe	rvious Area			
	Tc	Length	Slop		•		Description		
_	(min)	(feet)	(ft/f		(ft/sec)	(cfs)			
	1.1	75	0.015	50	1.12		Sheet Flow, A to B		
							Smooth surfaces n= 0.011 P2= 3.00"		
	4.4	550	0.016	64	2.06		Shallow Concentrated Flow, B to C		
							Unpaved Kv= 16.1 fps		
	1.6	325	0.050	00	3.35		Shallow Concentrated Flow, C to D		
							Grassed Waterway Kv= 15.0 fps		
	14.3	2,100	0.003	30	2.45	73.44			
							Area= 30.0 sf Perim= 35.0' r= 0.86'		
_							n= 0.030 Earth, grassed & winding		
	21.4	3,050	Total						

Summary for Subcatchment S40A: Subcatchment 40A

Runoff = 44.76 cfs @ 12.18 hrs, Volume= 4.535 af, Depth= 7.00"

	Area (sf)	CN	Description
*	275,000	98	
	4,986	39	>75% Grass cover, Good, HSG A
	43,462	80	>75% Grass cover, Good, HSG D
	3,315	74	>75% Grass cover, Good, HSG C
*	11,757	98	Water Surface
	338,520	95	Weighted Average
	51,763		15.29% Pervious Area
	286,757		84.71% Impervious Area

Printed 2/2/2018 Page 90

	Tc in)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	1.1	75	0.0150	1.12	, ,	Sheet Flow, A to B
						Smooth surfaces n= 0.011 P2= 3.00"
	1.7	204	0.0150	1.97		Shallow Concentrated Flow, B to C
						Unpaved Kv= 16.1 fps
7	7.0	629	0.0100	1.50		Shallow Concentrated Flow, C to D
						Grassed Waterway Kv= 15.0 fps
(0.2	148	0.0400	14.40	45.24	Pipe Channel, D to E
						24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
						n= 0.013 Corrugated PE, smooth interior
	1.9	37	0.0010	0.33		Sheet Flow, E to F
						Smooth surfaces n= 0.011 P2= 3.00"
•	1.8	32	0.1600	0.30		Sheet Flow, F to G
						Grass: Short n= 0.150 P2= 3.00"
13	3.7	1,125	Total			

Summary for Subcatchment S41: Subcatchment 41

Runoff = 40.75 cfs @ 12.10 hrs, Volume= 3.364 af, Depth= 6.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

	Α	rea (sf)	CN	De	escription				
*		187,081	98						
		22,827	79	50-	-75% Gras	s cover, Fair	r, HSG C		
		37,988	84	50-	-75% Gras	s cover, Fair	r, HSG D		
*	:	7,519	98	Wo	ater Surfac	ce			
	2	255,415	94	We	eighted A	verage			
	60,815			23.	.81% Pervi	ous Area			
	194,600			76.19% Impervious Area					
	Tc		-		•		Description		
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)			
	1.3	75	0.010	OC	0.95		Sheet Flow, A to B		
							Smooth surfaces n= 0.011 P2= 3.00"		
	3.3	383	0.014	43	1.93		Shallow Concentrated Flow, B to C		
							Unpaved Kv= 16.1 fps		
	2.5	243	0.053	35	1.62		Shallow Concentrated Flow, C to D		
_							Short Grass Pasture Kv= 7.0 fps		
	7.1	701	Tota						

Summary for Subcatchment \$42: Subcatchment 42

Runoff = 48.95 cfs @ 12.15 hrs, Volume= 4.407 af, Depth= 6.53"

	Ar	ea (sf)	CN	De	scription					
*	2	78,726	98							
*		11,139	98	Wo	ater Surfac	ce				
		26,977	49	50-	-75% Gras	s cover, Fair	r, HSG A			
		29,325	69	50-	-75% Gras	s cover, Fair	r, HSG B			
_		6,630	79	50-	-75% Gras	s cover, Fair	r, HSG C			
	3	52,797	91	We	eighted A	verage				
	62,932			17.	17.84% Pervious Area					
	2	89,865		82.	.16% Impe	rvious Area				
	Tc	Longth	Slor	20	Volocity	Capacity	Description			
	(min)	Length (feet)	Slop (ft/		(ft/sec)	(cfs)	Description			
_						(C13)	Charleton, Ala D			
	1.3	75	0.010	JU	0.95		Sheet Flow, A to B			
	/ 1	700	0.014	40	1.00		Smooth surfaces n= 0.011 P2= 3.00"			
	6.1	700	0.014	40	1.90		Shallow Concentrated Flow, B to C			
	3.4	225	0.02	50	1.11		Unpaved Kv= 16.1 fps Shallow Concentrated Flow, C to D			
	3.4	223	0.02	50	1,11		Short Grass Pasture Kv= 7.0 fps			
_							311011 01033 1 031010 INV - 7.0 1p3			
	10.8	1,000	Tota	l						

Summary for Subcatchment \$43: Subcatchment 43

Runoff = 39.37 cfs @ 12.37 hrs, Volume= 4.720 af, Depth= 4.57"

	Area	(sf)	CN	De	scription					
*	232,	052	98							
*	36,	376	98	Wo	ater Surfac	ce				
	122,	740	49	50-	-75% Gras	s cover, Fair	r, HSG A			
	38,	160	79	50-	-75% Gras	s cover, Fair	r, HSG C			
	8,	153	69	50-	-75% Gras	Grass cover, Fair, HSG B				
	86,	000	36	Wo	oods, Fair,	HSG A				
	5,	923	60	Wo	oods, Fair,	HSG B				
	10,	549	77	Wo	ods, Poo	r, HSG C				
	539,	953	74	We	eighted A	verage				
	271,525			50.29% Pervious Area						
	268,	428	49.71% Impervious Area			rvious Area				
	Tc Le	ength	Slop	ре	Velocity	Capacity	Description			
(m	nin) (feet)	(ft/	ft)	(ft/sec)	(cfs)				
1	0.6	75	0.010	00	0.12		Sheet Flow, A to B			
							Grass: Short n= 0.150 P2= 3.00"			
1	6.3	488	0.010	00	0.50		Shallow Concentrated Flow, B to C			
							Woodland Kv= 5.0 fps			
2	6.9	563	Tota	l						

Printed 2/2/2018

Page 92

Summary for Subcatchment S50: Subcatchment 50

Runoff = 5.33 cfs @ 12.09 hrs, Volume= 0.391 af, Depth= 5.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

	Area (sf)	CN	Description						
*	10,546	98	98						
	4,431	61	>75% Grass cover, Good, HSG B						
	24,859	74	>75% Grass cover, Good, HSG C						
	39,836	79	Weighted Average						
	29,290		73.53% Pervious Area						
	10,546		26.47% Impervious Area						
	Tc Length	Slo	pe Velocity Capacity Description						
(m	nin) (feet)	(ft,	/ft) (ft/sec) (cfs)						
	6.0		Direct Entry,						

Summary for Subcatchment S60: Subcatchment 60

Runoff = 21.82 cfs @ 12.74 hrs, Volume= 4.072 af, Depth= 1.62"

	Aı	ea (sf)	CN	De	escription						
	8	41,049	30	Woods, Good, HSG A							
	3	44,078	77	W	Woods, Good, HSG D						
		22,871	70	W	oods, God	d, HSG C					
	37,335 39 >75% Grass cover, Good, HSG A										
		32,830	80	>7	5% Grass of	cover, Good	d, HSG D				
*		17,570	98								
_		15,415	77	W	oods, God	d, HSG D					
1,311,148 46 Weighted Average											
	1,2	93,578		98	.66% Pervi	ous Area					
	17,570			1.3	34% Imper	vious Area					
	Tc	Length	Slo				Description				
_	(min)	(feet)	(f†/		(ft/sec)	(cfs)					
	30.7	75	0.02	200	0.04		Sheet Flow, A to B				
							Woods: Dense underbrush n= 0.800 P2= 3.00"				
	14.4	530	0.01	50	0.61		Shallow Concentrated Flow, B to C				
							Woodland Kv= 5.0 fps				
	1.4	430	0.01	00	5.10	122.30	Channel Flow, C to D				
							Area= 24.0 sf Perim= 23.0' r= 1.04'				
_							n= 0.030 Earth, grassed & winding				
	46.5	1,035	Tota	lr							

Printed 2/2/2018

Page 93

Summary for Subcatchment S60A: Subcatchment 60A

Runoff = 2.46 cfs @ 13.90 hrs, Volume= 1.112 af, Depth= 0.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

Are	ea (sf)	CN	Des	scription						
73	35,140	30	Woods, Good, HSG A							
7	71,251	70	Woods, Good, HSG C							
	22,354	77	Woods, Good, HSG D							
	26,632	39	>75	>75% Grass cover, Good, HSG A						
	9,029	74	>75	% Grass o	cover, Good	d, HSG C				
86	54,406	35	We	ighted A	verage					
86	54,406		100.00% Pervious Area							
Tc	Length	Slop	oe '	Velocity	Capacity	Description				
(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)					
30.7	75	0.02	00	0.04		Sheet Flow, A to B				
						Woods: Dense underbrush n= 0.800 P2= 3.00"				
71.5	1,175	0.00	30	0.27		Shallow Concentrated Flow, B to C				
						Woodland Kv= 5.0 fps				
102.2	1,250	Tota	1							

Summary for Subcatchment S61: Subcatchment 61

Runoff = 23.86 cfs @ 12.09 hrs, Volume= 1.881 af, Depth= 6.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

	Ar	ea (sf)	CN	Description					
		1,448	86	<50% Grass	cover, Poor,	HSG C			
		18,168	49	50-75% Gro	ss cover, Fair	r, HSG A			
*		8,570	98	Water Surfc	ice				
*	1	19,736	98						
	1	47,922	92	Weighted A	Average				
		19,616		13.26% Per	vious Area				
	1	28,306		86.74% Imp	.74% Impervious Area				
	Tc	Length	Slop	e Velocity	Capacity	Description			
_	(min)	(feet)	(ft/	t) (ft/sec	(cfs)				
	1.0	75	0.020	00 1.26)	Sheet Flow, A to B			
						Smooth surfaces n= 0.011 P2= 3.00"			
	3.7	500	0.020	00 2.28	}	Shallow Concentrated Flow, B to C			
						Unpaved Kv= 16.1 fps			
	1.2	100	0.008	30 1.34		Shallow Concentrated Flow, C to D			
						Grassed Waterway Kv= 15.0 fps			
_	5.9	475	Tota	Increased	l to minimum	$\Delta T_{\rm C} = 6.0 \text{min}$			

5.9 675 Total, Increased to minimum Tc = 6.0 min

Printed 2/2/2018 Page 94

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Summary for Subcatchment S62: Subcatchment 62

Runoff = 45.64 cfs @ 12.09 hrs, Volume= 3.748 af, Depth= 7.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

	Ar	ea (sf)	CN	De	scription						
*		11,953	98	W	ater Surfac	ce					
*	2	36,076	98								
		20,275	84	50	-75% Gras	s cover, Fair	r, HSG D				
		6,756	49	50	-75% Gras	s cover, Fair	r, HSG A				
	275,060		96	We	Weighted Average						
	27,031			9.8	3% Pervio	us Area					
	248,029			90.17% Impervious Area							
	_										
	Tc	Length	Slop		•	Capacity	Description				
_	(min)	(feet)	(ft/	ft)	(ft/sec)	(cfs)					
	1.1	75	0.01	50	1.12		Sheet Flow, A to B				
							Smooth surfaces n= 0.011 P2= 3.00"				
	1.3	150	0.01	50	1.97		Shallow Concentrated Flow, B to C				
							Unpaved Kv= 16.1 fps				
	2.5	600	0.030	00	4.05	24.32	Channel Flow, C to D				
							Area= 6.0 sf Perim= 12.0' r= 0.50'				
_							n= 0.040 Earth, cobble bottom, clean sides				
	10	825	Tota	l Ir	acreased	to minimum	$T_{C} = 60 \text{ min}$				

^{.9 825} Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment S63: Subcatchment 63

Runoff = 2.83 cfs @ 12.09 hrs, Volume= 0.212 af, Depth= 5.83"

_	Area (sf)	CN	Description			
*	15,000	98				
	4,049	39	>75% Grass of	cover, Good	d, HSG A	
	19,049	85	Weighted Av	/erage		
	4,049		21.26% Pervi	ous Area		
	15,000		78.74% Impe	rvious Area		
	Tc Length		·	Capacity	Description	
_	(min) (feet)	(††,	/ft) (ft/sec)	(cfs)		
	6.0				Direct Entry,	

<u>Page 95</u>

Summary for Subcatchment S64: Subcatchment 64

Runoff = 2.15 cfs @ 12.09 hrs, Volume= 0.163 af, Depth= 6.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

	Aı	rea (sf)	CN	Description					
-	*	11,520	98						
		2,572	39	>75% Grass	cover, Good	d, HSG A			
		14,092	87	Weighted A	verage				
		2,572	2,572 18.25% Pervious Area						
		11,520		81.75% Impe	ervious Area				
	-		0.1						
	Tc	Length		be Velocity	. ,	Description			
	(min)	(feet)	(ft/	ft) (ft/sec)	(cfs)				
	6.0					Direct Entry			

Summary for Subcatchment S65: Subcatchment 65

Runoff = 4.21 cfs @ 12.09 hrs, Volume= 0.306 af, Depth= 3.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

	Are	ea (sf)	CN	De	escription					
*	2	20,240	98	98						
	4	20,692	39	>7	5% Grass of	cover, Good	od, HSG A			
		40,932 68 Weighted Average								
	4	20,692		50	.55% Pervi	ous Area				
	2	20,240		49	.45% Impe	rvious Area	a			
1		Length			,	. ,	•			
<u>(n</u>	nin)	(feet)	(ft/	TT)	(ft/sec)	(cfs)				
	6.0						Direct Entry,			

Summary for Subcatchment S66: Subcatchment 66

Runoff = 1.55 cfs @ 12.10 hrs, Volume= 0.115 af, Depth= 3.04"

Area (sf)	CN	Description
9,622	39	>75% Grass cover, Good, HSG A
 10,094	80	>75% Grass cover, Good, HSG D
19,716	60	Weighted Average
19,716		100.00% Pervious Area

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 96

•	6.0	, ,			, ,	Direct Entry,	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	Tc	Length	Slope	Velocity	Capacity	Description	

Summary for Subcatchment S67: Subcatchment 67

Runoff = 1.09 cfs @ 12.10 hrs, Volume= 0.082 af, Depth= 2.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Yr Rainfall=7.60"

A	rea (sf)	CN	Des	Description			
	9,767	39	>75	75% Grass cover, Good, HSG A			
	6,617	80	>75	75% Grass cover, Good, HSG D			
	16,384	56 Weighted Average					
	16,384		100	100.00% Pervious Area			
_							
Tc	Length	Slop	ре	Velocity	Capacity	Description	
(min)	(feet)	(ft/	/ft)	(ft/sec)	(cfs)		
6.0						Direct Entry,	

Summary for Reach 8R: Forested Buffer #4

Inflow Area = 0.453 ac, 0.00% Impervious, Inflow Depth = 2.92" for 100-Yr event

Inflow = 1.52 cfs @ 12.10 hrs, Volume= 0.110 af

Outflow = 0.76 cfs @ 12.67 hrs, Volume= 0.110 af, Atten= 50%, Lag= 34.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.11 fps, Min. Travel Time= 22.2 min Avg. Velocity = 0.02 fps, Avg. Travel Time= 164.4 min

Peak Storage= 1,011 cf @ 12.30 hrs Average Depth at Peak Storage= 0.27'

Bank-Full Depth= 0.10' Flow Area= 2.5 sf, Capacity= 0.20 cfs

25.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 150.0' Slope= 0.0400 '/'

Inlet Invert= 249.00', Outlet Invert= 243.00'

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 97

Summary for Reach 9R: Forested Buffer #5

Inflow Area = 0.376 ac, 0.00% Impervious, Inflow Depth = 2.48" for 100-Yr event

Inflow = 1.28 cfs @ 12.10 hrs, Volume= 0.078 af

Outflow = 0.51 cfs @ 12.71 hrs, Volume= 0.078 af, Atten= 60%, Lag= 36.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.11 fps, Min. Travel Time= 22.4 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 171.5 min

Peak Storage= 684 cf @ 12.33 hrs Average Depth at Peak Storage= 0.18' Bank-Full Depth= 0.10' Flow Area= 2.5 sf, Capacity= 0.21 cfs

25.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 150.0' Slope= 0.0467 '/'

Inlet Invert= 249.00', Outlet Invert= 242.00'

Summary for Reach 34R: Wetland

Inflow Area = 3.396 ac, 86.74% Impervious, Inflow Depth > 6.48" for 100-Yr event

Inflow = 14.40 cfs @ 12.20 hrs, Volume= 1.832 af

Outflow = 2.50 cfs @ 15.95 hrs, Volume= 1.742 af, Atten= 83%, Lag= 224.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.08 fps, Min. Travel Time= 163.4 min Avg. Velocity = 0.04 fps, Avg. Travel Time= 378.4 min

Peak Storage= 24,476 cf @ 13.23 hrs Average Depth at Peak Storage= 0.31'

Bank-Full Depth= 0.10' Flow Area= 10.0 sf, Capacity= 0.57 cfs

100.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush

Length= 800.0' Slope= 0.0050 '/'

Inlet Invert= 245.00', Outlet Invert= 241.00'

Prepared by Stantec Consulting Services, Inc. Hydro CAD® 10.00-20, s/n 00734, © 2017 Hydro CAD Soft

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

<u>Page 98</u>

Summary for Reach 206R: Forested Buffer #3

Inflow Area = 0.940 ac, 49.45% Impervious, Inflow Depth = 3.90" for 100-Yr event

Inflow = 4.22 cfs @ 12.09 hrs, Volume= 0.306 af

Outflow = 2.12 cfs @ 12.63 hrs, Volume= 0.306 af, Atten= 50%, Lag= 32.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.07 fps, Min. Travel Time= 21.8 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 142.6 min

Peak Storage= 2,781 cf @ 12.27 hrs Average Depth at Peak Storage= 0.50' Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.28 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 86.0' Slope= 0.0116 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Summary for Reach 207R: Forested Buffer #2

Inflow Area = 0.324 ac, 81.75% Impervious, Inflow Depth = 6.06" for 100-Yr event

Inflow = 2.15 cfs @ 12.09 hrs, Volume= 0.163 af

Outflow = 0.94 cfs @ 12.85 hrs, Volume= 0.163 af, Atten= 56%, Lag= 45.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.05 fps, Min. Travel Time= 33.6 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 224.7 min

Peak Storage= 1,902 cf @ 12.28 hrs Average Depth at Peak Storage= 0.27'

Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.25 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 109.0' Slope= 0.0092 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 99

Summary for Reach 208R: Forested Buffer #1

Inflow Area = 0.437 ac, 78.74% Impervious, Inflow Depth = 5.83" for 100-Yr event

Inflow = 2.83 cfs @ 12.09 hrs, Volume= 0.212 af

Outflow = 1.06 cfs @ 13.08 hrs, Volume= 0.212 af, Atten= 63%, Lag= 59.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.05 fps, Min. Travel Time= 44.5 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 264.9 min

Peak Storage= 2,823 cf @ 12.34 hrs Average Depth at Peak Storage= 0.32' Bank-Full Depth= 0.10' Flow Area= 6.5 sf, Capacity= 0.22 cfs

65.00' x 0.10' deep channel, n= 0.800 Sheet flow: Woods+dense brush

Length= 134.0' Slope= 0.0075 '/'

Inlet Invert= 242.00', Outlet Invert= 241.00'

Summary for Reach R1: Portion of Davis Brook

Inflow Area = 26.358 ac, 65.57% Impervious, Inflow Depth > 2.60" for 100-Yr event

Inflow = 38.67 cfs @ 12.13 hrs, Volume= 5.705 af

Outflow = 28.38 cfs @ 12.84 hrs, Volume= 5.698 af, Atten= 27%, Lag= 42.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.84 fps, Min. Travel Time= 14.5 min Avg. Velocity = 0.39 fps, Avg. Travel Time= 67.9 min

Peak Storage= 24,738 cf @ 12.59 hrs Average Depth at Peak Storage= 1.33'

Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 176.49 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 1,600.0' Slope= 0.0031 '/'

Inlet Invert= 221.00', Outlet Invert= 216.00'

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20, s/n 00734 © 2017 HydroCAD Soft

<u>HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC</u>

<u>Page 100</u>

Summary for Reach R2: Portion of Davis Brook

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 1.81" for 100-Yr event

Inflow = 32.29 cfs @ 12.33 hrs, Volume= 3.097 af

Outflow = 23.02 cfs @ 12.51 hrs, Volume= 3.095 af, Atten= 29%, Lag= 10.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.78 fps, Min. Travel Time= 5.6 min Avg. Velocity = 0.36 fps, Avg. Travel Time= 28.1 min

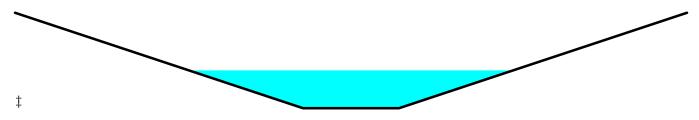
Peak Storage= 7,805 cf @ 12.41 hrs Average Depth at Peak Storage= 1.19' Bank-Full Depth= 3.00' Flow Area= 60.0 sf, Capacity= 182.28 cfs

5.00' x 3.00' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 35.00'

Length= 600.0' Slope= 0.0033 '/'

Inlet Invert= 223.00', Outlet Invert= 221.00'



Summary for Reach R3: Natural Ditch

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth > 1.81" for 100-Yr event

Inflow = 50.70 cfs @ 12.30 hrs, Volume= 3.097 af

Outflow = 32.29 cfs @ 12.33 hrs, Volume= 3.097 af, Atten= 36%, Lag= 2.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.27 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.07 fps, Avg. Travel Time= 4.7 min

Peak Storage= 2,234 cf @ 12.31 hrs

Average Depth at Peak Storage= 0.58'

Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 998.06 cfs

10.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 5.0 '/' Top Width= 40.00'

Length= 300.0' Slope= 0.0433 '/'

Inlet Invert= 236.00', Outlet Invert= 223.00'



Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 101

Summary for Reach R4: Existing Ditch

12.396 ac, 49.71% Impervious, Inflow Depth = 0.35" for 100-Yr event Inflow Area =

Inflow 2.10 cfs @ 19.85 hrs. Volume= 0.358 af

1.14 cfs @ 20.00 hrs, Volume= Outflow 0.358 af, Atten= 46%, Lag= 9.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.15 fps, Min. Travel Time= 2.8 min Avg. Velocity = 0.88 fps, Avg. Travel Time= 3.6 min

Peak Storage= 189 cf @ 19.95 hrs Average Depth at Peak Storage= 0.12'

Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 168.09 cfs

8.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 20.00'

Length= 190.0' Slope= 0.0053 '/'

Inlet Invert= 244.00', Outlet Invert= 243.00'



Summary for Reach R5: Natural Ditch

Inflow Area = 5.864 ac, 76.19% Impervious, Inflow Depth > 5.34" for 100-Yr event

Inflow 44.72 cfs @ 12.10 hrs, Volume= 2.610 af

Outflow 38.66 cfs @ 12.13 hrs, Volume= 2.610 af, Atten= 14%, Lag= 2.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.66 fps, Min. Travel Time= 1.1 min

Avg. Velocity = 0.79 fps, Avg. Travel Time= 5.3 min

Peak Storage= 2,741 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.38'

Bank-Full Depth= 3.00' Flow Area= 165.0 sf, Capacity= 1,945.52 cfs

25.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds

Side Slope Z-value= 10.0 '/' Top Width= 85.00'

Length= 250.0' Slope= 0.0320 '/'

Inlet Invert= 229.00', Outlet Invert= 221.00'



Printed 2/2/2018

Page 102

Summary for Pond 30A: Grassed Underdrained Soil Filter A

Inflow Area = 3.277 ac, 81.25% Impervious, Inflow Depth = 6.88" for 100-Yr event

Inflow 23.40 cfs @ 12.09 hrs, Volume= 1.880 af

23.40 cfs @ 12.09 hrs, Volume= 1.880 af, Atten= 0%, Lag= 0.0 min Primary

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 30B: Grassed Underdrained Soil Filter B

Inflow Area = 4.812 ac, 94.45% Impervious, Inflow Depth = 7.00" for 100-Yr event

34.59 cfs @ 12.09 hrs. Volume= 2.808 af Inflow

34.59 cfs @ 12.09 hrs, Volume= Primary 2.808 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 101: Pond #1 (plus wetland storage)

Inflow Area = 38.340 ac, 65.45% Impervious, Inflow Depth > 5.28" for 100-Yr event

Inflow 70.90 cfs @ 12.47 hrs, Volume= 16.870 af

Outflow 12.33 cfs @ 15.04 hrs, Volume= 14.614 af, Atten= 83%, Lag= 153.9 min =

Primary 12.33 cfs @ 15.04 hrs, Volume= 14.614 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 242.67' @ 15.04 hrs Surf.Area= 282,742 sf Storage= 406,699 cf

Plug-Flow detention time= 650.4 min calculated for 14.604 af (87% of inflow)

Center-of-Mass det. time=473.0 min (1,459.7 - 986.7)

Volume	Invert	Avail.	Storage	Storag	ge Description	
#1	240.00'	83	35,458 cf	Custo	m Stage Data (I	Prismatic) Listed below (Recalc)
Elevation (feet)	Surf./	Area sq-ft)	Inc. (cubic-	Store feet)	Cum.Store (cubic-feet)	
240.00	22	2,686		0	0	
241.00	115	5,923	6	9,305	69,305	
242.00	221	,289	16	8,606	237,911	
243.00	313	3,044	26	7,167	505,077	
244.00	347	7,717	33	0,381	835,458	

Device	Routing	Invert	Outlet Devices
#1	Primary	237.64'	24.0" Round Culvert
			L= 87.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 237.64' / 237.30' S= 0.0039 '/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=3.14 sf
#2	Device 1	241 00'	24.0" Vert. Orifice/Grate. C= 0.600

Primary OutFlow Max=12.33 cfs @ 15.04 hrs HW=242.67' (Free Discharge)

-1=Culvert (Passes 12.33 cfs of 19.04 cfs potential flow)

12-Orifice/Grate (Orifice Controls 12.33 cfs @ 4.40 fps)

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 103

Summary for Pond 102: Pond #2

Inflow Area = 12.396 ac, 49.71% Impervious, Inflow Depth = 4.57" for 100-Yr event

Inflow = 39.37 cfs @ 12.37 hrs, Volume= 4.720 af

Outflow = 2.10 cfs @ 19.85 hrs, Volume= 0.358 af, Atten= 95%, Lag= 448.7 min

Primary = 2.10 cfs @ 19.85 hrs, Volume= 0.358 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 248.65' @ 19.85 hrs Surf.Area= 127,000 sf Storage= 190,120 cf

Plug-Flow detention time= 685.0 min calculated for 0.357 af (8% of inflow)

Center-of-Mass det. time=478.8 min (1,316.6 - 837.8)

Volume	Invert	Avail.Storage	Storage Description
#1	242.00'	190,120 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.00	25,000	0	0
244.68	36,400	82,276	82,276
246.00	127,000	107,844	190,120

Device	Routing	Invert	Outlet Devices
#1	Primary	246.53'	18.0" Round Culvert
			L= 165.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 246.53' / 244.00' S= 0.0153'/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=1.77 sf
#2	Device 1	248 05'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=2.10 cfs @ 19.85 hrs HW=248.65' (Free Discharge)

1=Culvert (Passes 2.10 cfs of 6.97 cfs potential flow)
2=Orifice/Grate (Orifice Controls 2.10 cfs @ 2.64 fps)

Summary for Pond 103: Pond #3

Inflow Area = 20.495 ac, 62.54% Impervious, Inflow Depth = 2.79" for 100-Yr event

Inflow = 48.95 cfs @ 12.15 hrs, Volume= 4.765 af

Outflow = 50.70 cfs @ 12.30 hrs, Volume= 3.097 af, Atten= 0%, Lag= 9.2 min

Primary = 50.70 cfs @ 12.30 hrs, Volume= 3.097 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 261.29' @ 12.30 hrs Surf.Area= 28,630 sf Storage= 94,130 cf

Plug-Flow detention time= 312.6 min calculated for 3.097 af (65% of inflow)

Center-of-Mass det. time= 180.1 min (999.4 - 819.3)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1	234.00'	94,130 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 104

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
234.00	4,000	0	0
236.00	7,000	11,000	11,000
238.92	11,139	26,483	37,483
240.00	14,287	13,730	51,213
242.00	28,630	42,917	94,130

Device	Routing	Invert	Outlet Devices
#1	Primary	234.22'	24.0" Round Culvert
	•		L= 95.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 234.22' / 233.90' S= 0.0034 '/' Cc= 0.900
			n=0.025 Corrugated metal, Flow Area=3.14sf
#2	Device 1	241.29'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	236.00'	0.5" W x 0.5" H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=50.67 cfs @ 12.30 hrs HW=261.26' (Free Discharge)

1=Culvert (Barrel Controls 50.67 cfs @ 16.13 fps)

2=Orifice/Grate (Passes < 67.61 cfs potential flow)

-3=Orifice/Grate (Passes < 0.04 cfs potential flow)

Summary for Pond 104: Pond #4

Inflow Area = 5.864 ac, 76.19% Impervious, Inflow Depth = 6.88" for 100-Yr event

Inflow = 40.75 cfs @ 12.10 hrs, Volume= 3.364 af

Outflow = 44.72 cfs @ 12.10 hrs, Volume= 2.610 af, Atten= 0%, Lag= 0.0 min

Primary = 44.72 cfs @ 12.10 hrs, Volume= 2.610 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 250.30' @ 12.10 hrs Surf.Area= 16,786 sf Storage= 36,018 cf

Plug-Flow detention time= 203.4 min calculated for 2.610 af (78% of inflow)

Center-of-Mass det. time= 122.7 min (886.3 - 763.6)

#2

Device 1

<u>Volume</u>	Inve	<u>rt Avail.Stor</u>	age Storaç	ge Description	
#1	230.00	0' 36,01	8 cf Custo	m Stage Data (Prismatic) Listed be	elow (Recalc)
Elevation (feet		urf.Area (sq-ft) (c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	
230.00	0	620	0	0	
232.0	0	3,391	4,011	4,011	
234.0	5	7,519	11,183	15,194	
235.0	0	9,087	7,888	23,082	
236.0	0	16,786	12,937	36,018	
Device	Routing	Invert	Outlet De	rices	
#1	Primary	230.62'	24.0" Rou		
			Inlet / Out	MP, projecting, no headwall, Keed Invert= 230.62' / 228.40' S= 0.0 Corrugated metal, Flow Area= 3.1	231 '/' Cc= 0.900

237.61' **24.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Prepared by Stantec Consulting Services, Inc. HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018

<u>Page 105</u>

#3 Device 1 231.00' **0.5" W x 0.5" H Vert. Orifice/Grate** C= 0.600

Primary OutFlow Max=44.71 cfs @ 12.10 hrs HW=250.29' (Free Discharge)

-1=Culvert (Barrel Controls 44.71 cfs @ 14.23 fps)

2=Orifice/Grate (Passes < 53.86 cfs potential flow)

-3=Orifice/Grate (Passes < 0.04 cfs potential flow)

Summary for Pond 201: Pond #201

Inflow Area = 3.396 ac, 86.74% Impervious, Inflow Depth = 6.65" for 100-Yr event

Inflow = 23.86 cfs @ 12.09 hrs, Volume= 1.881 af

Outflow = 14.40 cfs @ 12.20 hrs, Volume= 1.832 af, Atten= 40%, Lag= 7.1 min

Primary = 14.40 cfs @ 12.20 hrs, Volume= 1.832 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 250.30' @ 12.20 hrs Surf.Area= 17,949 sf Storage= 34,270 cf

Plug-Flow detention time= 618.8 min calculated for 1.832 af (97% of inflow)

Center-of-Mass det. time=602.8 min (1,373.3 - 770.5)

Volume	Invert	Avail.Storage	Storage Description
#1	248.00'	47,142 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf. <i>A</i> (s	Area Inc :q-ft) (cubic	.Store Cum.Store -feet) (cubic-feet)

C0111.3101C	1110.31010	3011./AICG	Licvanon
(cubic-feet)	(cubic-feet)	(sq-ft)	(feet)
0	0	8,570	248.00
12,225	12,225	15,880	249.00
28,998	16,773	17,665	250.00
47,142	18,145	18,624	251.00

Device	Routing	Invert	Outlet Devices
#1	Primary	250.00'	20.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#2	Primary	245.40'	12.0" Round Culvert L= 29.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 245.40' / 245.00' S= 0.0138'/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	249.65'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	245.50'	1.6" Vert. Orifice/Grate C= 0.600
#5	Device 4	248.00'	2.000 in/hr Exfiltration over Surface area above 248.00'
			Excluded Surface area = 8,570 sf

Primary OutFlow Max=14.32 cfs @ 12.20 hrs HW=250.29' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Weir Controls 8.06 cfs @ 1.37 fps)

-2=Culvert (Inlet Controls 6.26 cfs @ 7.97 fps)

3=Orifice/Grate (Passes < 10.63 cfs potential flow)

4=Orifice/Grate (Passes < 0.15 cfs potential flow)

5=Exfiltration (Passes < 0.43 cfs potential flow)

Printed 2/2/2018

<u>Page 106</u>

Summary for Pond 202: Pond #202

Inflow Area = 6.315 ac, 90.17% Impervious, Inflow Depth = 7.12" for 100-Yr event

Inflow = 45.64 cfs @ 12.09 hrs, Volume= 3.748 af

Outflow = 25.50 cfs @ 12.22 hrs, Volume= 3.659 af, Atten= 44%, Lag= 7.8 min

Primary = 25.50 cfs @ 12.22 hrs, Volume= 3.659 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 244.44' @ 12.22 hrs Surf.Area= 32,263 sf Storage= 73,122 cf

Plug-Flow detention time= 601.4 min calculated for 3.659 af (98% of inflow)

Center-of-Mass det. time=586.3 min (1,339.6 - 753.3)

Volume	Invert	Avail.Storage	Storage Description
#1	241.00'	92,645 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
241.00	12,606	0	0
242.00	18,880	15,743	15,743
243.00	20,722	19,801	35,544
244.00	27,811	24,267	59,811
245.00	37,857	32,834	92,645

Device	Routing	Invert	Outlet Devices
#1	Primary	244.00'	25.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#2	Primary	238.40'	12.0" Round Culvert
			L= 115.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 238.40' / 237.75' S= 0.0057'/' Cc= 0.900
			n=0.013 Corrugated PE, smooth interior, Flow Area=0.79 sf
#3	Device 2	243.50'	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	238.50'	2.4" Vert. Orifice/Grate C= 0.600
#5	Device 4	241.00'	2.000 in/hr Exfiltration over Surface area above 241.00'
			Excluded Surface area = 12,606 sf

Primary OutFlow Max=25.15 cfs @ 12.22 hrs HW=244.44' (Free Discharge)

1=Broad-Crested Rectangular Weir (Weir Controls 18.75 cfs @ 1.71 fps)

-2=Culvert (Barrel Controls 6.40 cfs @ 8.15 fps)

3=Orifice/Grate (Passes < 14.65 cfs potential flow)

-4=Orifice/Grate (Passes < 0.37 cfs potential flow)

5=Exfiltration (Passes < 0.91 cfs potential flow)

Summary for Pond 203: Pond #203

Inflow Area = 9.999 ac, 86.53% Impervious, Inflow Depth = 6.77" for 100-Yr event

Inflow = 64.56 cfs @ 12.12 hrs, Volume= 5.638 af

Outflow = 23.30 cfs @ 12.43 hrs, Volume= 4.660 af, Atten= 64%, Lag= 18.5 min

Primary = 23.30 cfs @ 12.43 hrs, Volume= 4.660 af

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 107

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 250.15' @ 12.43 hrs Surf.Area= 41,749 sf Storage= 132,209 cf

Plug-Flow detention time=710.1 min calculated for 4.660 af (83% of inflow) Center-of-Mass det. time=640.3 min (1,409.8 - 769.5)

Volume	Inver	Avail.Store	age Storag	ge Description
#1	246.00	177,12	3 cf Custo i	om Stage Data (Prismatic) Listed below (Recalc)
	_			
Elevation	n Su	rf.Area	Inc.Store	Cum.Store
(feet	.)	(sq-ft) (c	ubic-feet)	(cubic-feet)
246.0	0	25,338	0	0
247.0	0	28,403	26,871	26,871
248.0	0	31,540	29,972	56,842
249.0	0	34,748	33,144	89,986
250.0	0	38,027	36,388	126,374
251.0	0	63,471	50,749	177,123
Device	Routing	Invert	Outlet Dev	vices
#1	Primary	249.75'	25.0' long	x 8.0' breadth Broad-Crested Rectangular Weir
	•		Head (fee	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00 3.50	4.00 4.50 5.00 5.50
			Coef. (Eng	glish) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64
				2.66 2.66 2.68 2.70 2.74
#2	Primary	243.40'	12.0" Rour	nd Culvert L= 65.0' CPP, projecting, no headwall, Ke= 0.900
	•		Inlet / Outl	tlet Invert= 243.40' / 243.00' S= 0.0062'/' Cc= 0.900
			n= 0.013 C	Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	249.25'		z. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	243.50'		Orifice/Grate C= 0.600
#5	Device 4	246.00'		r Exfiltration over Surface area above 246.00'
_				

Primary OutFlow Max=23.19 cfs @ 12.43 hrs HW=250.14' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Weir Controls 15.73 cfs @ 1.59 fps)

-2=Culvert (Inlet Controls 7.46 cfs @ 9.50 fps)

-3=Orifice/Grate (Passes < 14.31 cfs potential flow)

4=Orifice/Grate (Passes < 0.49 cfs potential flow)

5=Exfiltration (Passes < 0.76 cfs potential flow)

Summary for Pond 204: Pond #204

Excluded Surface area = 25,338 sf

Inflow Area = 7.771 ac, 84.71% Impervious, Inflow Depth = 7.00" for 100-Yr event

Inflow 44.76 cfs @ 12.18 hrs, Volume= 4.535 af

Outflow = 27.72 cfs @ 12.37 hrs, Volume= 4.106 af, Atten= 38%, Lag= 11.5 min

27.72 cfs @ 12.37 hrs, Volume= 4.106 af Primary

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 239.46' @ 12.37 hrs Surf.Area= 25,511 sf Storage= 92,962 cf

Plug-Flow detention time=775.6 min calculated for 4.104 af (90% of inflow) Center-of-Mass det. time=729.7 min (1,495.0 - 765.3)

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 108

Volume	Invert	Avail.Storc	ige Storaç	ge Description
#1	235.00	106,900	3 cf Custo	om Stage Data (Prismatic) Listed below (Recalc)
El	- C	-E A	L Cl	C C1
Elevatio		rf.Area	Inc.Store	Cum.Store
(feet	.)	(sq-ft) (cu	ubic-feet)	(cubic-feet)
235.0	0	11,757	0	0
236.0	0	18,909	15,333	15,333
237.0	0	20,939	19,924	35,257
238.0	0	23,032	21,986	57,243
239.0	0	25,210	24,121	81,364
240.0	0	25,868	25,539	106,903
Device	Routing	Invert	Outlet Dev	vices
#1	Primary	239.00'	25.0' long	x 12.0' breadth Broad-Crested Rectangular Weir
	•		Head (fee	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
				glish) 2.57 2.62 2.70 2.67 2.66 2.67 2.66 2.64
#2	Primary	232.40'	, .	nd Culvert L= 93.0' CPP, projecting, no headwall, Ke= 0.900
	,			tlet Invert= 232.40' / 232.00' S= 0.0043 '/' Cc= 0.900
				Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	232.50'		Orifice/Grate C= 0.600
#4	Device 2	238.75'		z. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 3	235.00'		r Exfiltration over Surface area above 235.00'
0		_00.00	•	Surface area = 11,757 sf

Primary OutFlow Max=27.37 cfs@12.37 hrs HW=239.45' (Free Discharge)

-1=Broad-Crested Rectangular Weir (Weir Controls 20.08 cfs @ 1.78 fps)

-2=Culvert (Barrel Controls 7.29 cfs @ 9.28 fps)

-3=Orifice/Grate (Passes < 0.40 cfs potential flow)

5=Exfiltration (Passes < 0.64 cfs potential flow)

-4=Orifice/Grate (Passes < 12.09 cfs potential flow)

Summary for Pond 205: Level Lip Spreader #1

Inflow Area = 0.437 ac, 78.74% Impervious, Inflow Depth = 5.83" for 100-Yr event

Inflow = 2.83 cfs @ 12.09 hrs, Volume= 0.212 af

Outflow = 2.83 cfs @ 12.09 hrs, Volume= 0.212 af, Atten= 0%, Lag= 0.1 min

Primary = 2.83 cfs @ 12.09 hrs, Volume= 0.212 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.57' @ 12.09 hrs Surf.Area= 159 sf Storage= 10 cf

Plug-Flow detention time=0.1 min calculated for 0.212 af (100% of inflow)

Center-of-Mass det. time=0.1 min (792.3 - 792.2)

Volume	Invert	Avail.Storage	Storage Description
#1	242.50'	306 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

50465_postdev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Printed 2/2/2018 Page 109

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.50	139	0	0
243.00	292	108	108
243.50	500	198	306

Device Routing Invert Outlet Devices

#1 Primary 242.50' 65.0' long x 2.0' breadth Broad-Crested Rectangular Weir

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=2.76 cfs @ 12.09 hrs HW=242.57' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 2.76 cfs @ 0.65 fps)

Summary for Pond 206: Level Lip Spreader #2

Inflow Area = 0.324 ac, 81.75% Impervious, Inflow Depth = 6.06" for 100-Yr event

Inflow = 2.15 cfs @ 12.09 hrs, Volume= 0.163 af

Outflow = 2.15 cfs @ 12.09 hrs, Volume= 0.163 af, Atten= 0%, Lag= 0.1 min

Primary = 2.15 cfs @ 12.09 hrs, Volume= 0.163 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.56' @ 12.09 hrs Surf.Area= 156 sf Storage= 8 cf

Plug-Flow detention time=0.1 min calculated for 0.163 af (100% of inflow)

Income Access Champions Describedions

Center-of-Mass det. time=0.1 min (786.8 - 786.6)

Volume	Inve	<u>rt Avail.Sto</u>	<u>rage Storag</u>	ge Description
#1	242.5	0' 3	06 cf Custo	om Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet		urf.Area (sq-ft) (c	Inc.Store cubic-feet)	Cum.Store (cubic-feet)
242.5	0	139	0	0
243.0	0	292	108	108
243.5	0	500	198	306
Device	Routing	Inver	t Outlet De	vices
#1	Primary	242.50	Head (fee 3.00 3.50	x 2.0' breadth Broad-Crested Rectangular Weir et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 glish) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=242.55' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 2.09 cfs @ 0.59 fps)

3.07 3.20 3.32

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 110

Summary for Pond 207: Level Lip Spreader #3

Inflow Area = 0.940 ac, 49.45% Impervious, Inflow Depth = 3.90" for 100-Yr event

Inflow = 4.21 cfs @ 12.09 hrs, Volume= 0.306 af

Outflow = 4.22 cfs @ 12.09 hrs, Volume= 0.306 af, Atten= 0%, Lag= 0.0 min

Primary = 4.22 cfs @ 12.09 hrs, Volume= 0.306 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 242.59' @ 12.09 hrs Surf.Area= 166 sf Storage= 13 cf

Plug-Flow detention time=0.1 min calculated for 0.305 af (100% of inflow)

Center-of-Mass det. time=0.1 min (831.5 - 831.4)

Volume	Invert	Avail.Storage	Storage Description
#1	242.50'	306 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation	Surt.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
242.50	139	0	0
243.00	292	108	108
243.50	500	198	306

Device Routing Invert Outlet Devices

#1 Primary 242.50' 65.0' long x 2.0' breadth Broad-Crested Rectangular Weir

 $\mbox{Head (feet)} \ \ 0.20 \ \ 0.40 \ \ 0.60 \ \ 0.80 \ \ 1.00 \ \ 1.20 \ \ 1.40 \ \ 1.60 \ \ 1.80 \ \ 2.00 \ \ 2.50$

3.00 3.50

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

3.07 3.20 3.32

Primary OutFlow Max=4.15 cfs @ 12.09 hrs HW=242.59' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 4.15 cfs @ 0.74 fps)

Summary for Pond 208: Level Lip Spreader #4

Inflow Area = 0.453 ac, 0.00% Impervious, Inflow Depth = 3.04" for 100-Yr event

Inflow = 1.55 cfs @ 12.10 hrs, Volume= 0.115 af

Outflow = 1.52 cfs @ 12.10 hrs, Volume= 0.110 af, Atten= 2%, Lag= 0.0 min

Primary = 1.52 cfs @ 12.10 hrs, Volume= 0.110 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.08' @ 12.10 hrs Surf.Area= 300 sf Storage= 200 cf

Plug-Flow detention time=29.5 min calculated for 0.110 af (96% of inflow)

Center-of-Mass det. time=7.8 min (856.7 - 848.9)

Volume	Invert	Avail.Storage	Storage Description
#1	248.00'	200 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

50465_postdev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 111

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
248.00	100	0	0
248.50	200	75	75
249.00	300	125	200

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 249.00'
 25.0' long x 2.0' breadth Broad-Crested Rectangular Weir

 Head (feet)
 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50

 3.00 3.50
 Coef. (English)
 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

0.10.10.L LINV. 0.40.00L /F ... D. . L

Primary OutFlow Max=1.51 cfs @ 12.10 hrs HW=249.08' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 1.51 cfs @ 0.73 fps)

3.07 3.20 3.32

Summary for Pond 209: Level Lip Spreader #5

Inflow Area = 0.376 ac, 0.00% Impervious, Inflow Depth = 2.62" for 100-Yr event

Inflow = 1.09 cfs @ 12.10 hrs, Volume= 0.082 af

Outflow = 1.28 cfs @ 12.10 hrs, Volume= 0.078 af, Atten= 0%, Lag= 0.0 min

Primary = 1.28 cfs @ 12.10 hrs, Volume= 0.078 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 249.07' @ 12.10 hrs Surf.Area= 300 sf Storage= 200 cf

Plug-Flow detention time=37.1 min calculated for 0.078 af (95% of inflow)

Center-of-Mass det. time= 10.0 min (868.4 - 858.4)

<u>Volume</u>	Invert	Avail.Storaç	ge Storag	e Description	
#1	248.00'	200	cf Custor	n Stage Data (P	rismatic) Listed below (Recalc)
Elevation (feet)	Surf. <i>A</i> (s		nc.Store pic-feet)	Cum.Store (cubic-feet)	
248.00		100	0	0	
248.50		200	75	75	
249.00		300	125	200	
Device f	Routing	Invert	Outlet Dev	rices	
#1 F	Primary	;	Head (feet 3.00 3.50	t) 0.20 0.40 0.6 lish) 2.54 2.61	road-Crested Rectangular Weir 0 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85

Primary OutFlow Max=1.28 cfs @ 12.10 hrs HW=249.07' (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 1.28 cfs @ 0.69 fps)

50465_postdev

Prepared by Stantec Consulting Services, Inc.

HydroCAD® 10.00-20 s/n 00734 © 2017 HydroCAD Software Solutions LLC

Page 112

Summary for Pond 301: Point of Analysis 301

Inflow Area = 43.307 ac, 58.99% Impervious, Inflow Depth > 4.37" for 100-Yr event

Inflow = 13.42 cfs @ 14.60 hrs, Volume= 15.783 af

Primary = 13.42 cfs @ 14.60 hrs, Volume= 15.783 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 401: Point of Analysis 401

Inflow Area = 59.444 ac, 46.27% Impervious, Inflow Depth > 4.02" for 100-Yr event

Inflow = 133.82 cfs @ 12.36 hrs, Volume= 19.921 af

Primary = 133.82 cfs @ 12.36 hrs, Volume= 19.921 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 501: Point of Analysis 501

Inflow Area = 0.915 ac, 26.47% Impervious, Inflow Depth = 5.14" for 100-Yr event

Inflow = 5.33 cfs @ 12.09 hrs, Volume= 0.391 af

Primary = 5.33 cfs @ 12.09 hrs, Volume= 0.391 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Pond 601: Point of Analysis 601

Inflow Area = 62.183 ac, 16.27% Impervious, Inflow Depth > 2.21" for 100-Yr event

Inflow = 36.86 cfs @ 12.60 hrs, Volume= 11.454 af

Primary = 36.86 cfs @ 12.60 hrs, Volume= 11.454 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link 1LA: Link 1

Inflow Area = 2.529 ac, 42.44% Impervious, Inflow Depth = 4.12" for 100-Yr event

Inflow = 4.76 cfs @ 12.83 hrs, Volume= 0.869 af

Primary = 4.76 cfs @ 12.83 hrs, Volume= 0.869 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link 1LB: Link 1

Inflow Area = 2.529 ac, 42.44% Impervious, Inflow Depth = 4.12" for 100-Yr event

Inflow = 4.76 cfs @ 12.83 hrs, Volume= 0.869 af

Primary = 4.76 cfs @ 12.83 hrs, Volume= 0.869 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

ATTACHMENT C

WATER QUALITY TREATMENT SUMMARY

Water Quality Treatment Summary - New Development
(Westerly Portion of the Project Site)

	Total Deve	eloped Area		Treated De	veloped Area
Subcatchment ID	New Impervious	New Landscaped	Treatment Method	Treated Impervious	Treated Landscaped
	Area (sf)	Area (sf)		Area (sf)	Area (sf)
\$32	0	7,974	Untreated	0	0
\$60	32,985	66,824	Untreated	0	0
S60A	0	36,652	Untreated	0	0
S61	119,736	28,185	Wet Pond	119,736	28,185
\$62	236,076	38,984	Wet Pond	236,076	38,984
\$63	15,000	4,049	Forested Buffer	15,000	4,049
S64	11,520	2,572	Forested Buffer	11,520	2,572
S65	20,240	20,692	Forested Buffer	20,240	20,692
S66	0	19,716	Forested Buffer	0	19,716
S67	0	16,384	Forested Buffer	0	16,384
Total	435,557	242,032		402,572	130,582

Treatment Summary						
Required Provided						
Treated Impervious (%)	90%1	92.43%				
Treated Landscaped (%)	75% ¹	78.68%				

Notes:

^{1.} Required treatment values were computed by applying Table 1 of Maine DEP's Chapter 500 Rules for Stormwater Management section 4.C.2.a.iii where 15.64 acres of land is developed of the available 43.07 acres deemed developable (36%).

	Water Quality Treatment Summary - Redevelopment (Easterly Portion of the Project Site)					
	Total Redev	eloped Area		Treated Red	eveloped Area	
Subcatchment ID	Impervious Area	Landscaped Area	Treatment Method	Treated Impervious	Treated Landscaped	
	(sf)	(sf)		Area (sf)	Area (sf)	
\$30	118,003	50,950	Untreated	0	0	
\$30C	360,000	75,913	Wet Pond	360,000	75,913	
S32	0	11,713	Untreated	0	0	
\$40	36,663	145,817	Untreated	0	0	
S40A	275,000	63,336	Wet Pond	275,000	63,336	
S 4 1	68,079	13,568	Untreated	0	0	
Total	857,745	361,297		635,000	139,249	

Treatment Summary						
Required Provided						
Treated Impervious (%)	N/A	74.03%				
Treated Landscaped (%)	60% ¹	63.51%				

Notes:

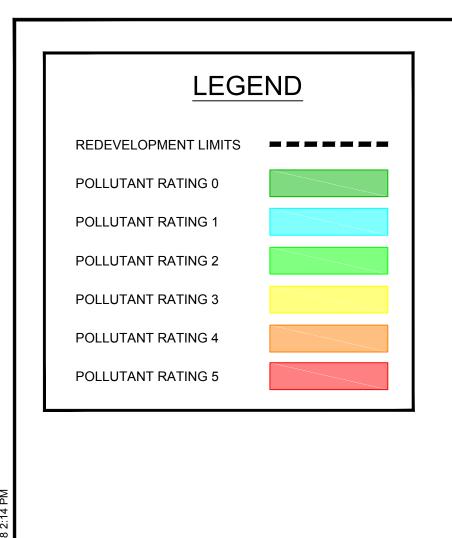
^{1.} Required treatment values were computed by applying section 4.C.2.d.i of Maine DEP's Chapter 500 Rules for Stormwater Management for Redevelopment Projects.

ATTACHMENT D

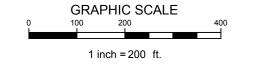
REDEVELOPMENT COMPUTATIONS AND FIGURES

	R	Redevelopi	ment Treatme	nt Calculat	ion		
	Existing Condition			Pro	Proposed Condition		
Pollutant Ranking	Area (sf)	Area (ac)	Impact Rating	Area (sf)	Area (ac)	Impact Rating	
0	9,710	0.22	0.00	0	0	0	
1	0	0.00	0.00	69,041	1.58	1.58	
2	297,062	6.82	13.64	279,119	6.41	12.82	
3	113,575	2.61	7.82	0	0.00	0.00	
4	798,695	18.34	73.34	870,882	19.99	79.97	
5	0	0.00	0.00	0	0	0	
Total	1,219,042	27.99	94.80	1,219,042	27.99	94.37	

De development Treatment Comment					
Redevelopment Treatment Summary					
Total Redevelopment Area (ac)	27.99				
Existing Impact Rating	94.80				
Proposed Impact Rating	94.37				
Existing Ranked Impact	3.39				
Proposed Ranked Impact	3.37				
Resultant Ranked Impact Change	-0.02				
Redeveloped Area Treatment	E097				
Designation (Site Law project)	50%				



Client/Project Copyright Reserved







Stantec 482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355

www.stantec.com

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

ST.GERMAIN COLLINS

MB BARK RECYCLING FACILITY EXPANSION

POLAND/AUBURN, ME
File Name: ex_redeve_calc

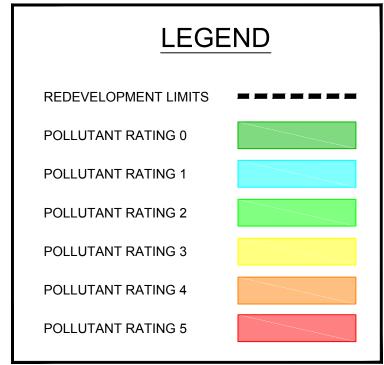
DWN. DBS AMS

DWN. CHKD. DSGN.

AMS 18.02.02
DSGN. DATE

REDEVELOPMENT FIGURE 1 EXISTING

Figure No.







Stantec 482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

MB BARK RECYCLING FACILITY EXPANSION

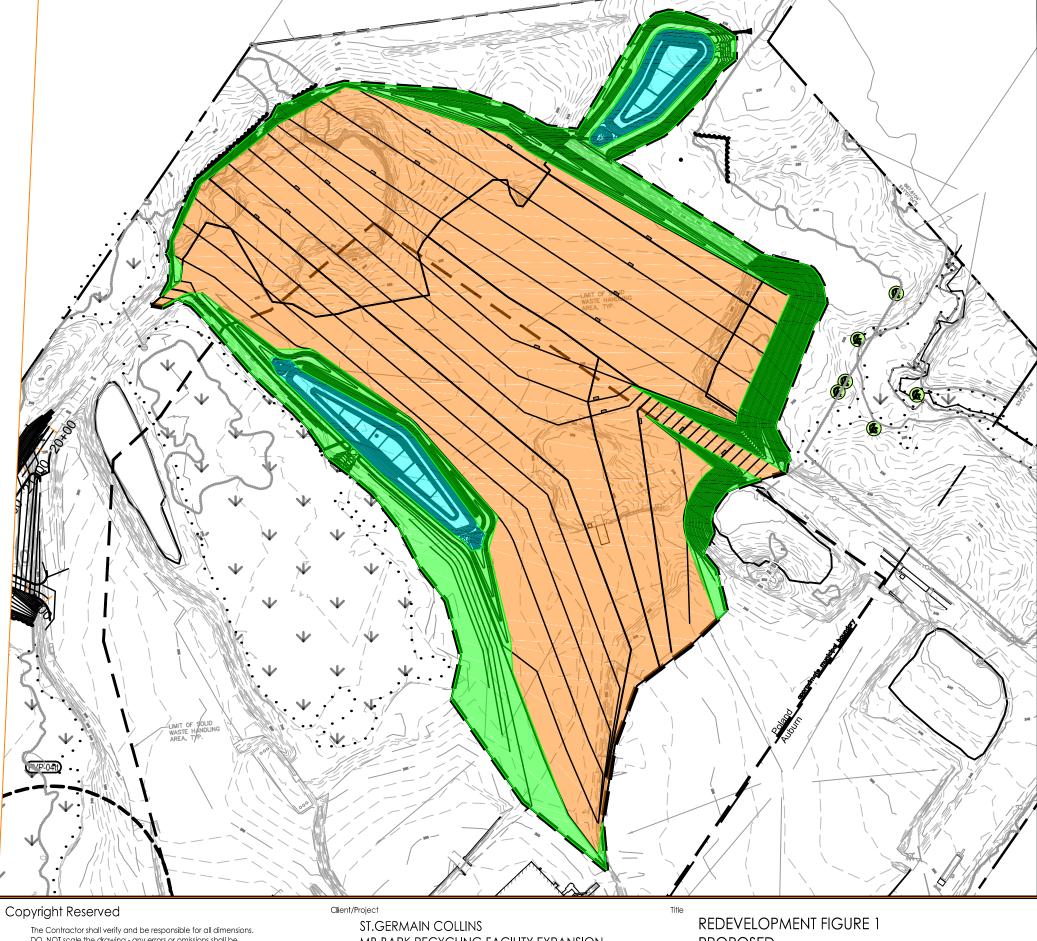
POLAND/AUBURN, ME

File Name: prop_redev_calc

AMS DBS AMS 18.02.02 DWN. CHKD. DSGN.

PROPOSED

GRAPHIC SCALE 1 inch = 200 ft.



ATTACHMENT E

WET POND COMPUTATIONS SUMMARY

Wet Pond Summary

Wet Pond #1

Tributary Impervious Area Tributary Landscaped Area Required Permanent Pool Volume Required Channel Protection Volume Provided Permanent Pool Volume Provided Channel Protection Volume	119,736 sf 28,185 sf 21,835 cf 10,918 cf 33,566 cf 10,918 cf
	10,010 01
Wet Pond #2	
Tributary Impervious Area Tributary Landscaped Area Required Permanent Pool Volume Required Channel Protection Volume Provided Permanent Pool Volume Provided Channel Protection Volume	236,076 sf 38,984 sf 41,945 cf 20,972 cf 46,728 cf 21,102 cf
Wet Pond #3	
Tributary Impervious Area Tributary Landscaped Area Required Permanent Pool Volume Required Channel Protection Volume Provided Permanent Pool Volume Provided Channel Protection Volume	360,000 sf 75,913 sf 65,061 cf 32,530 cf 69,139 cf 32,692 cf
Wet Pond #4	

275,000 sf

50,000 sf

49,167 cf

24,583 cf 49,669 cf

24,643 cf

Tributary Impervious Area

Tributary Landscaped Area

Required Permanent Pool Volume

Provided Permanent Pool Volume

Required Channel Protection Volume

Provided Channel Protection Volume

ATTACHMENT F

WET POND STAGE STORAGE COMPUTATIONS

	Wet Pond #1 Sizing					
Elevation	Surface Area	Average Area	Stage Volume	Cumulative Volume	Volume Above PPE	Mean Depth
242	874			0		0.00
		2094.5	2094.5			
243	3315			2094.5	0	0.63
		4212.5	4212.5			
244	5110			6307	0	1.23
		5536.5	5536.5		_	
245	5963			11843.5	0	1.99
0.40	0700	6376	6376	10010 5	•	0.00
246	6789	7000	7000	18219.5	0	2.68
247	7667	7228	7228	25447.5	0	3.32
247	7007	8118.5	8118.5	25447.5	U	3.32
248	8570	0110.0	0110.0	33566	0	PPV
210	0070	11867	10917.64	00000	Ŭ	11.
248.92	15164			36365.14	10917.64	CPV
	- 1 - 1	12225	12225			
249	15880			45791	23142.64	
		16772.5	16772.5			
250	17665			62563.5	39915.14	

			Wet Pond #2	Sizing		
Elevation	Surface Area	Average Area	Stage Volume	Cumulative Volume	Volume Above PPE	Mean Depth
236.2	6045			0	0	0.00
		6812	5449			
237	7578			5449	0	0.72
		8352	8352			
238	9125			13801	0	1.51
		9820	9820		_	
239	10514	44004	44004	23620	0	2.25
0.40	44540	11031	11031	0.4054	•	0.00
240	11548	10077	40077	34651	0	3.00
241	12606	12077	12077	46728	0	PPV
241	12000	15743	15743	40720	U	FFV
242	18880	10740	10740	62471	15743	
	10000	19138	5359	02111	101 10	
242.28	19396			67830	21102	CPV
		20059	14442			-
243	20722			82272	35544	
		24267	24267			
244	27811			106539	59811	

Wet Pond #3 Sizing						
Elevation	Surface Area	Average Area	Stage Volume	Cumulative Volume	Volume Above PPE	Mean Depth
239	1522			0	0	0.00
		2651	2651			
240	3780			2651	0	0.70
0.4.4	0077	5028.5	5028.5	7070 5	•	4.00
241	6277	7660 F	7660 F	7679.5	0	1.22
242	9062	7669.5	7669.5	15349	0	1.69
242	9002	10519.5	10519.5	15549	U	1.09
243	11977	.00.010	.00.00	25868.5	0	2.16
		12785.5	12785.5			
244	13594			38654	0	2.84
		14415	14415			
245	15236	40000 5	40000 5	53069	0	3.48
246	16903	16069.5	16069.5	69138.5	0	PPV
240	10903	22599.5	22599.5	09130.5	U	PPV
247	28296	22000.0	22000.0	91738	22599.5	
	20200	28835.35	10092.3725	000		
247.35	29374.7			101830.3725	32691.8725	CPV
		30376.35	19744.6275			
248	31378			121575	52436.5	
		32947.5	32947.5			
249	34517			154522.5	85384	

Wet Pond #4 Sizing						
Elevation	Surface Area	Average Area	Stage Volume	Cumulative Volume	Volume Above PPE	Mean Depth
229	3318			0	0	0
		4412.5	4412.5			
230	5507			4412.5	0	0.80
		6445	6445			
231	7383			10857.5	0	1.47
000	0707	8075	8075	40000 5	•	0.40
232	8767	0050	0050	18932.5	0	2.16
233	9739	9253	9253	28185.5	0	2.00
233	9739	10237	10237	20100.0	U	2.89
234	10735	10237	10201	38422.5	0	3.58
204	10700	11246	11246	00122.0	Ŭ	0.00
235	11757			49668.5	0	PPV
		15333	15333			
236	18909			65001.5	15333	
		19396.2	9310.176			
236.48	19883.4			74311.676	24643.176	CPV
		20411.2	10613.824			
237	20939			84925.5	35257	
000	00000	21985.5	21985.5	100011	570.40.5	
238	23032	0.4404	04404	106911	57242.5	
220	05040	24121	24121	404000	04000 5	
239	25210			131032	81363.5	

ZONING

G-2, GENERAL PURPOSE 2 (POLAND) ID, INDUSTRIAL DISTRICT (AUBURN)

TAX ASSESSOR'S TAX MAP and LOT

0015-0002 (POLAND) 119.20 AC. 001-001 (AUBURN) 23.98 AC.

OWNER

MB INVESTMENTS, LLC

70 PLEASANT HILL ROAD SCARBOROUGH, MAINE 04074 (207) 883-3325

APPLICANT MB BARK, LLC

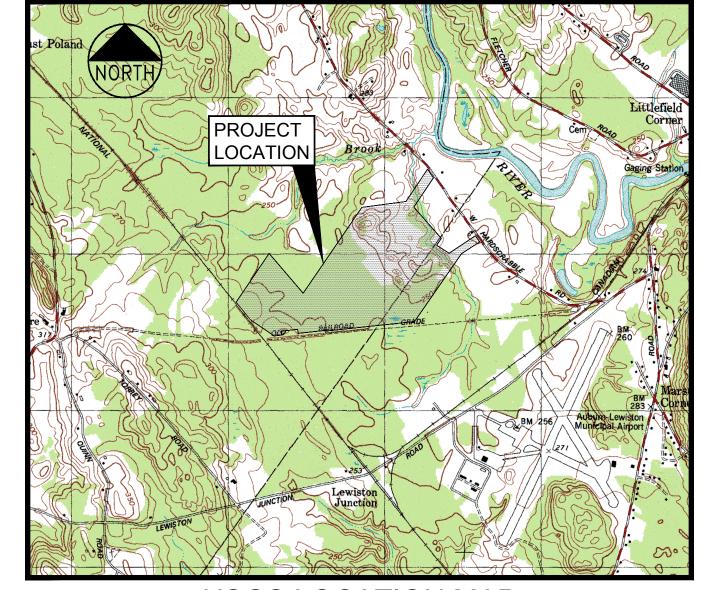
100 BARK MULCH DRIVE AUBURN, MAINE 04210

SITE DEVELOPMENT PLANS FOR

MB BARK RECYCLING FACILITY EXPANSION

100 BARK MULCH DRIVE AUBURN, MAINE / POLAND, MAINE

PERMIT SET FEBRUARY 2019



USGS LOCATION MAP SCALE: 1" = 2000'

Notes

482 Payne Road Scarborough Court Scarborough, Maine 04074-8929

> The Contractor shall verify and be responsible for all dimensions. DC NOT scale the drawing - any errors or omissions shall be reported t

The Copyrights to all designs and drawings are the property of

Stantec. Reproduction or use for any purpose other than that

St.Germain Collins

Tel. 207.883.3355

www.stantec.com

Consultants

846 Main Street

Tel. 207.591.7000

Westbrook, Maine 04092

Copyright Reserved

INDEX

COVER SHEET GENERAL NOTES & LEGEND **EXISTING CONDITIONS SURVEY 1 OF 3 EXISTING CONDITIONS SURVEY 2 OF 3 EXISTING CONDITIONS SURVEY 3 OF 3** OVERALL SITE PLAN CONSTRUCTION PHASING PLAN C-3.1 POST-CONSTRUCTION MATERIALS HANDLING PLAN OVERALL GRADING PLAN - INDEX SHEET GRADING, DRAINAGE, & EROSION CONTROL PLAN 1 OF 10 GRADING, DRAINAGE, & EROSION CONTROL PLAN 2 OF 10 GRADING, DRAINAGE, & EROSION CONTROL PLAN 3 OF 10 C-4.4 GRADING, DRAINAGE, & EROSION CONTROL PLAN 4 OF 10 GRADING, DRAINAGE, & EROSION CONTROL PLAN 5 OF 10 C-4.5 GRADING, DRAINAGE, & EROSION CONTROL PLAN 6 OF 10 C-4.6 C-4.7 GRADING, DRAINAGE, & EROSION CONTROL PLAN 7 OF 10 GRADING, DRAINAGE, & EROSION CONTROL PLAN 8 OF 10 C-4.8 C-4.9 GRADING, DRAINAGE, & EROSION CONTROL PLAN 9 OF 10 C-4.10 GRADING, DRAINAGE, & EROSION CONTROL PLAN 10 OF 10 C-5.0 WET POND #1 PLAN VIEW C-5.1 WET POND #1 SECTIONS WET POND #2 PLAN VIEW C-5.2 C-5.3 WET POND #2 SECTIONS C-5.4 WET POND #3 PLAN VIEW

C-5.5 WET POND #3 SECTIONS C-5.6 WET POND #4 PLAN VIEW C-5.7 WET POND #4 SECTIONS

C-6.0 PREDEVELOPMENT WATERSHED PLAN C-6.1 POST DEVELOPMENT WATERSHED PLAN

I HEREBY ACKNOWLEDGE THAT THESE PLANS AND SPECIFICATIONS WERE PREPARED UNDER MY DIRECT SUPERVISION, AND THAT I AM A

DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE

STATE OF MAINE AND THAT I AM COMPETENT TO PREPARE THIS

C-7.0 **EROSION CONTROL NARRATIVE** C-7.1 SITE DETAILS C-7.2 SITE DETAILS C-7.3 SITE DETAILS

SITE DETAILS

C-7.4

UTILITIES

WATER **AUBURN WATER DISTRICT** 268 COURT STREET - P.O. BOX 414 AUBURN, MAINE 04212-0414 (207) 784-6469

ELECTRIC CENTRAL MAINE POWER 162 CANCO ROAD PORTLAND. MAINE 04103 (207) 753-3117

TELEPHONE **FAIRPOINT COMMUNICATIONS** 5 DAVIS FARM ROAD PORTLAND, MAINE 04103 (207) 761-1200

CABLE SPECTRUM/TIME WARNER 118 JOHNSON ROAD PORTLAND, MAINE 04102

CALL BEFORE YOU DIG

PERMITS

TYPE: **ISSUING AGENCY:** DATE OF APPLICATION: STATUS: LOCAL SITE PLAN TOWN OF POLAND FEB. 01, 2019 1231 MAINE STREET POLAND, ME 04274 PH. (207) 998-4601

STATE

FEDERAL

(CATEGORY 2)

ACOE GENERAL PERMIT

MAINE DEPARTMENT OF SOLID WASTE HANDLING **FACILITY PERMIT**

ENVIRONMENTAL PROTECTION 17 STATE HOUSE STATION AUGUSTA. MAINE 04333 (207) 287-7688

MAINE DEPARTMENT OF NRPA INDIVIDUAL PERMIT **ENVIRONMENTAL PROTECTION** 17 STATE HOUSE STATION AUGUSTA, MAINE 04333

(207) 287-7688

(207) 623-8367

MAINE PROJECT OFFICE 442 CIVIC CENTER DRIVE SUITE 350

AUGUSTA, MAINE 04330

#S-021741-WK-I-A (APPROVED 11/28/2018) #S-021741-W4-J-N (APPROVED 11/28/2018)

L-19467-TG-C-N (APPROVED 6/19/2018) L-19467-L6-D-N (APPROVED 6/19/2018)

NAE-2018-00135 (APPROVED 6/19/2018)

STANTEC

30 PARK DR TOPSHAM, ME 04086 TEL: (207) 729-1199 ATTN: THOMAS TETREAU TOM.TETREAU@STANTEC.COM

133 GRAY ROAD. FALMOUTH, MAINE 04105 TEL. (207) 797-9199 ATTN.: REX CROTEAU RCROTEAU@TITCOMBSURVEY.COM

GEOTECHNICAL ENGINEER:

LEWISTON, MAINE 04240 TEL: (207) 576-3313 ATTN: BILL PETERLEIN BPETERLEIN@SUMMITGEOENG.COM

PREPARED BY

PROJECT MANAGEMENT & PERMITTING: ST.GERMAIN COLLINS

846 MAIN STREET WESTBROOK, MAINE 04092 TEL: (207) 591-7000 ATTN: PETER MOHLIN PETERM@STGERMAINCOLLINS.COM

CIVIL ENGINEER: STANTEC

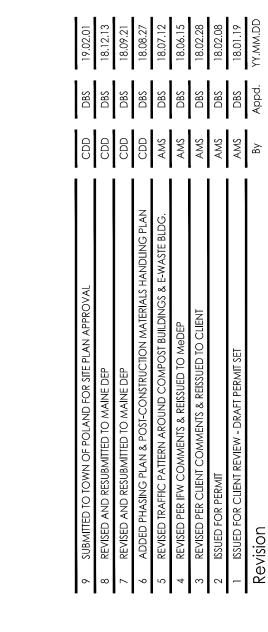
482 PAYNE ROAD SCARBOROUGH COURT SCARBOROUGH, MAINE 04074 TEL: (207) 883-3355 ATTN: DARRIN STAIRS, P.E. DARRIN.STAIRS@STANTEC.COM

WETLAND/NATURAL RESOURCES:

SURVEYOR: TITCOMB ASSOCIATES

SUMMIT GEOENGINEERING

145 LISBON STREET, SUITE 701



Stantec

Permit-Seal

02.01.19

Client/Project

File Name: 50465 cover

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION AUBURN, MAINE / POLAND, MAINE

COVER SHEET

Scale Project No. 195350465 Sheet

DARRIN STAIRS, P.E.#15118

DOCUMENT.

C-1.0

GENERAL NOTES

- 1. THIS PROJECT WILL BE SUBJECT TO THE TERMS AND CONDITIONS OF ALL PERMITS ISSUED BY THE MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION, ARMY CORPS OF ENGINEERS, LOCAL UTILITY COMPANIES, AND THE TOWN OF POLAND.
- 2. ALL MATERIALS AND CONSTRUCTION METHODS SHALL CONFORM TO THE MOST RESTRICTIVE OF THE MAINE DEPARTMENT OF TRANSPORTATION SPECIFICATIONS, THE PROJECT SPECIFICATIONS, THE LOCAL UTILITY COMPANIES AND TOWN OF POLAND REQUIREMENTS.
- 3. THE SITE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COLLECTION, TRANSPORT, AND DISPOSAL OF ALL CONSTRUCTION AND DEMOLITION WASTES IN ACCORDANCE WITH STATE AND LOCAL RECULATIONS
- 4. THE SITE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR THE ELEVATION OF THE EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES AND, WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THIS INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANY AND DIG SAFE AND A PRIVATE UTILITY LOCATION COMPANY SUCH AS DIG SMART AT LEAST 72 HOURS PRIOR TO ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.
- 5. MAINTENANCE OF EROSION CONTROL MEASURES IS OF PARAMOUNT IMPORTANCE TO THE OWNER. THE SITE CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH ALL EROSION CONTROL MEASURES SHOWN ON THE PLANS. ADDITIONAL EROSION CONTROL MEASURES SHALL BE INSTALLED IF DEEMED NECESSARY BY ON—SITE INSPECTIONS OF THE OWNER, REPRESENTATIVES OF THE DEP, OR REPRESENTATIVES OF THE TOWN OF POLAND, AT NO ADDITIONAL COST TO THE OWNER.
- 6. ALL MATERIAL SCHEDULES SHOWN ON THE PLANS ARE FOR GENERAL INFORMATION ONLY. THE SITE CONTRACTOR SHALL PREPARE HIS OWN MATERIAL SCHEDULES BASED UPON HIS PLAN REVIEW. ALL SCHEDULES SHALL BE VERIFIED IN THE FIELD BY THE CONTRACTOR PRIOR TO ORDERING MATERIALS OR PERFORMING WORK.
- 7. ALL REQUIRED AND NECESSARY INSPECTIONS AND/OR CERTIFICATIONS REQUIRED BY CODES AND/OR UTILITY SERVICE COMPANIES SHALL BE PERFORMED PRIOR TO THE FINAL SERVICE CONNECTIONS IN THE CONTRACT.
- 9. RECORD DRAWINGS REQUIRE ALL BURIED BENDS, APPURTENANCES, AND OTHER FEATURES TO BE LOCATED BY COORDINATES. SEE SURVEY AND LAYOUT SPECIFICATIONS FOR EXACT REQUIREMENTS.
- 10. TEST PITS SHALL BE PERFORMED WITH IN 3 DAYS OF THE START OF WORK.

LANDSCAPE NOTES:

- 1. LIMITS OF TREE AND SHRUB CLEARING TO BE FLAGGED BY THE SITE CONTRACTOR. THE ENGINEER AND/OR THE OWNER'S REPRESENTATIVE SHALL APPROVE ALL CLEARING LIMITS AND SHALL FLAG AND MARK EXISTING TREES NEAR CLEARING LIMIT TO REMAIN. THIS LAYOUT AND OWNER'S REVIEW SHALL BE CONDUCTED PRIOR TO ANY TREE OR SHRUB CUTTING.
- 2. ALL DISTURBED AREAS ARE TO RECEIVE A MINIMUM OF 4" OF TOPSOIL PRIOR TO PERMANENT SEEDING.

EROSION CONTROL NOTES:

- 1. REFER TO THE EROSION CONTROL REPORT FOR REQUIREMENTS FOR THIS PROJECT.
- 2. THE LIMITS OF GRADING SHALL BE ESTABLISHED, IN THE FIELD, BY THE CONTRACTOR BY SURVEY LAYOUT AND APPROVED BY THE OWNER, IN WRITING, PRIOR TO ANY OTHER WORK. PRIOR TO BEGINNING OTHER LAND DISTURBING ACTIVITIES, THE CONTRACTOR SHALL INSTALL THE PERIMETER SILT FENCES AND THE CONSTRUCTION ENTRANCES.
- 3. ALL AREAS OUTSIDE THE PROPOSED PAVEMENT AND BUILDING LIMITS SHALL BE GRADED, LOAMED, MULCHED, MESHED (IF REQUIRED BY SLOPE) AND SEEDED AS SOON AS POSSIBLE EXCEPT WHERE RIPRAP IS SPECIFIED. SEED MIXTURES SHALL CONFORM TO THE SEEDING PLAN CONTAINED IN THE EROSION CONTROL REPORT PREPARED FOR THIS PROJECT.
- 4. ALL STORM DRAIN INLETS AND OUTLETS NOT IN PAVED AREAS ARE TO RECEIVE RIPRAP PROTECTION
- 5. SILT FENCES SHALL BE INSPECTED, REPAIRED AND CLEANED AS NOTED IN THE EROSION CONTROL NOTES SHOWN ON THE EROSION CONTROL DETAIL SHEET.
- 6. THE SITE CONTRACTOR SHALL REPAIR AND ADD STONE TO THE CONSTRUCTION ENTRANCE AS IT BECOMES SATURATED WITH MUD TO ENSURE THAT IT WORKS AS PLANNED DURING CONSTRUCTION AND KEEP ALL OFFSITE ROADS CLEAR OF DIRT AND MUD.
- 7. SILT REMOVED FROM AROUND INLETS AND BEHIND THE SILT FENCES SHALL BE PLACED AND MIXED INTO TOPSOIL FOR USE IN LANDSCAPING OPERATIONS.
- 8. EROSION CONTROL MEASURES ARE SHOWN ON THE EROSION AND SEDIMENT CONTROL PLAN SHEET OF THIS PLAN SET. THESE ARE MINIMUM REQUIREMENTS. ADDITIONAL MEASURES SHALL BE INSTALLED AS NECESSARY. THE USE OF CHEMICAL AGENTS OR FLOCCULANTS MAY BE REQUIRED AND SHALL BE USED IF NECESSARY TO CONTROL AND PREVENT TURBID DISCHARGES.
- 9. LAND DISTURBING ACTIVITIES SHALL BE ACCOMPLISHED IN A MANNER AND SEQUENCE THAT CAUSES THE LEAST PRACTICAL DISTURBANCE OF THE SITE AND COMPLIES WITH THE EROSION CONTROL PLANS AND SPECIFICATIONS FOR THE PROJECT.

DEMOLITION NOTES:

1. THE SITE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLETE REMOVAL & DISPOSAL OF DEMOLITION & CONSTRUCTION DEBRIS GENERATED FROM SITE WORK ACTIVITIES.

GRADING AND DRAINAGE NOTES:

- 1. ALL STORM DRAIN PIPE SHALL BE SMOOTH BORE INTERIOR PROVIDING A MANNING'S ROUGHNESS COEFFICIENT OF N=.012 OR LESS EXCEPT WHERE CONCRETE IS USED FOR INLET FLARES, SEE SPECIFICATION SECTION 02670.
- 2. ALL SLOPES IN EXCESS OF 3:1 HORIZONTAL TO VERTICAL ARE TO RECEIVE SPECIAL SLOPE TREATMENT AS DEFINED BY THE EROSION CONTROL NARRATIVE, DETAIL SHEETS, OR TYPICAL SECTIONS OF THIS PLAN SET. ALL SLOPES OVER 10% ARE TO BE MULCHED AND NETTED.
- 3. AN "AS-BUILT" CERTIFICATION OF THE STORMWATER DRAINAGE SYSTEM PREPARED BY A REGISTERED ENGINEER IS REQUIRED PRIOR TO SUBMITTING A REQUEST FOR SUBSTANTIAL COMPLETION OF THE PROJECT. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT ANY DEVIATION FROM THE PLANS MAY DELAY THE ACCEPTANCE OF THE PROJECT.
- 4. ALL EXPOSED PIPES OVER 18" IN DIAMETER ARE TO HAVE A REINFORCED CONCRETE FLARE WITH BAR RACK. PIPES 18" DIAMETER AND UNDER ARE TO HAVE AN HDPE FLARE AS SHOWN ON THE DETAILS OF THE DRAWINGS.

UTILITY NOTES:

- 1. ALL REQUIRED UTILITIES SERVING THE PROJECT SHALL BE COORDINATED AND CONSTRUCTED BY THE SITE CONTRACTOR AND THEIR SUBCONTRACTORS.
- 2. THE SITE CONTRACTOR SHALL COORDINATE THE PROTECTION OF THE UNDERGROUND ELECTRIC POWER SERVICE WITH CENTRAL MAINE POWER.

LEGEND

EXISTING	DESCRIPTION	PROPOSED
	PROPERTY LINE	
	ABUTTING PROPERTY LINE / R.O.W. LINE	≣
N/F JOHN Q. OWNER 1111/222	PROPERTY OWNER NAME & DEED INFO.	
<u> </u>	EASEMENT LINE	C SMU1
© 0	SEWER MANHOLE DRAIN MANHOLE	⊚ ^{SMH1}
© CB□ ©	CATCH BASIN	© DMH1 CB1 ■
sd		CB1 ■ CB1
g		6" G
s	SEWER LINE	12"SAN
——— W ———	WATER LINE	8"W
aptc	AERIAL POWER - TEL - CABLE	———OHE/T/C———
up+t	POWER - TEL - CABLE LINES	———UGE/T/C——
UP CO.	UTILITY POLE	ø
	POLE W/LIGHT	
- 0.	POLE W/GUY	AA BB DD
\$	LIGHT POLE	
tran.	TRANSFORMER PAD	<u>T</u>
-Q- нүр. wv	FIRE HYDRANT WATER GATE	+
G∨ ⊠	GAS GATE	
①	TEL MANHOLE	
©	ELEC MANHOLE	
© ®	BOULDERS	₿
	BENCH	•
•	CONCRETE SEAT	

	EDGE OF PAVEMENT	99
99	GRADING CONTOUR LINE	99.63
X 99.63	GRADING SPOT GRADE	× ************************************
/	TREELINE / CLEARING LIMIT	······
	BUILDING LINE	
	STEPS	
-	SIGN	•
FOUND IRON PIPE		
O D REBAR	BOUNDARY MONUMENTATION	• •
tbm#1	TEMPORARY BENCH MARK	
TP-7	TEST PIT	
0 0 0 0	GUIDERAIL	
• 7117 717 •	EXISTING WETLANDS	
· · · · · · · · · ·		
	STREAM / POND	
——————————————————————————————————————	FENCE	xx
	RIPRAP	
	BUILDING HATCH	
	PAVEMENT HATCH	
	CONCRETE HATCH	
	CONCRETE PAVERS HATCH	
	LEDGE OUTCROP HATCH	
	LEGE GOLOVOL HVIOLI	



Stantec
482 Payne Road Scarborough Court

Scarborough, Maine 04074-8929
Tel. 207.883.3355
www.stantec.com

Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

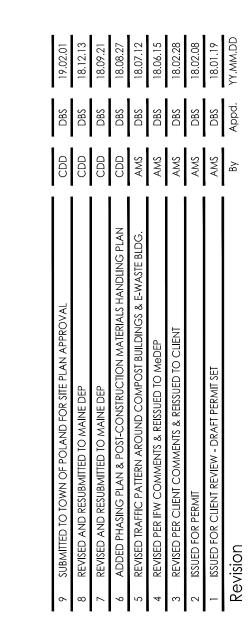
The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

Consultants

St.Germain Collins

846 Main Street Westbrook, Maine 04092 Tel. 207.591.7000

Notes



02.01.19
CENSED

Client/Project

File Name: 50465 aen

Permit-Seal

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION

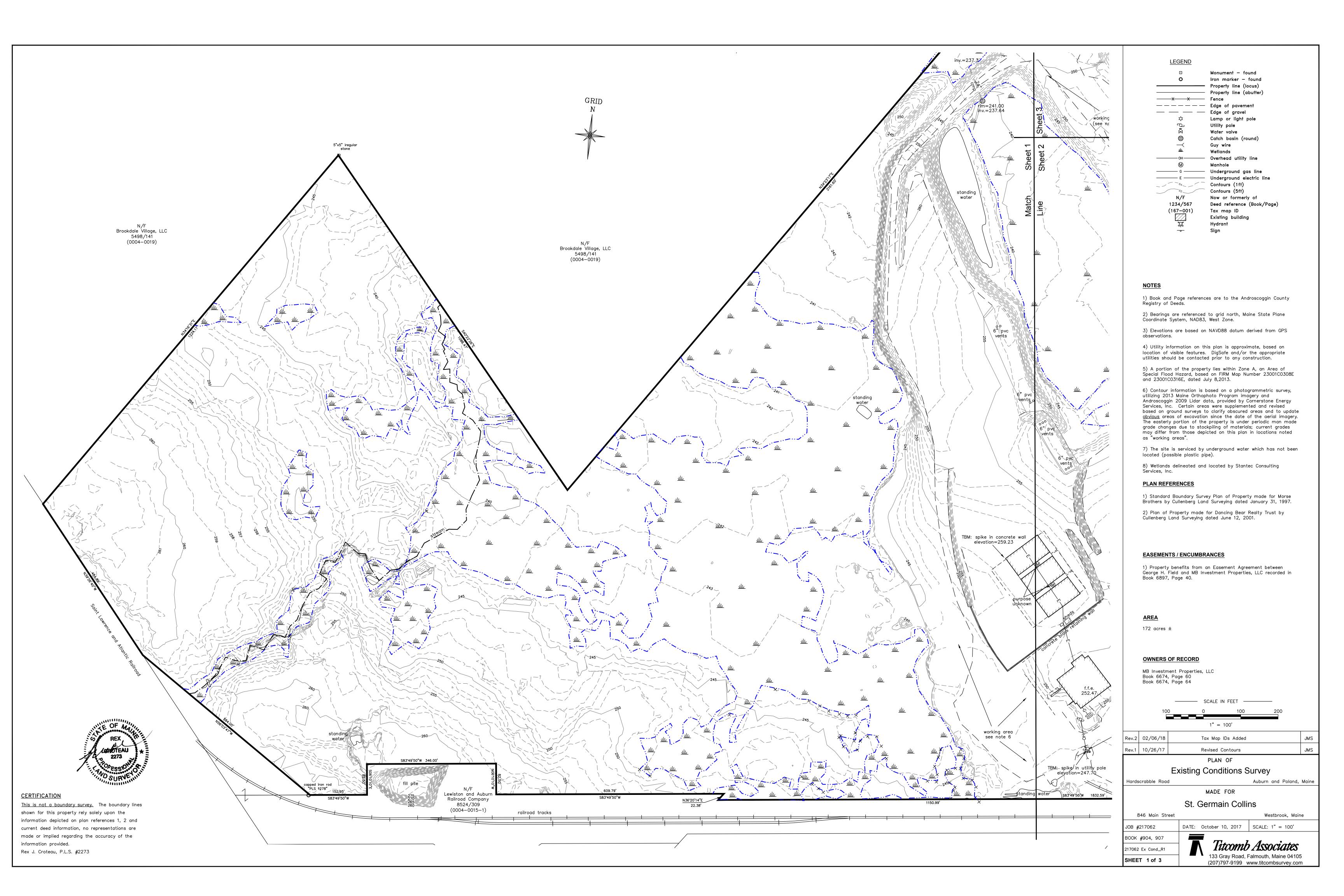
AUBURN, MAINE / POLAND, MAINE

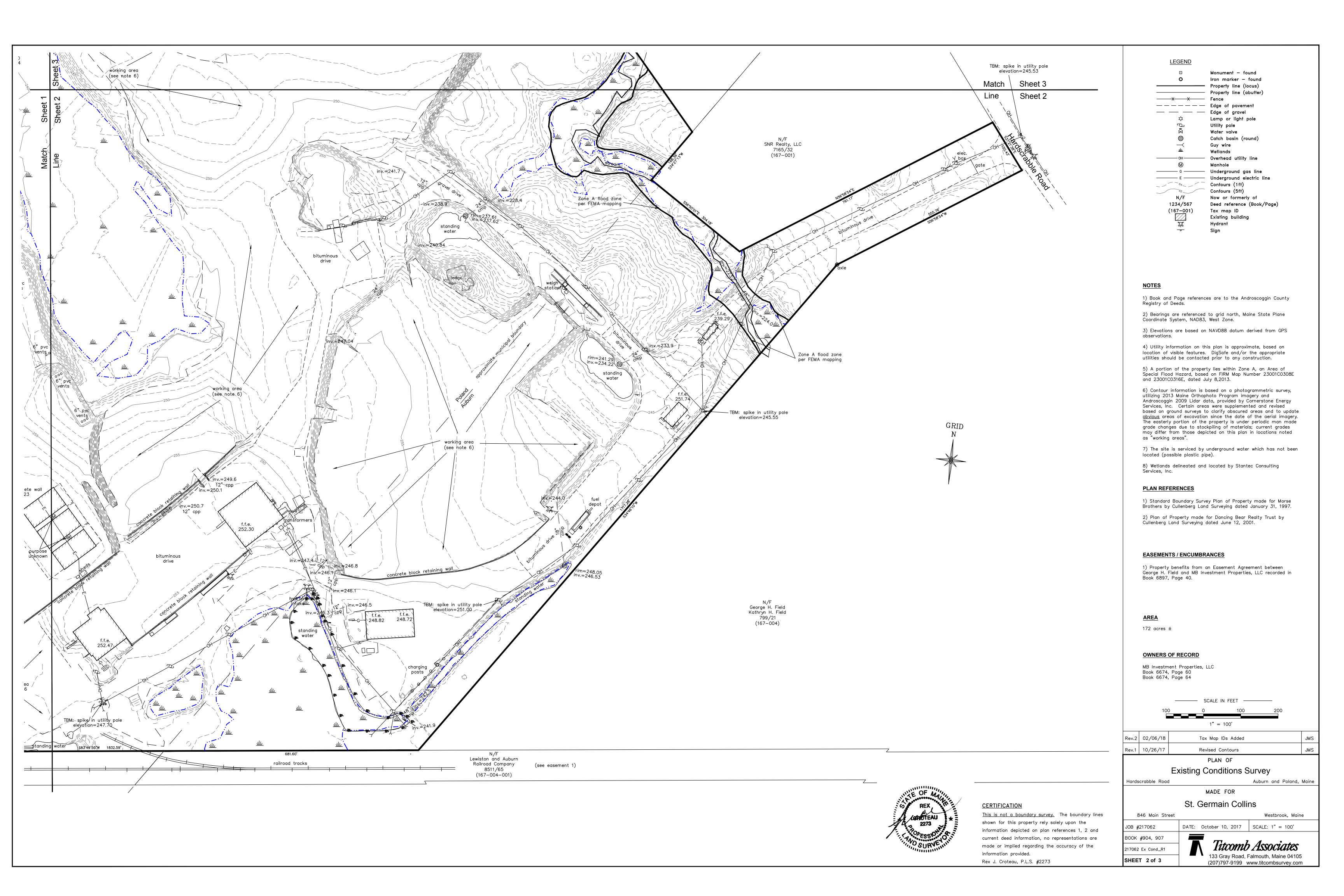
Title

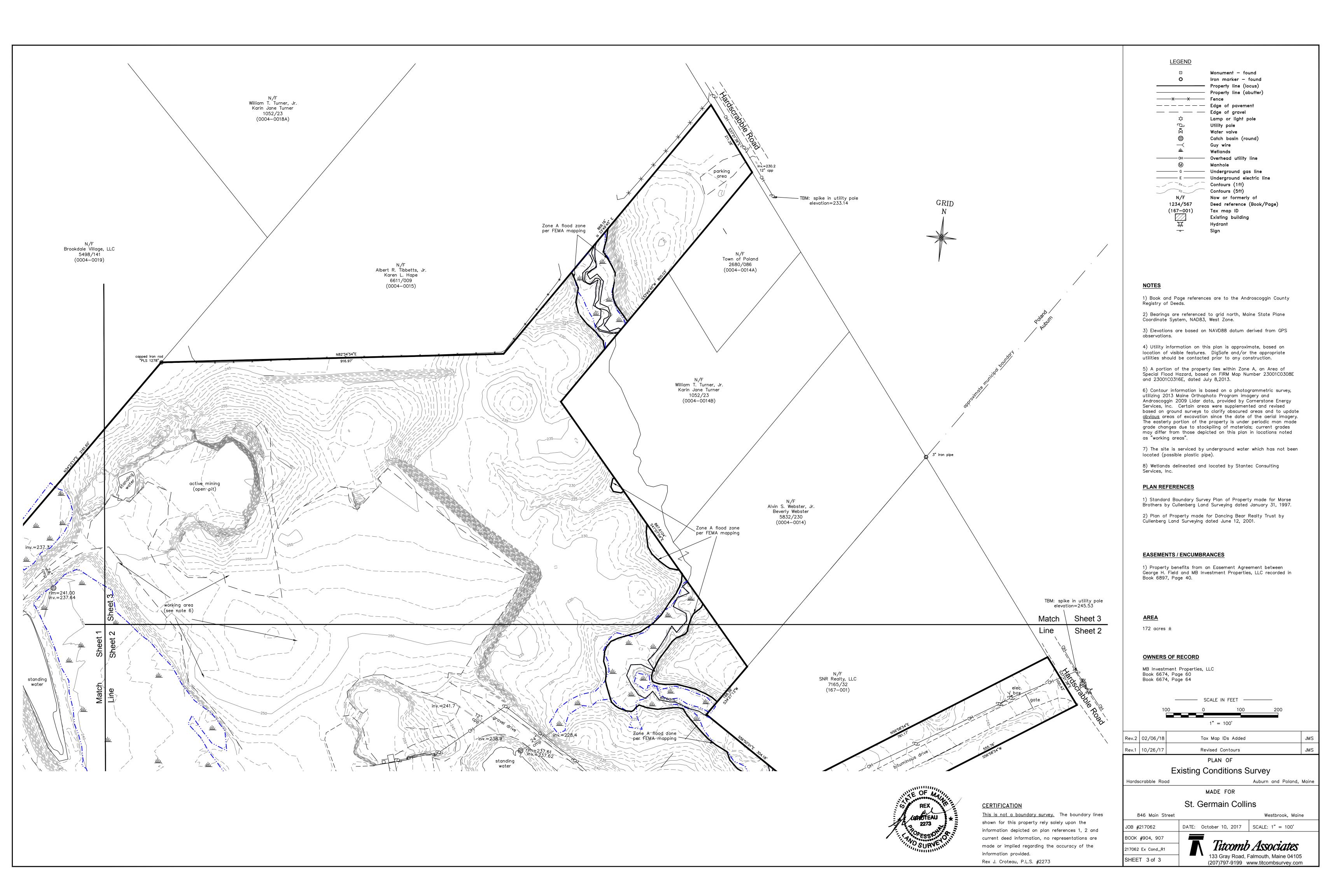
GENERAL NOTES & LEGEND

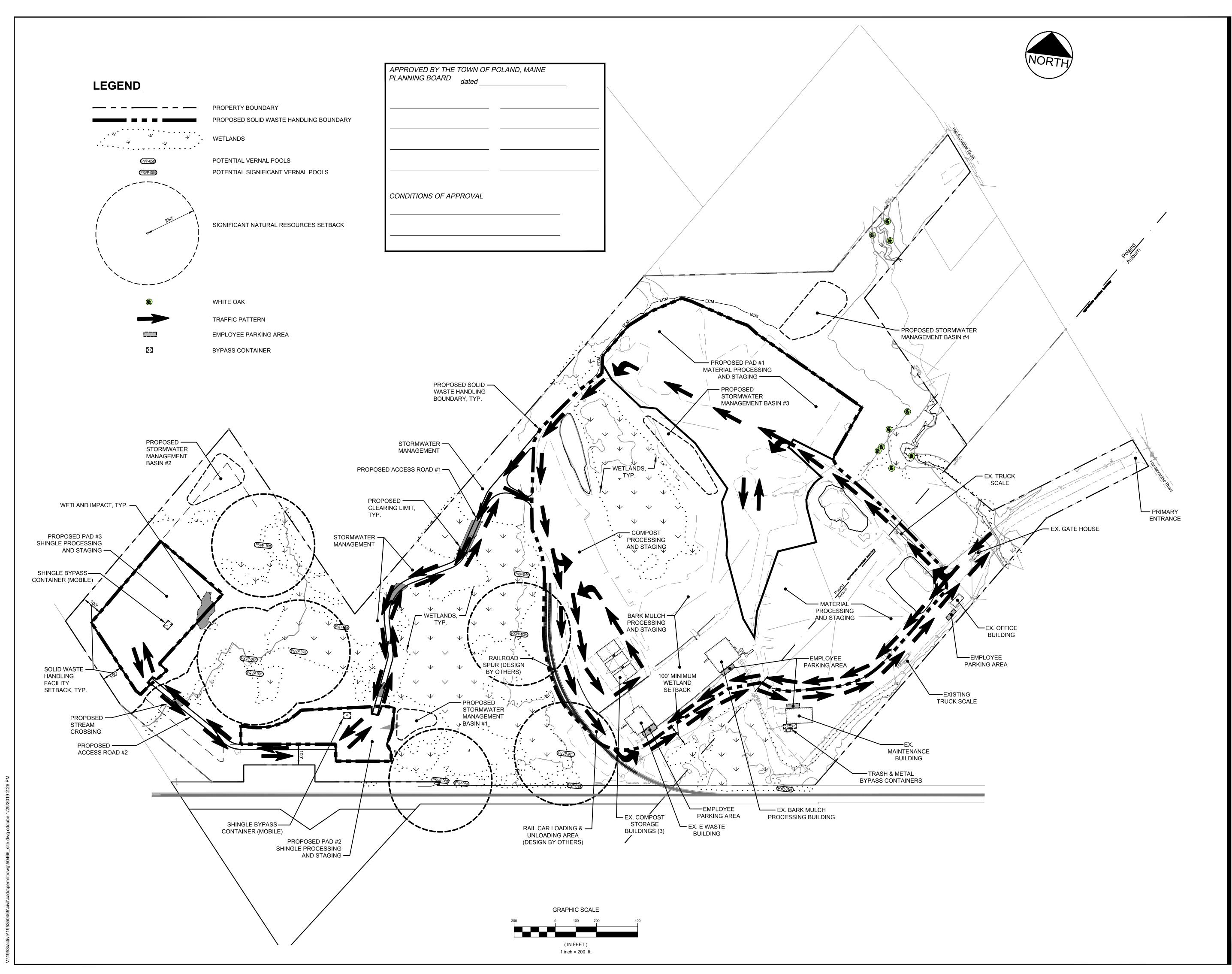
Project No. Scale
195350465
Sheet

C-1.1











482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355

authorized by Stantec is forbidden.

Copyright Reserved

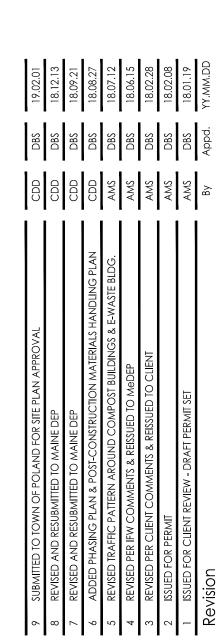
www.stantec.com

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay. The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

Consultants

846 Main Street Westbrook, Maine 04092

Tel. 207.591.7000



File Name: 50465_site

Permit-Seal



Client/Project

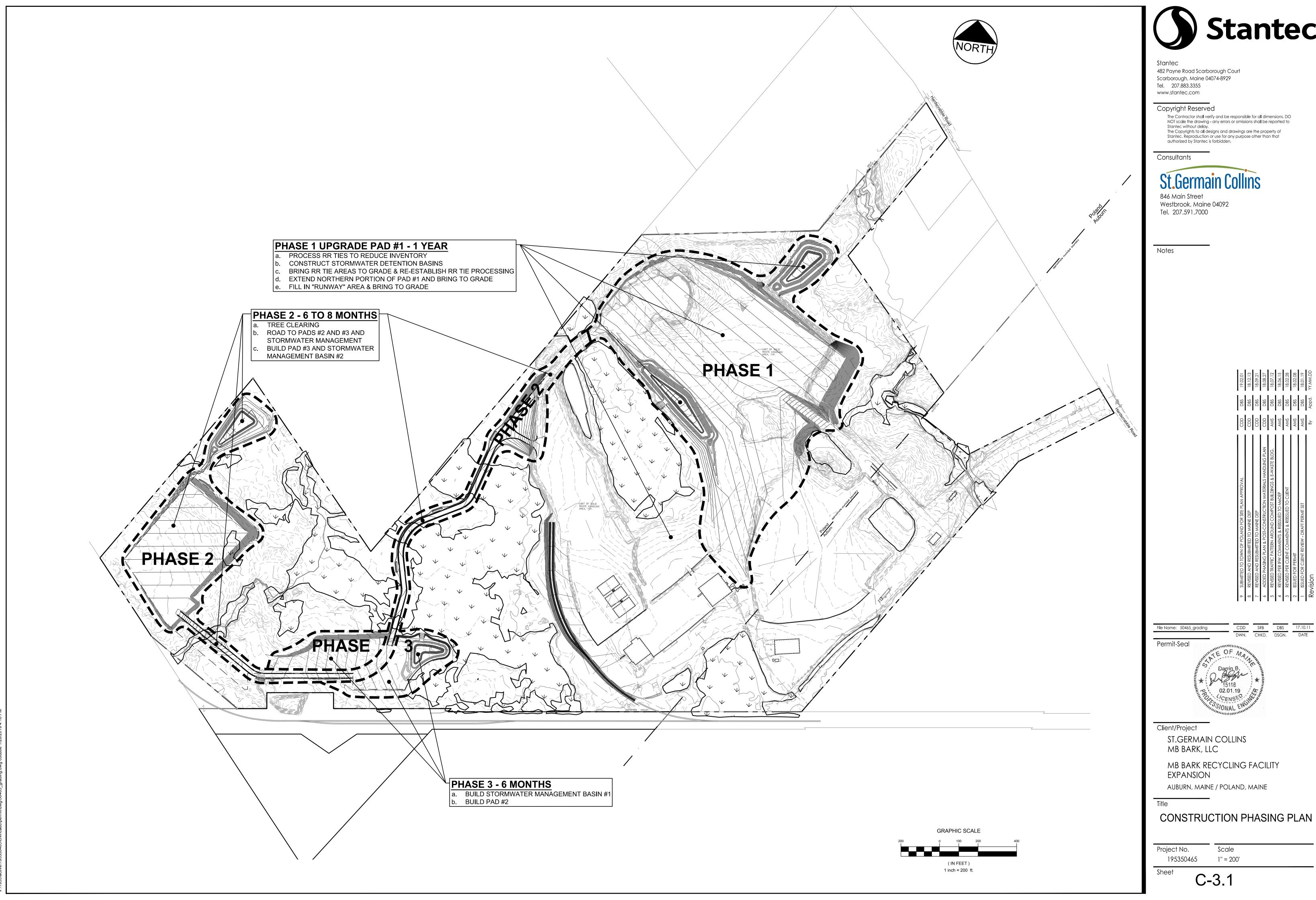
ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION AUBURN, MAINE / POLAND, MAINE

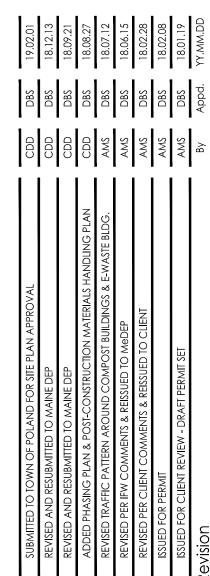
OVERALL SITE PLAN

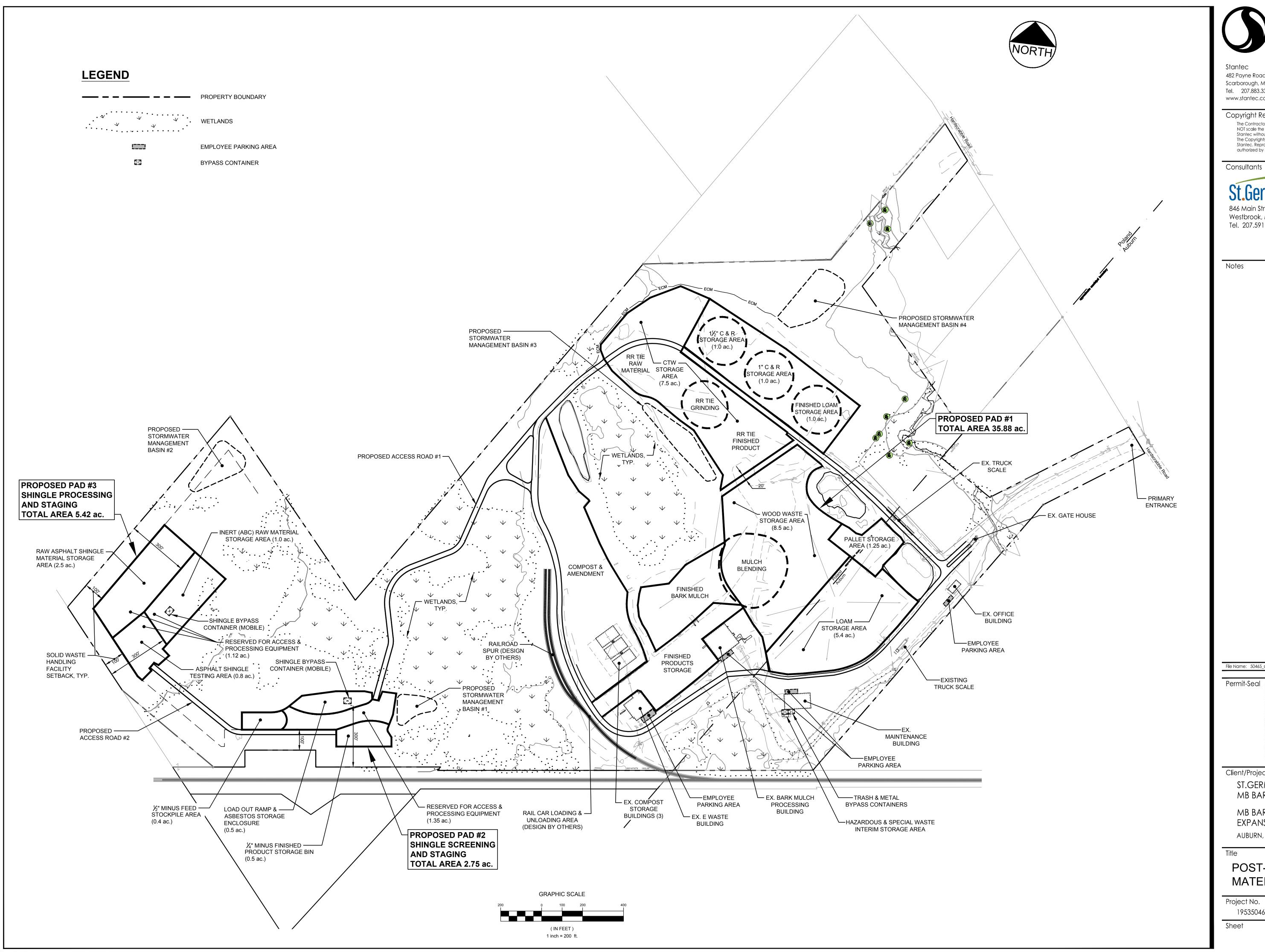
Project No. Scale 195350465 1'' = 200'

C-3.0











482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355

Copyright Reserved

www.stantec.com

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay. The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

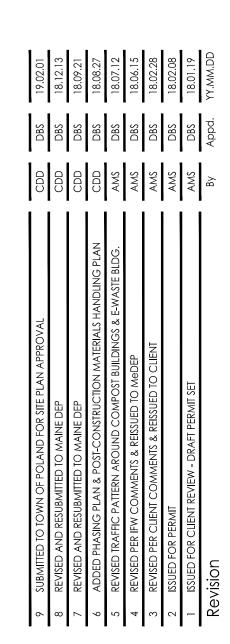
Consultants

846 Main Street

Westbrook, Maine 04092

Tel. 207.591.7000

authorized by Stantec is forbidden.



File Name: 50465_site



Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

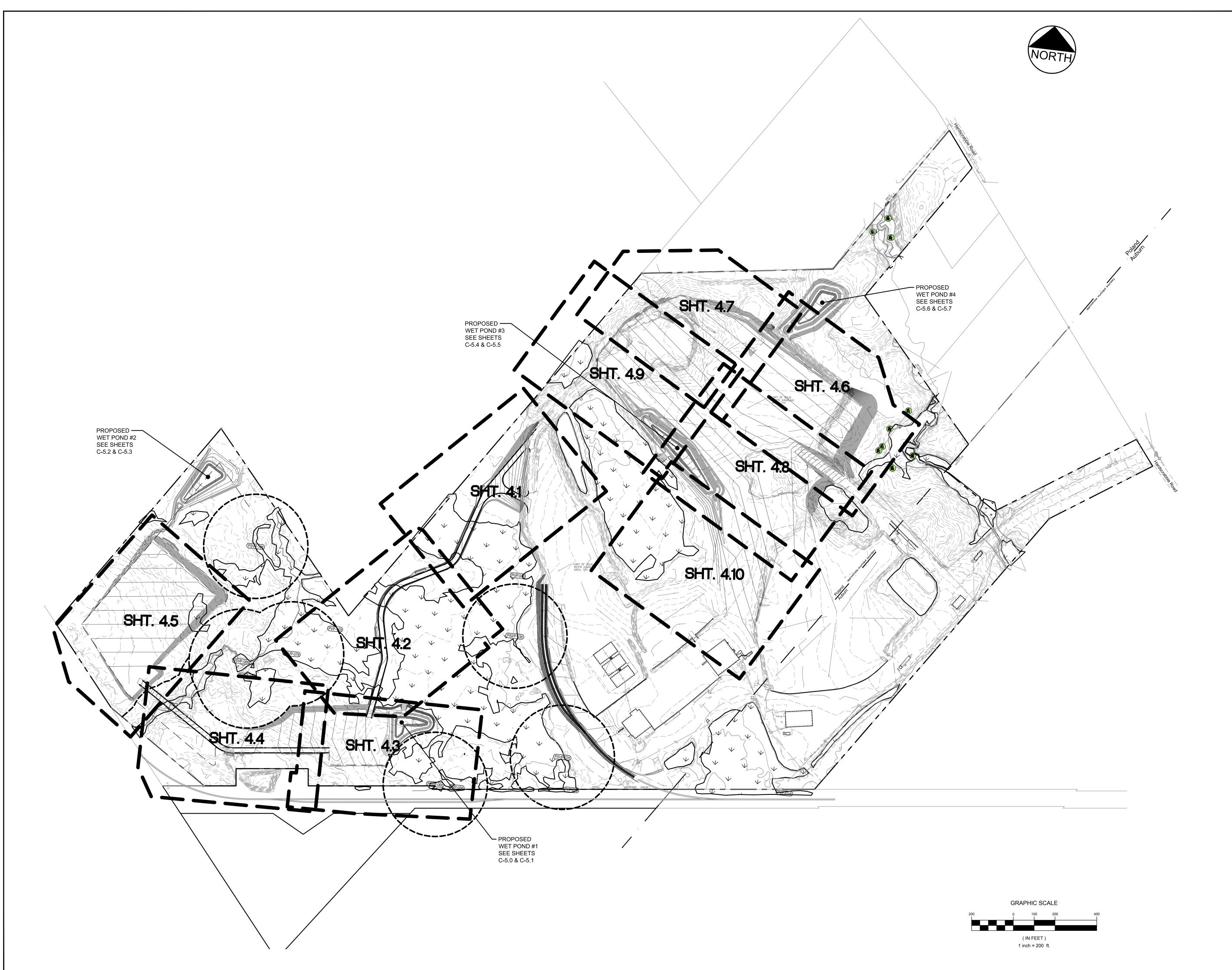
MB BARK RECYCLING FACILITY EXPANSION

AUBURN, MAINE / POLAND, MAINE

POST-CONSTRUCTION MATERIALS HANDLING PLAN

Project No. Scale 195350465 1" = 200'

C-3.2





482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

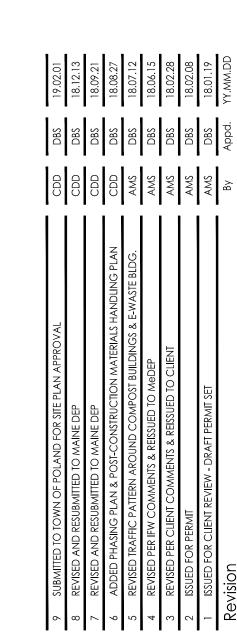
The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

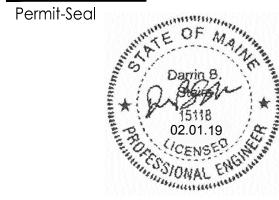
Consultants

846 Main Street Westbrook, Maine 04092

Tel. 207.591.7000



File Name: 50465_grading



Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY **EXPANSION**

AUBURN, MAINE / POLAND, MAINE

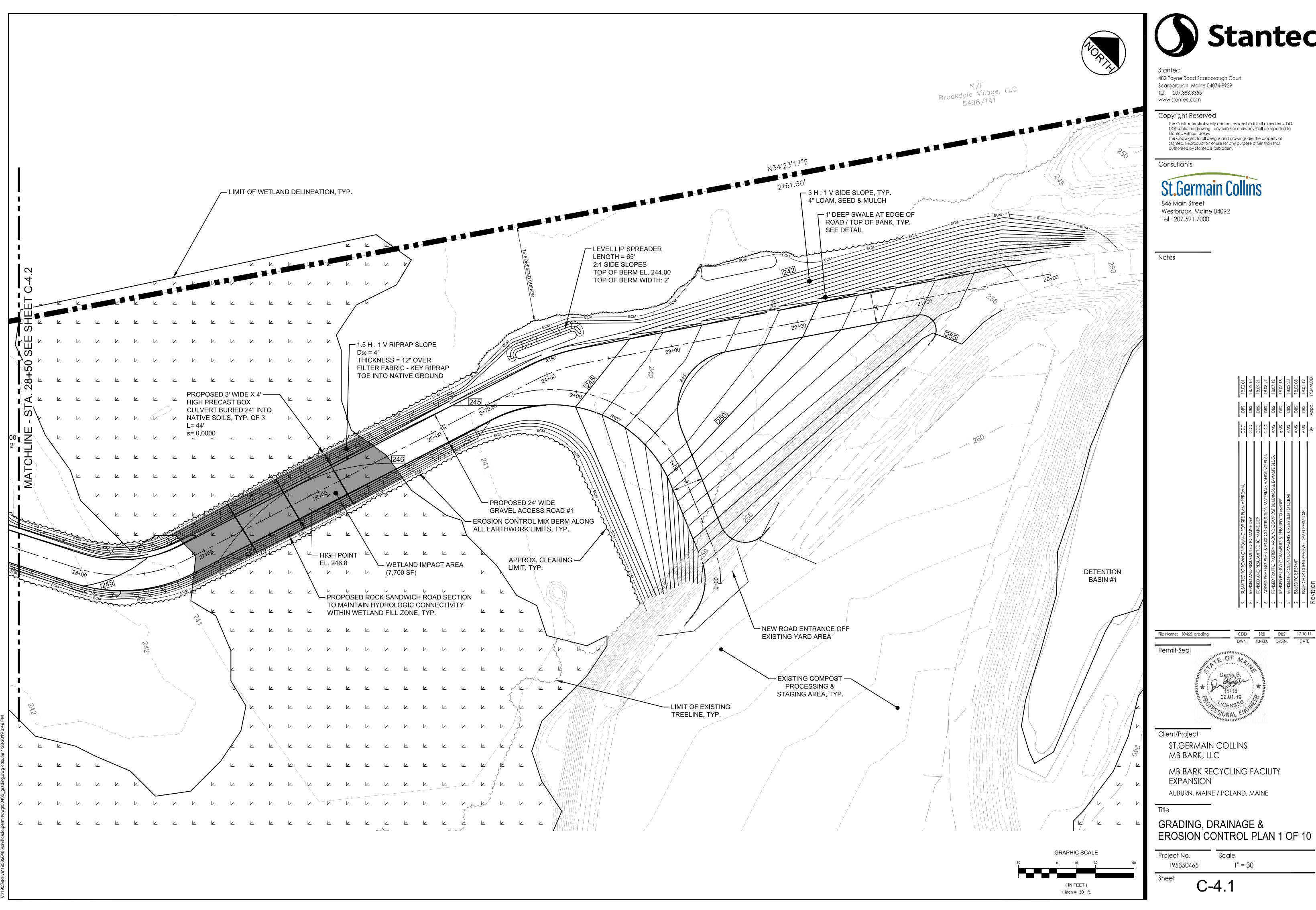
OVERALL GRADING PLAN -**INDEX SHEET**

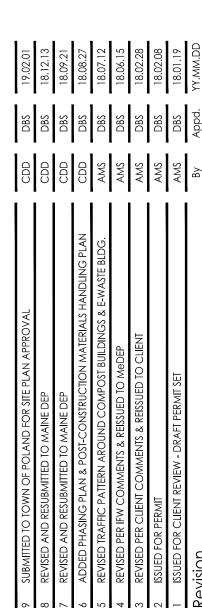
Project No. Scale

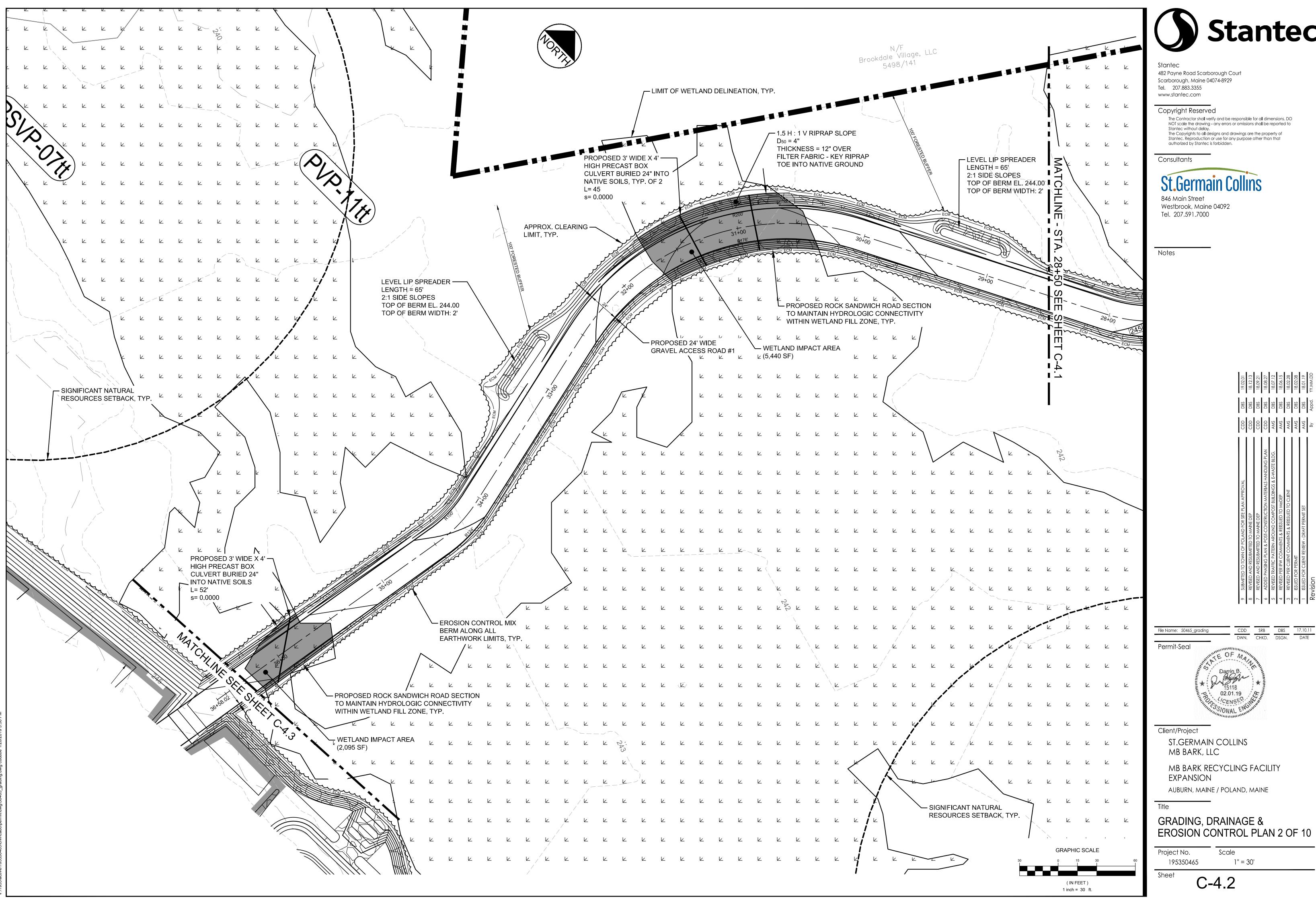
Sheet

195350465 1" = 200'

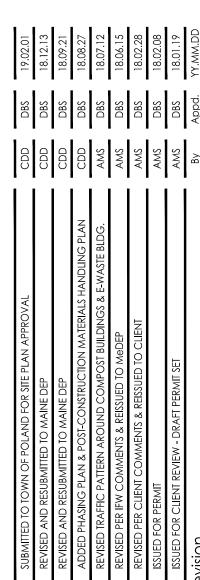
C-4.0

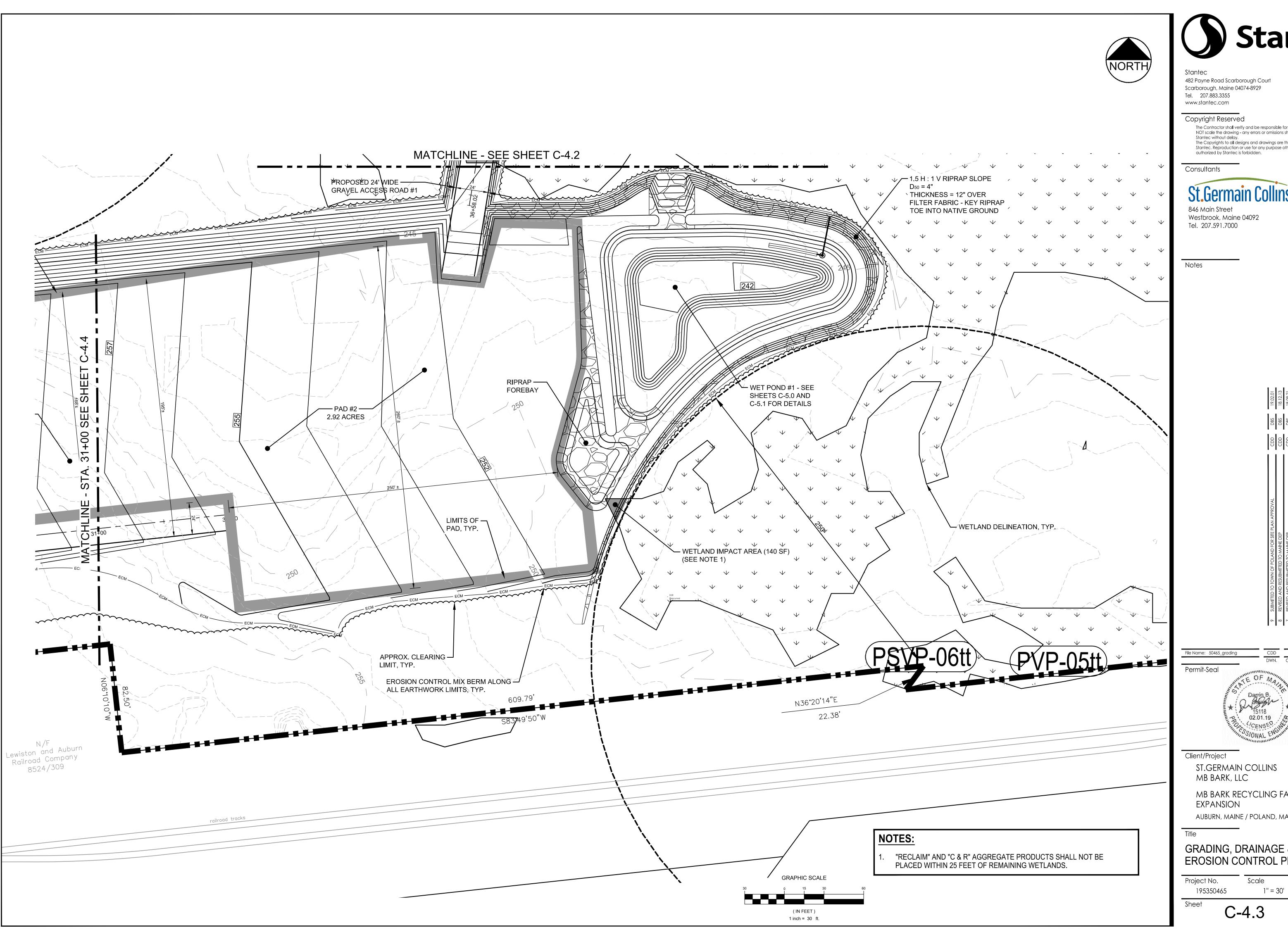








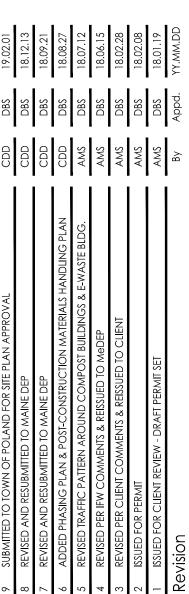






The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

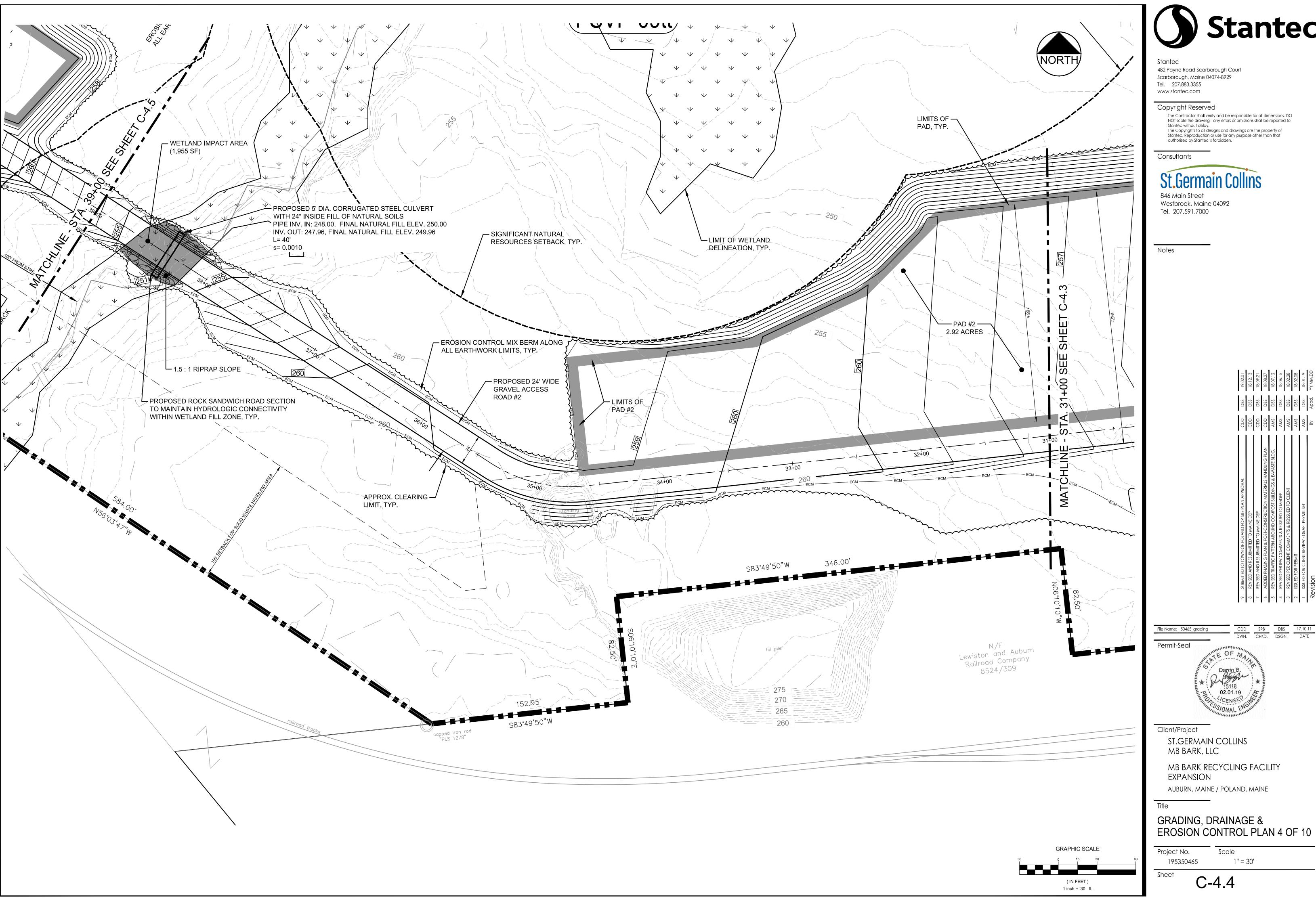




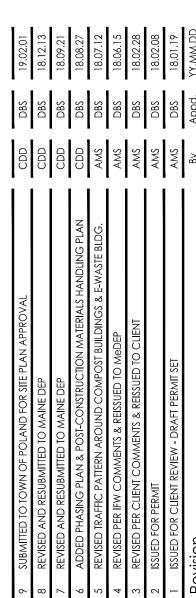
MB BARK RECYCLING FACILITY

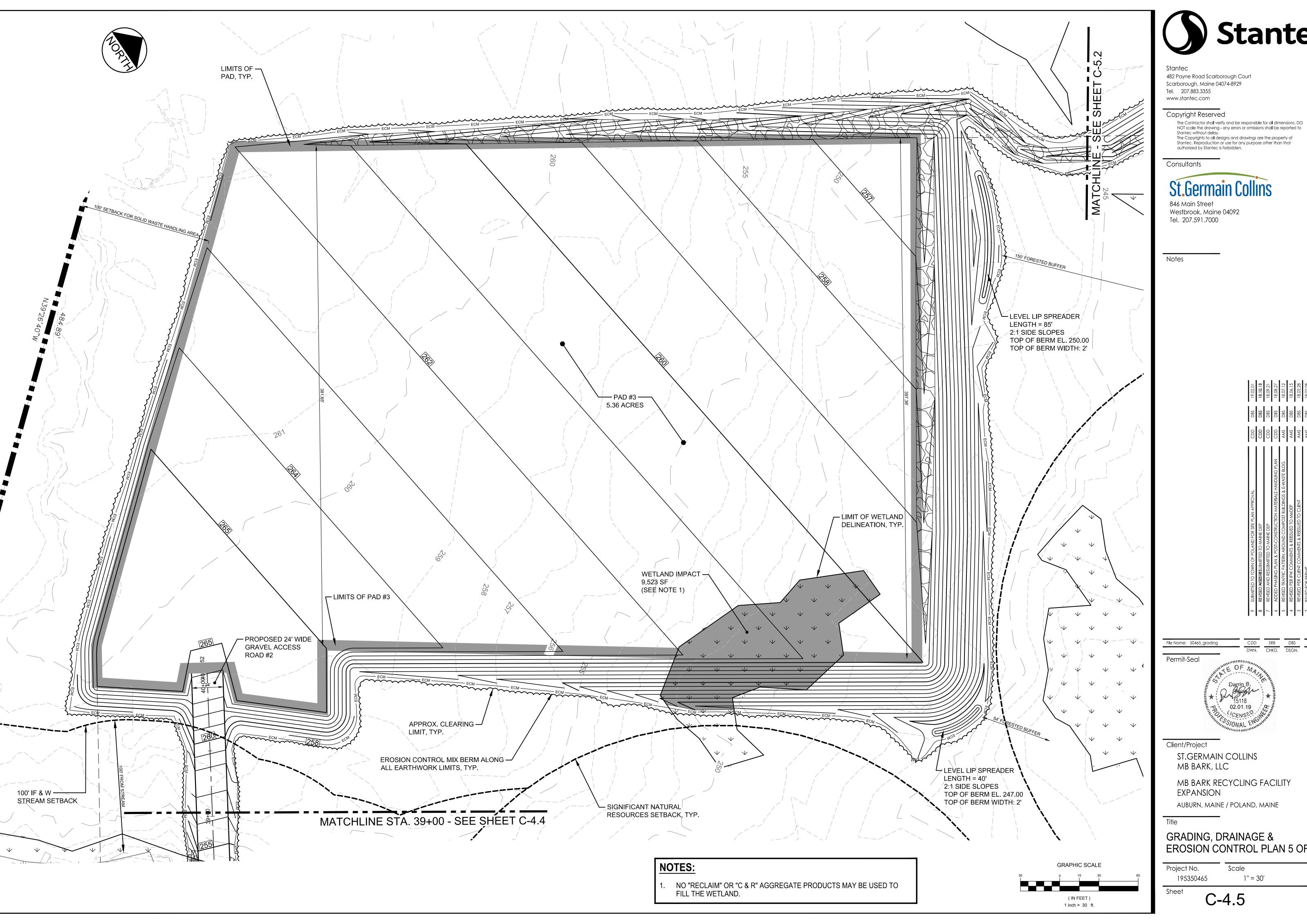
AUBURN, MAINE / POLAND, MAINE

GRADING, DRAINAGE & EROSION CONTROL PLAN 3 OF 10

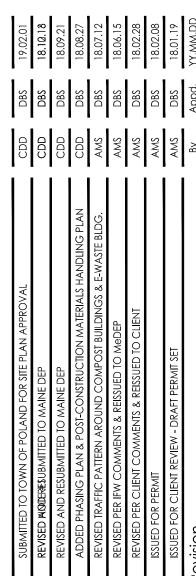








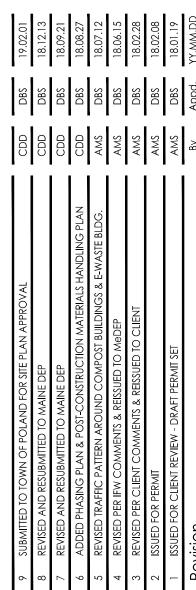


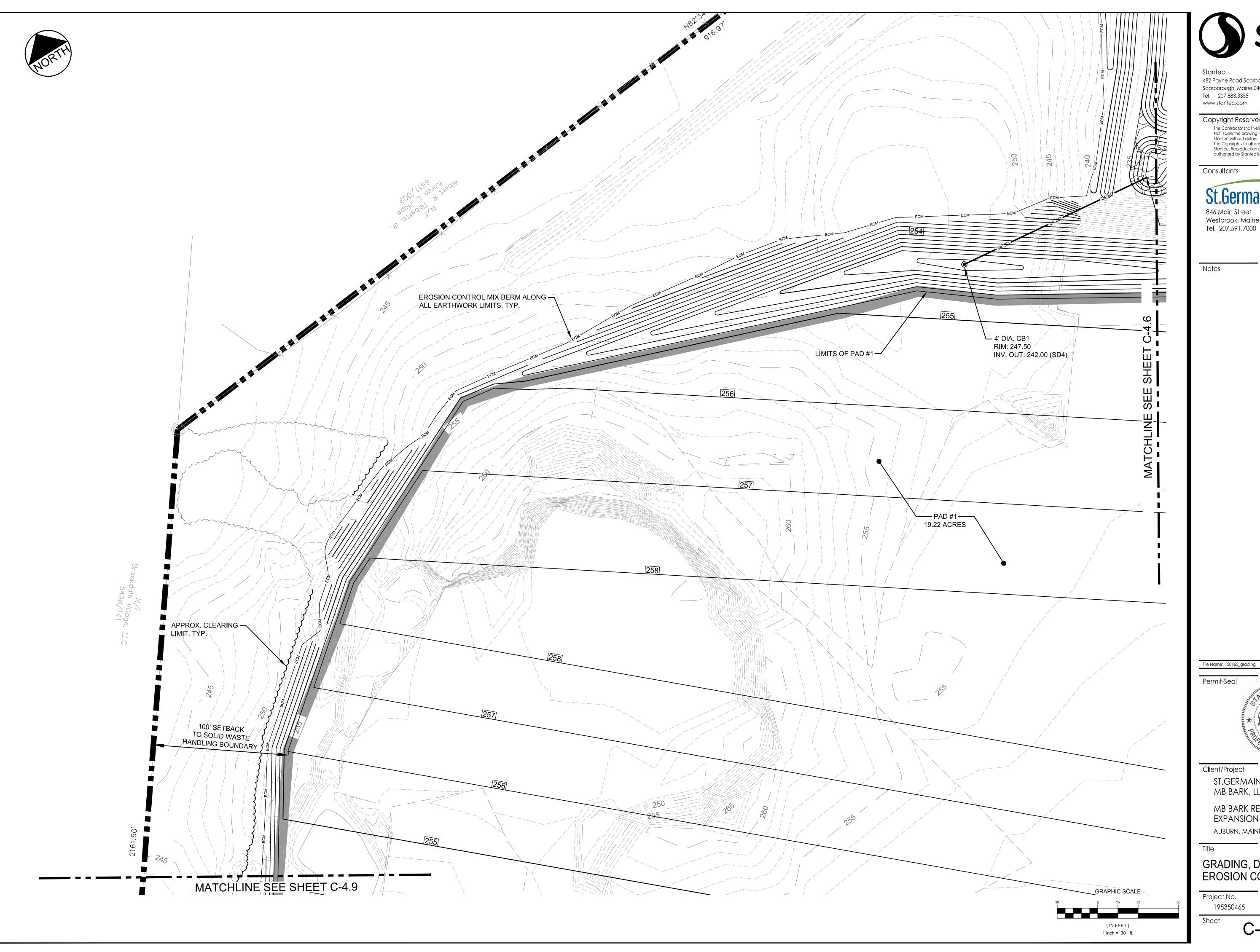


EROSION CONTROL PLAN 5 OF 10











482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355

Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

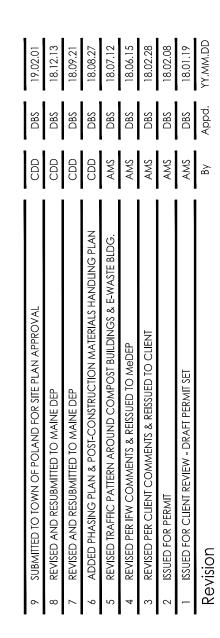
The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

Consultants

St.Germain Collins

846 Main Street Westbrook, Maine 04092

Notes



File Name: 50465_grading

Client/Project ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY **EXPANSION**

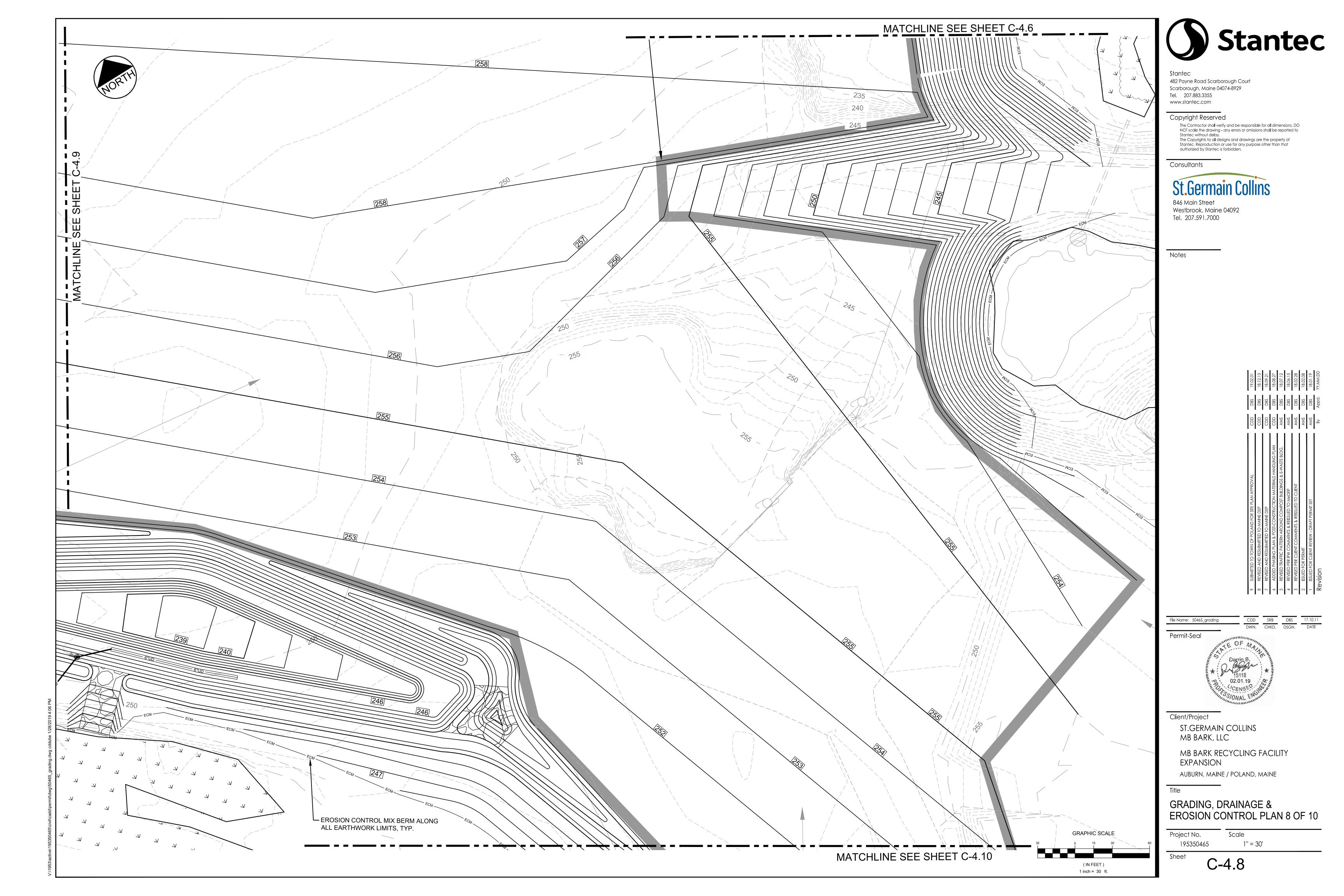
AUBURN, MAINE / POLAND, MAINE

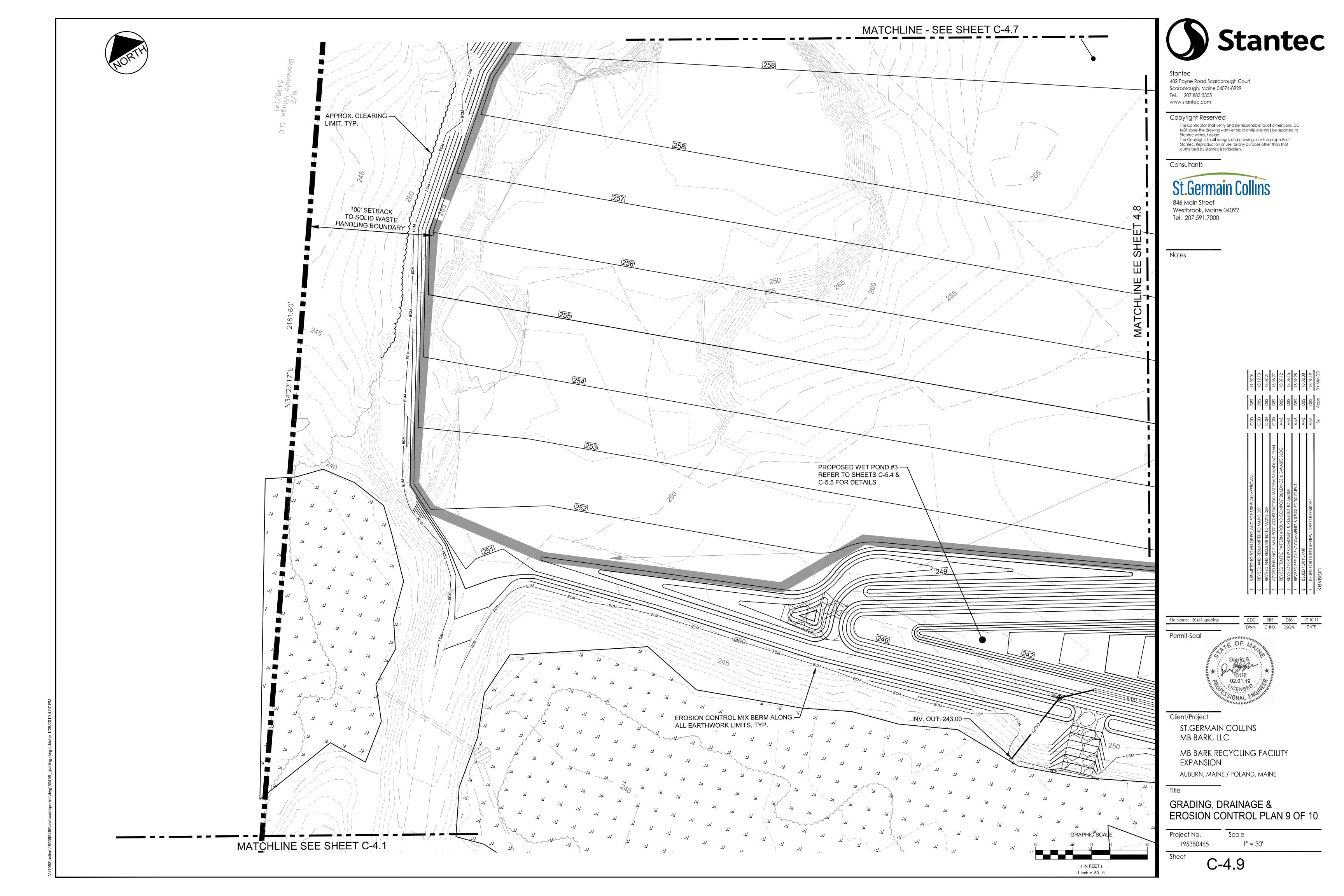
GRADING, DRAINAGE & EROSION CONTROL PLAN 7 OF 10

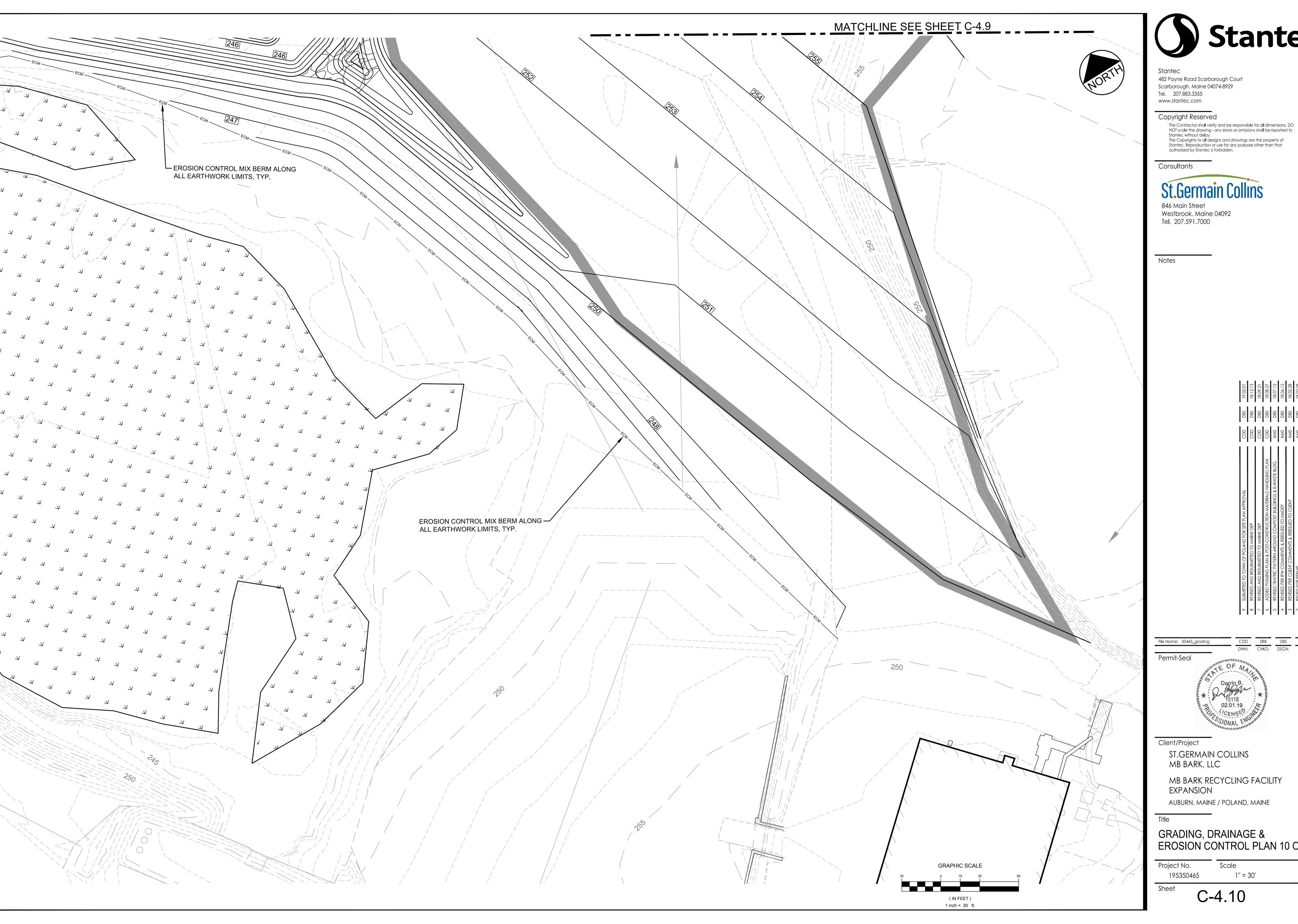
Project No. Scale 195350465

1" = 30'

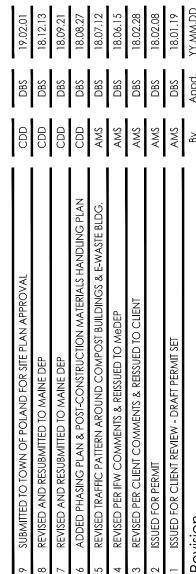
C-4.7







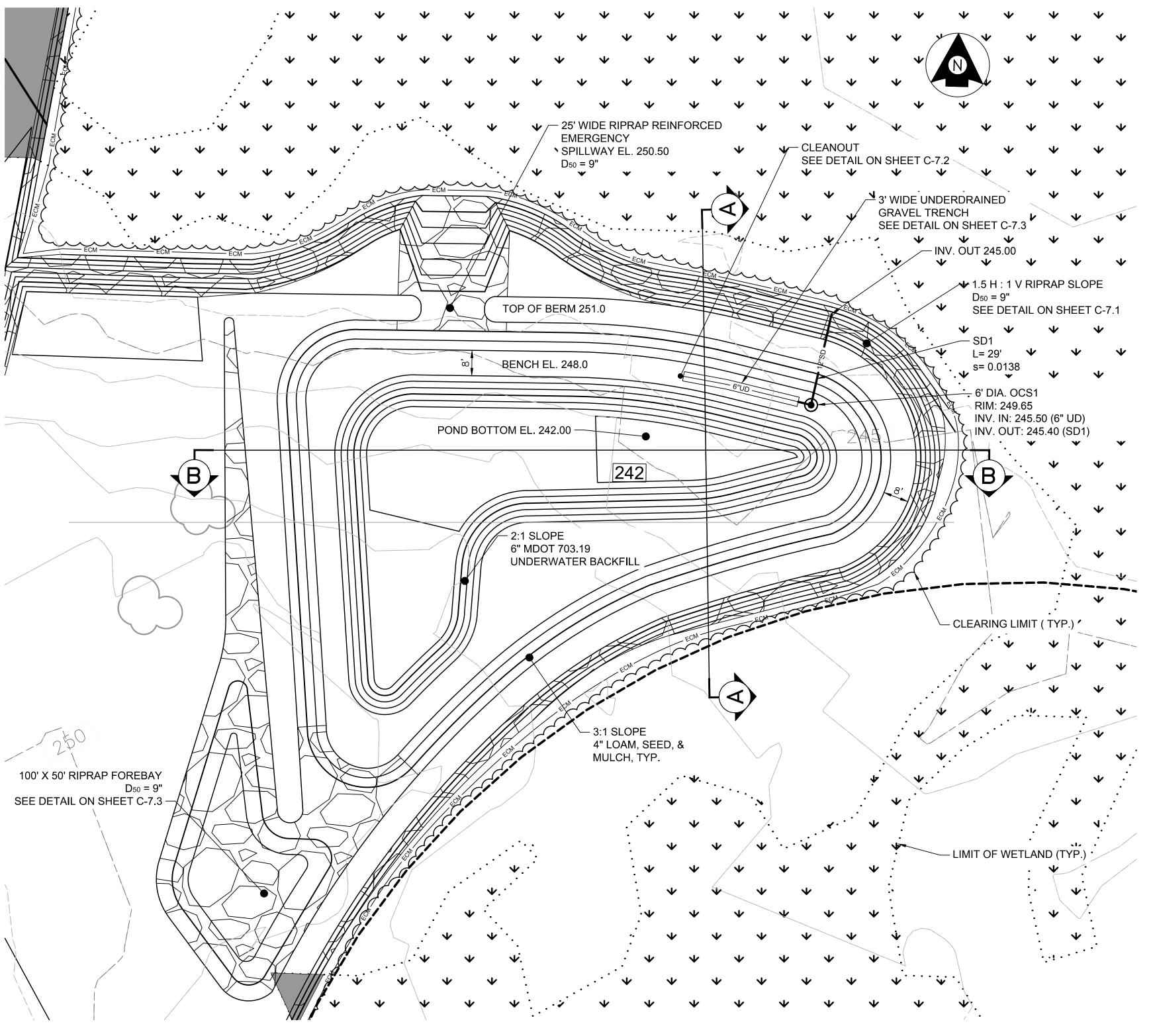




MB BARK RECYCLING FACILITY

AUBURN, MAINE / POLAND, MAINE

EROSION CONTROL PLAN 10 OF 10



WET POND #1 PLAN VIEW

SCALE: 1" = 20'

Wet Pond #1 Desig	n Criteria	
Tributary Impervious Area	119,736	sf
Tributary Landscaped Area	28,185	sf
Required Permanent Pool Volume	21,835	cf
Required Channel Protection Volume	10,918	cf
Provided Permanent Pool Volume	33,566	cf
Provided Channel Protection Volume	10,918	cf
Length of Permanent Pool	211	ft
Average Width of Permanent Pool	41	ft
Length to Width Ratio	5.19	:1
UD Length Required (3' / 1000 CF of CPV)	32.75	ft
UD Length Provided	37	ft



Stantec

482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

authorized by Stantec is forbidden.

Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

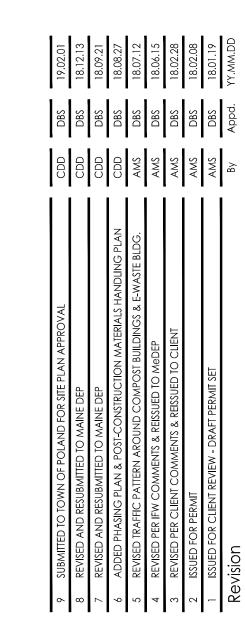
Consultants

St. Germain Collins

846 Main Street

Westbrook, Maine 04092 Tel. 207.591.7000

Notes



File Name: 50465_pond_sections

DWN. CHKD. DSG
Permit-Seal



Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION

AUBURN, MAINE / POLAND, MAINE

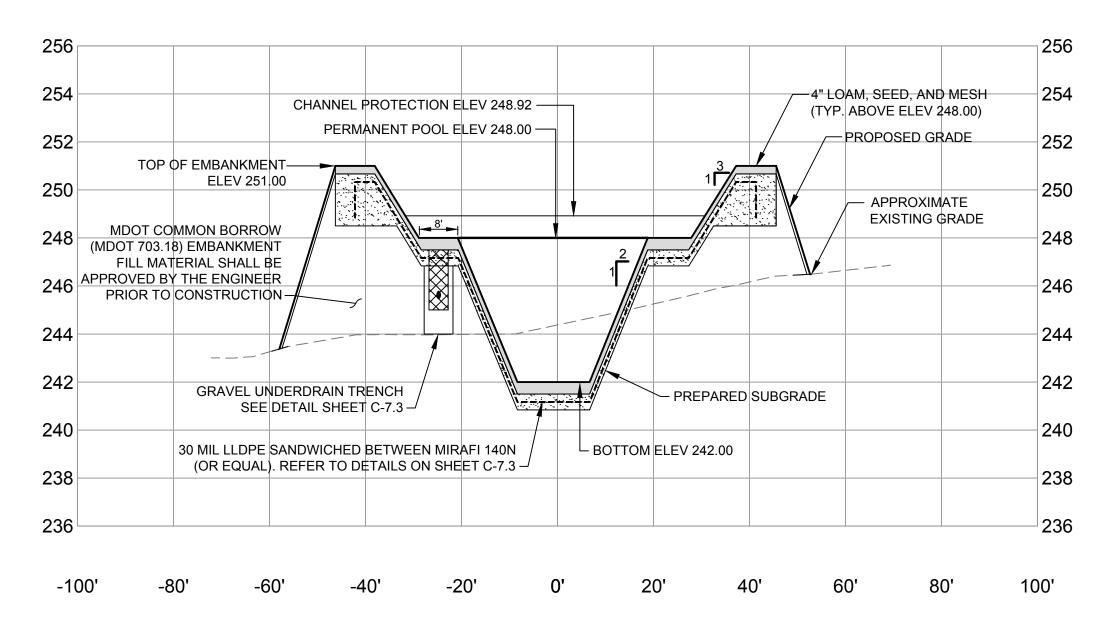
Title

WET POND #1 PLAN VIEW

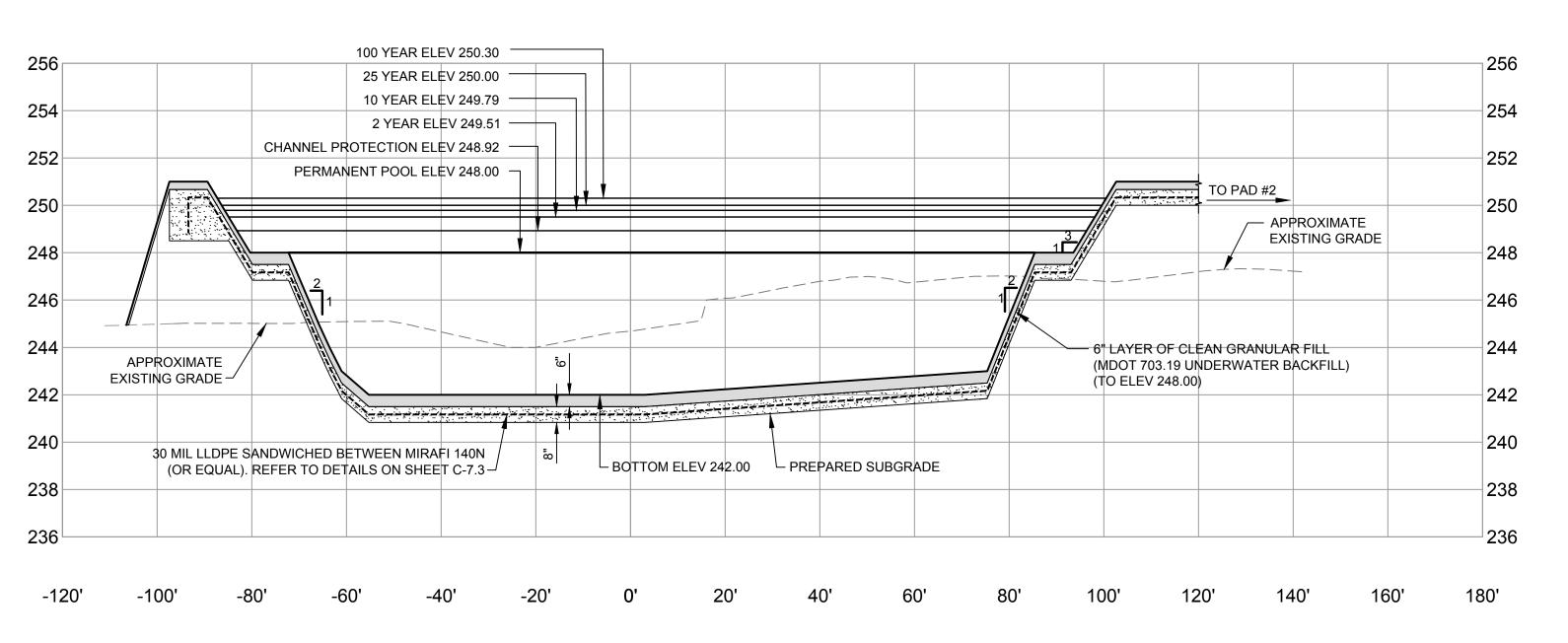
Project No. Scale
195350465 AS NOTED

Sheet

C-5.0



WET POND #1 SECTION A-A SCALE: 1" = 20' H 1" = 4' V



WET POND #1 SECTION B-B SCALE: 1" = 20' H

1" = 4' V



482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay. The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

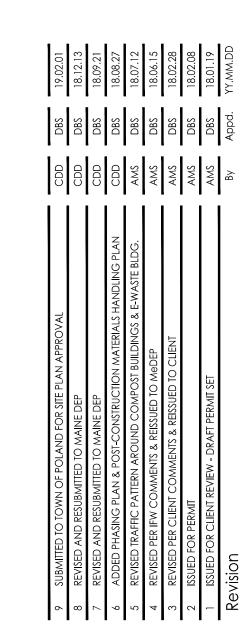
Consultants

authorized by Stantec is forbidden.

846 Main Street

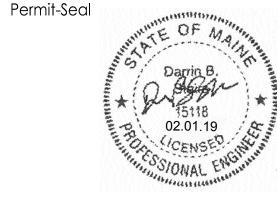
Westbrook, Maine 04092 Tel. 207.591.7000

Notes



File Name: 50465_pond_sections CDD SRB DBS 17.10.11

DWN. CHKD. DSGN. DATE



Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION AUBURN, MAINE / POLAND, MAINE

WET POND #1 SECTIONS

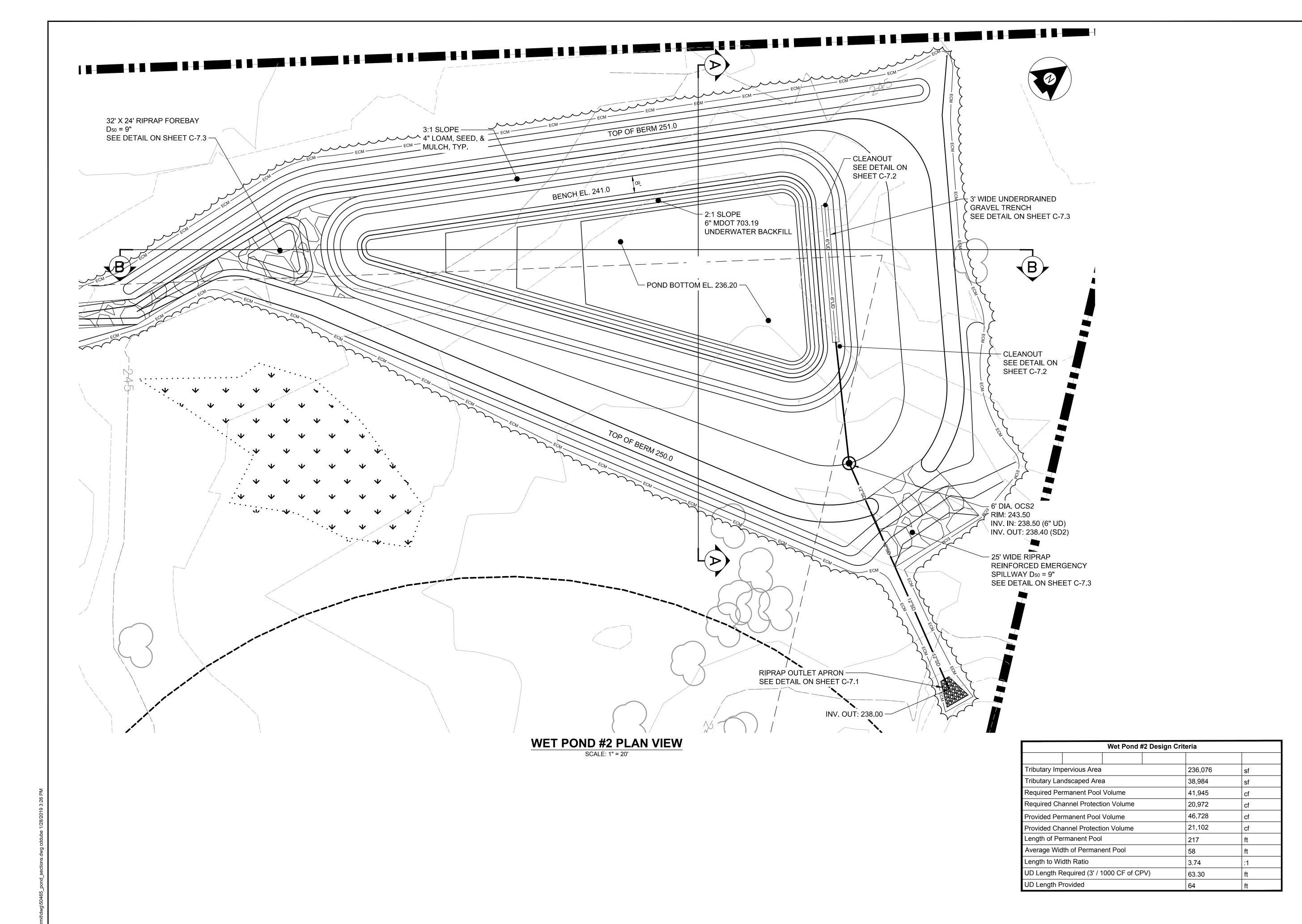
Scale

Project No. 195350465

Sheet

C-5.1

AS NOTED





Stanted

482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

Consultants

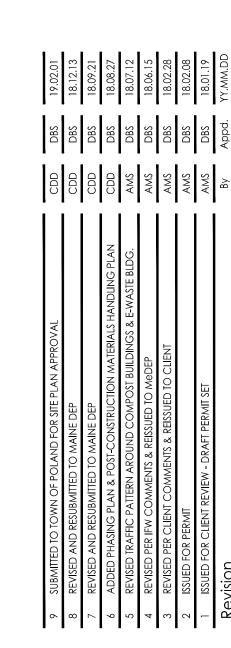
St. Germain Collins

authorized by Stantec is forbidden.

846 Main Street Westbrook, Maine 04092

Tel. 207.591.7000

Notes



File Name: 50465_pond_sections CDD

DWN. CHKD. DSGN. DA

Darrin B.

Darrin B.

15118
02.01.19
CENSED

Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

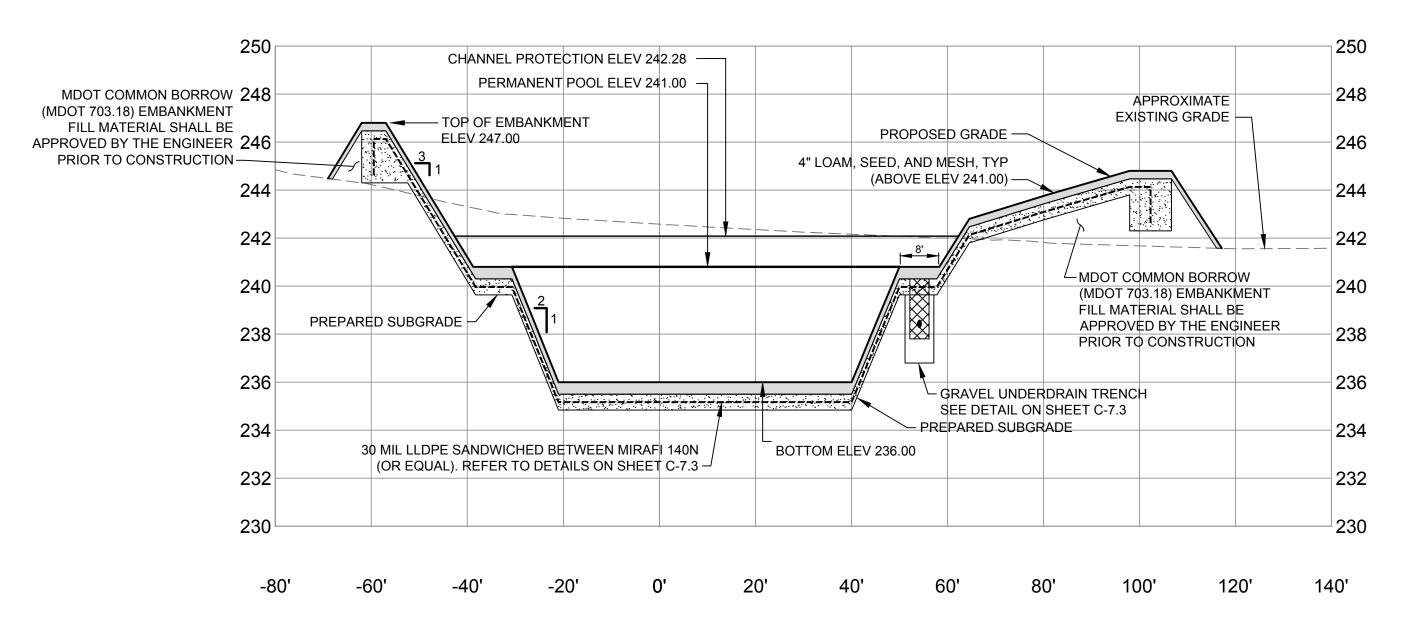
MB BARK RECYCLING FACILITY EXPANSION

AUBURN, MAINE / POLAND, MAINE

Title

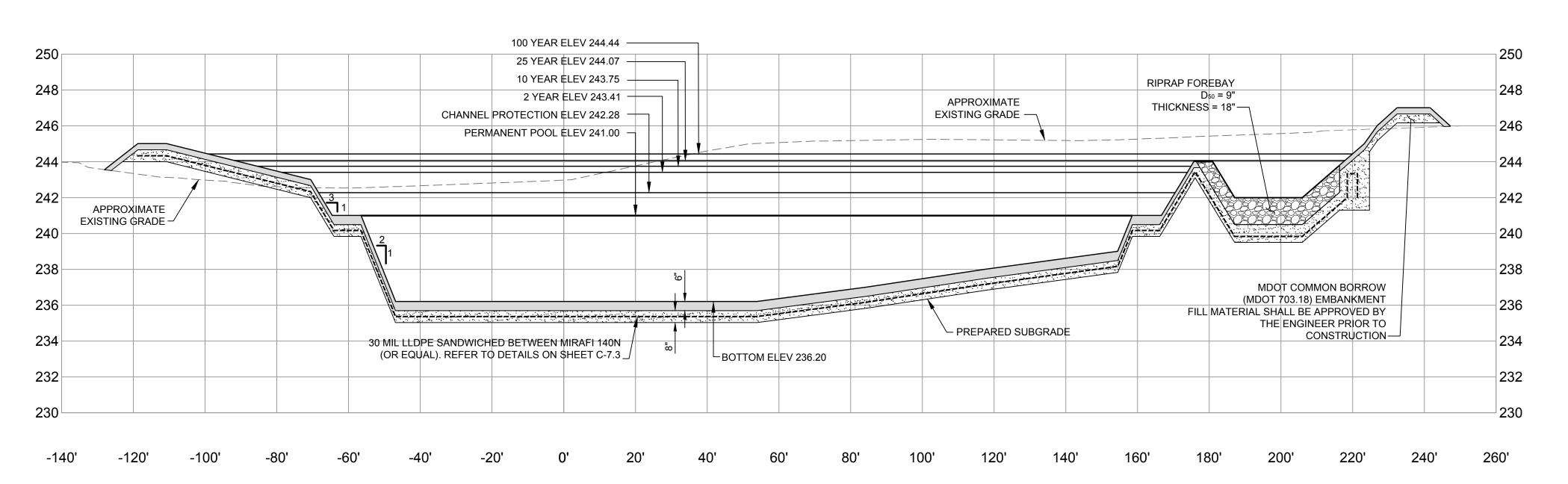
WET POND #2 PLAN VIEW

Project No. Scale
195350465 AS NOTED
Sheet



WET POND #2 SECTION A-A SCALE: 1" = 20' H

1" = 4' V



WET POND #2 SECTION B-B SCALE: 1" = 20' H 1" = 4' V



482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

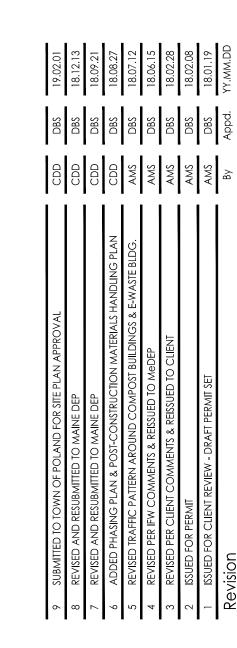
The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay. The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

Consultants

authorized by Stantec is forbidden.

846 Main Street Westbrook, Maine 04092 Tel. 207.591.7000

Notes



 File Name:
 50465_pond_sections
 CDD
 SRB
 DBS
 17.10.11

 DWN.
 CHKD.
 DSGN.
 DATE

Permit-Seal



Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

> MB BARK RECYCLING FACILITY EXPANSION AUBURN, MAINE / POLAND, MAINE

WET POND #2 SECTIONS

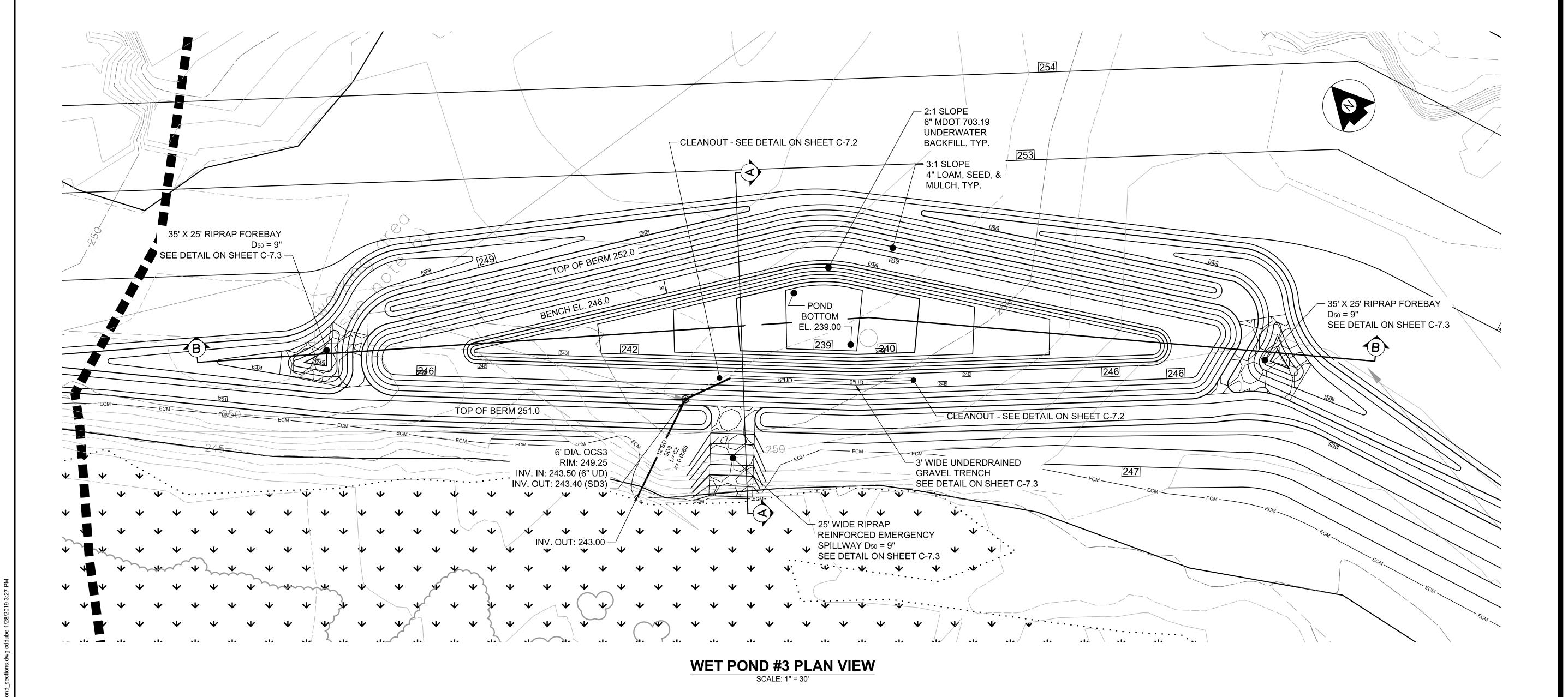
Scale

Project No. 195350465

Sheet

AS NOTED

Wet Pond #3 Design Criteria			
Tributary Im	pervious Area	360,000	sf
Tributary La	ndscaped Area	75,913	sf
Required Pe	rmanent Pool Volume	65,061	cf
Required Channel Protection Volume		32,530	cf
Provided Pe	rmanent Pool Volume	69,139	cf
Provided Ch	annel Protection Volume	32,692	cf
Length of Pe	ermanent Pool	236	ft
Average Wid	dth of Permanent Pool	72	ft
Length to W	idth Ratio	3.30	:1
UD Length F	Required (3' / 1000 CF of CPV)	98.08	ft
UD Length F	Provided	104	ft





482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

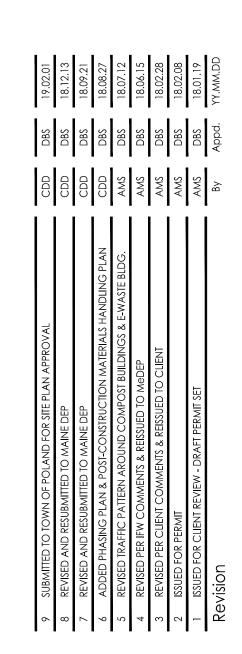
Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay. The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

Consultants

authorized by Stantec is forbidden.

846 Main Street Westbrook, Maine 04092 Tel. 207.591.7000



File Name: 50465_pond_sections

Permit-Seal

ST.GERMAIN COLLINS

Client/Project

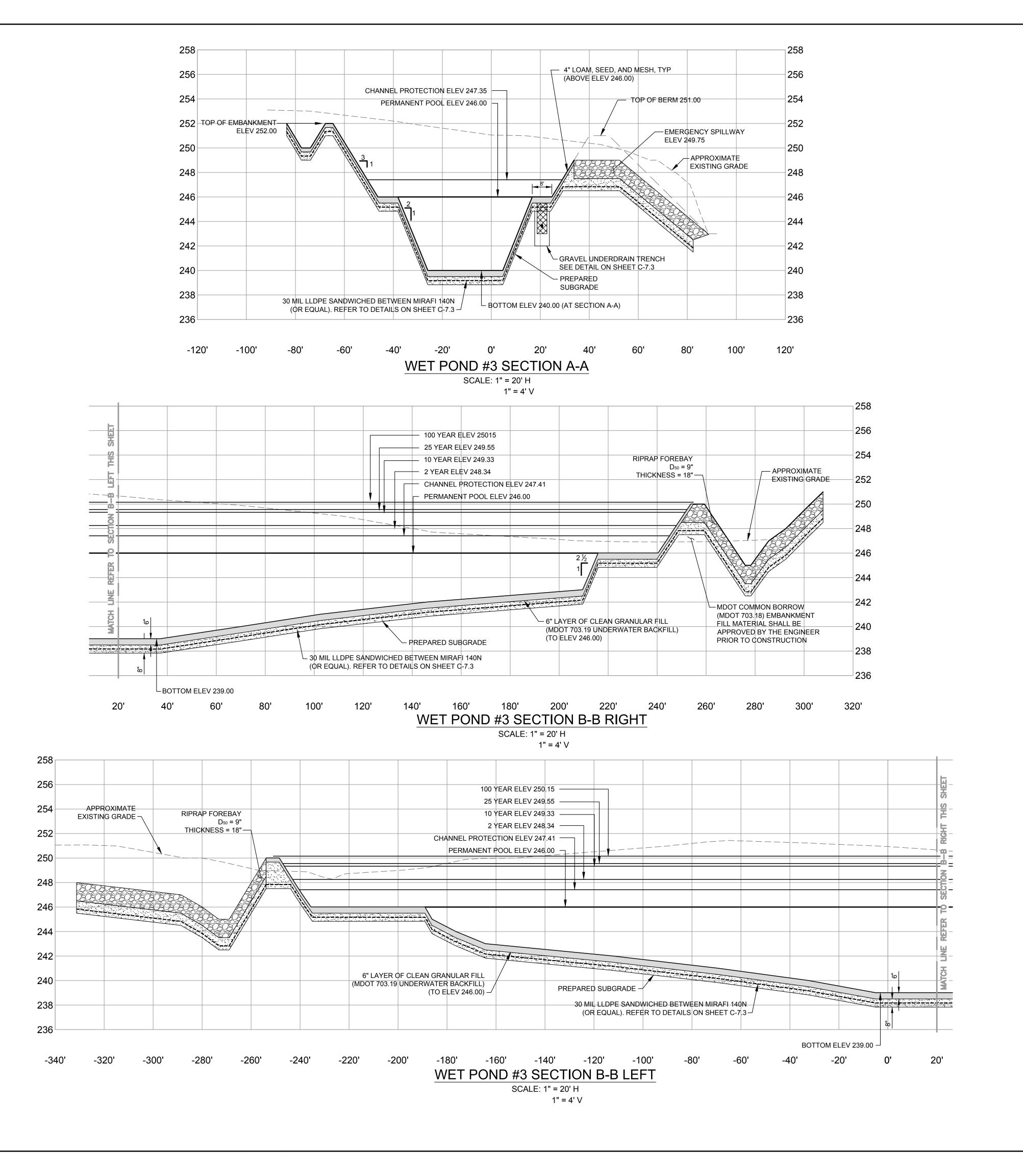
MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION AUBURN, MAINE / POLAND, MAINE

Sheet

WET POND #3 PLAN VIEW

Scale Project No. 195350465 **AS NOTED**





Stanted

482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

Consultants

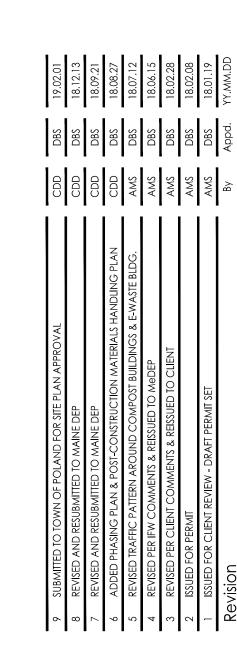
St.Germain Collins

846 Main Street

Westbrook, Maine 04092 Tel. 207.591.7000

authorized by Stantec is forbidden.

Notes



File Name: 50465_pond_sections

nd_sections CDD SRB DBS 17.1

DWN. CHKD. DSGN. Date of the control

Permit-Seal



Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION

AUBURN, MAINE / POLAND, MAINE

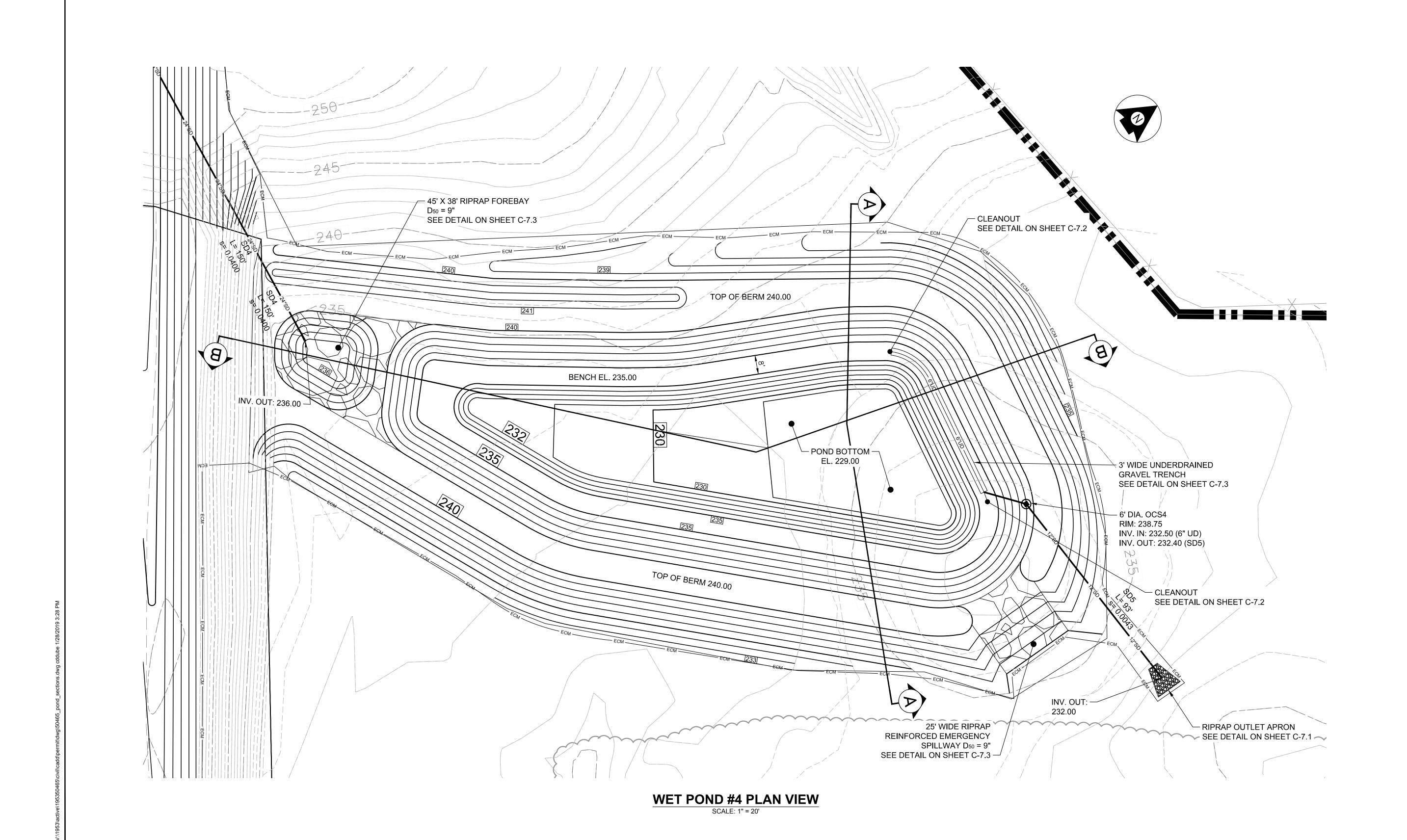
Title

WET POND #3 SECTIONS

Project No. Scale
195350465 AS NOTED

Sheet

Wet Pond #4 Design Criteria			
Tributary Im	pervious Area	275,000	sf
Tributary La	ndscaped Area	50,000	sf
Required Pe	rmanent Pool Volume	49,167	cf
Required Channel Protection Volume		24,583	cf
Provided Pe	rmanent Pool Volume	49,669	cf
Provided Ch	annel Protection Volume	24,643	cf
Length of Pe	rmanent Pool	244	ft
Average Wid	Ith of Permanent Pool	48	ft
Length to Wi	dth Ratio	5.06	:1
UD Length F	Required (3' / 1000 CF of CPV)	73.93	ft
UD Length F	Provided	74	ft





482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

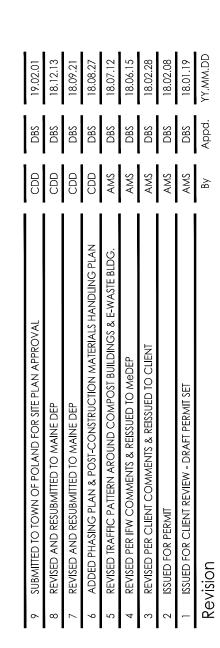
The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

Consultants

846 Main Street Westbrook, Maine 04092 Tel. 207.591.7000

Notes



File Name: 50465_pond_sections CDD SRB DBS 17.10.11

DWN. CHKD. DSGN. DATE

Client/Project

Permit-Seal

ST.GERMAIN COLLINS MB BARK, LLC

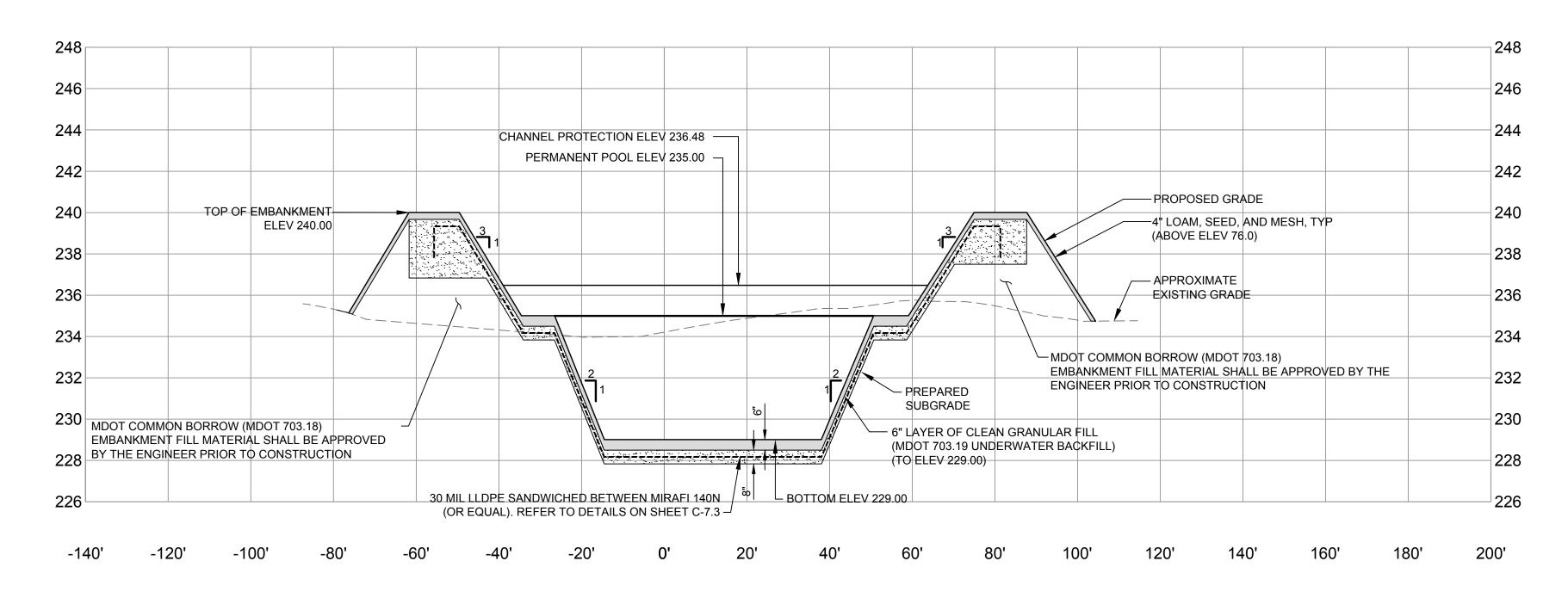
MB BARK RECYCLING FACILITY EXPANSION

AUBURN, MAINE / POLAND, MAINE

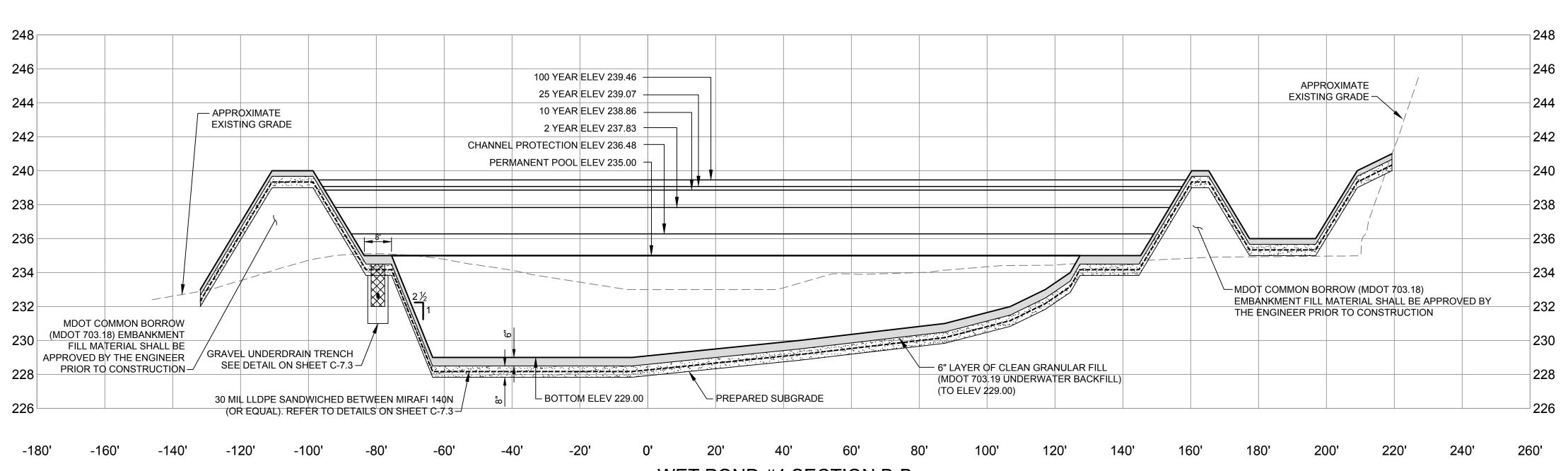
WET POND #4 PLAN VIEW

Project No. Scale AS NOTED 195350465

Sheet



WET POND #4 SECTION A-A SCALE: 1" = 20' H 1" = 4' V



WET POND #4 SECTION B-B SCALE: 1" = 20' H 1" = 4' V



482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

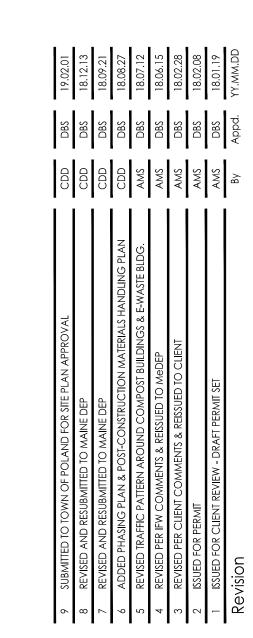
Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay. The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

Consultants

846 Main Street Westbrook, Maine 04092 Tel. 207.591.7000

Notes



Permit-Seal

 File Name:
 50465_pond_sections
 CDD
 SRB
 DBS
 17.10.11

 DWN.
 CHKD.
 DSGN.
 DATE



Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION AUBURN, MAINE / POLAND, MAINE

WET POND #4 SECTIONS

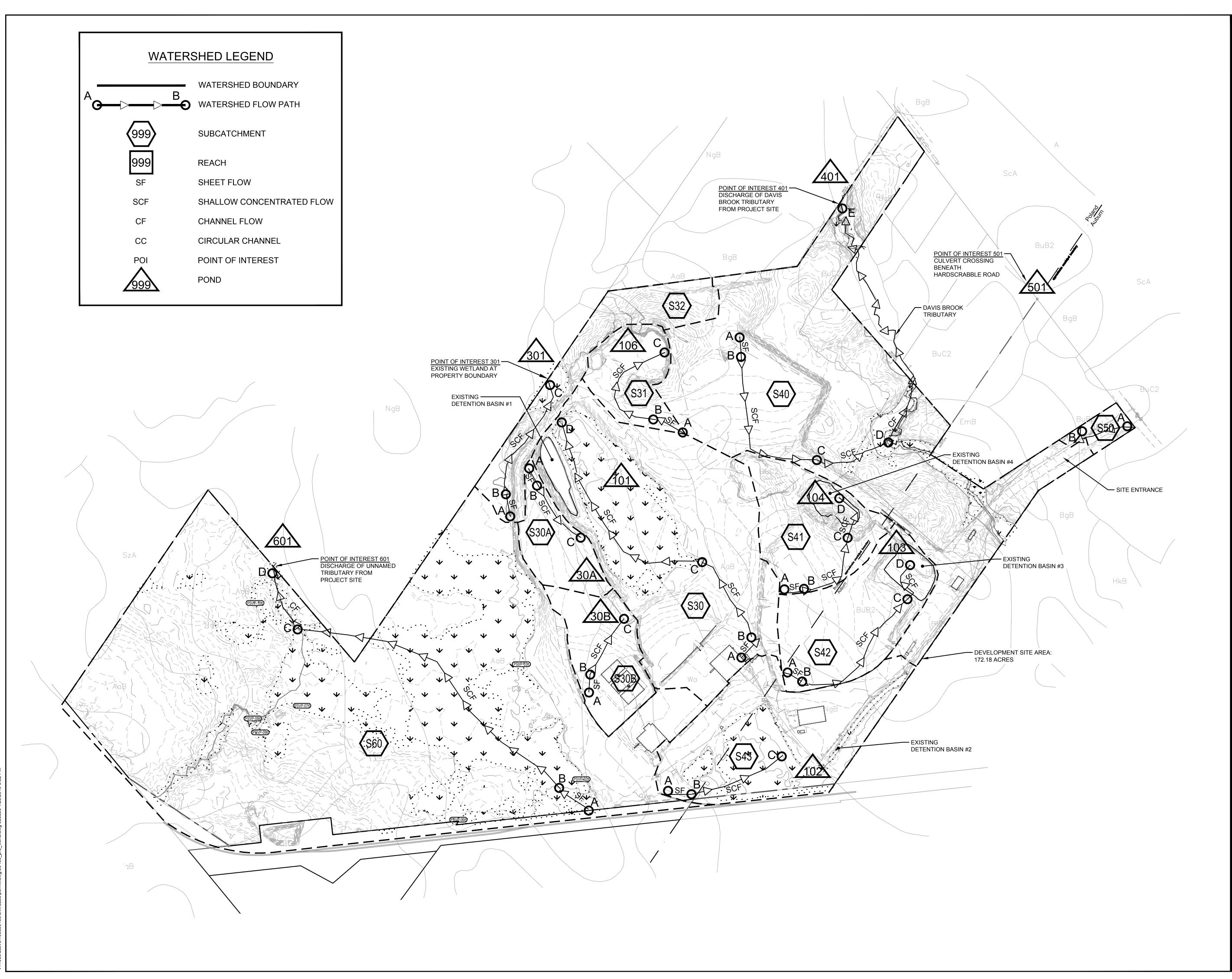
Project No. Scale

Sheet

195350465

C-5.7

as noted





482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355

Copyright Reserved

www.stantec.com

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

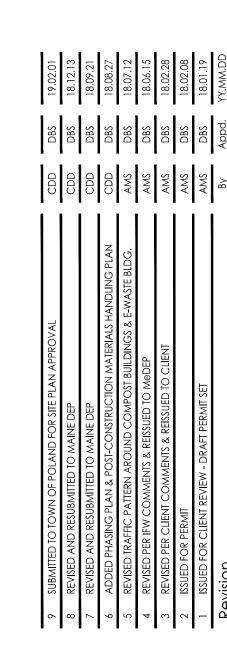
The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

Consultants

846 Main Street

Westbrook, Maine 04092 Tel. 207.591.7000

Notes



Permit-Seal



Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION

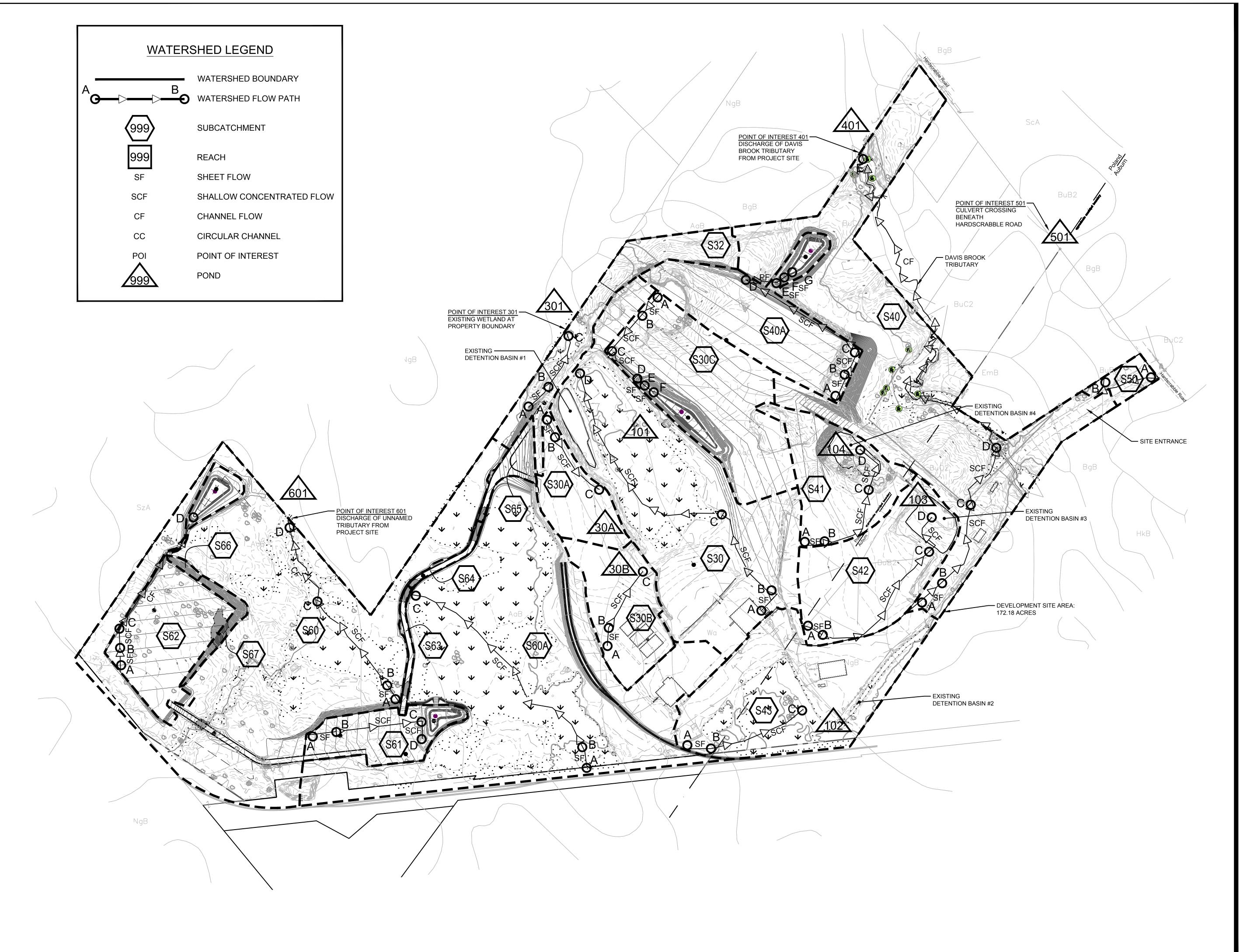
AUBURN, MAINE / POLAND, MAINE

PREDEVELOPMENT WATERSHED PLAN

Project No. Scale

195350465 1" = 200'

C-6.0





482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355

Copyright Reserved

www.stantec.com

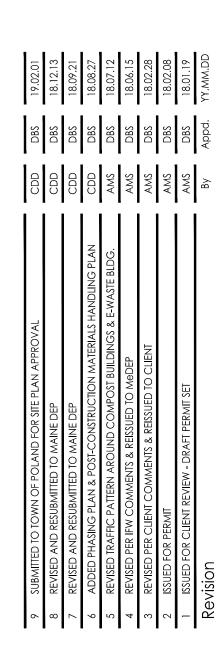
The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

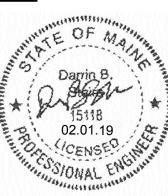
Consultants

846 Main Street Westbrook, Maine 04092

Tel. 207.591.7000



Permit-Seal



Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION

AUBURN, MAINE / POLAND, MAINE

POST DEVELOPMENT WATERSHED PLAN

Project No. Scale

195350465 1" = 200'

C-6.1

II.4 EROSION CONTROL MEASURES AND SITE STABILIZATION

THE PROPOSED EROSION AND SEDIMENTATION CONTROL PLAN MEETS OR EXCEEDS THE SOIL CONSERVATION SERVICE

THE PRIMARY EMPHASES OF THE EROSION/SEDIMENTATION CONTROL PLAN TO BE IMPLEMENTED FOR THE INFRASTRUCTURE CONSTRUCTION ARE AS FOLLOWS:

- DEVELOPMENT OF A CAREFUL CONSTRUCTION SEQUENCE.
- RAPID REVEGETATION OF DENUDED AREAS TO MINIMIZE THE PERIOD OF SOIL EXPOSURE.
- RAPID STABILIZATION OF DRAINAGE PATHS TO AVOID RILL AND GULLY EROSION. • THE USE OF ON-SITE MEASURES TO CAPTURE SEDIMENT (STONE CHECK DAMS/HAY BALES/SILT FENCE, ETC.)

THE FOLLOWING TEMPORARY AND PERMANENT EROSION AND SEDIMENT CONTROL DEVICES WILL BE IMPLEMENTED AS PART OF THE SITE DEVELOPMENT. THESE DEVICES SHALL BE INSTALLED AS INDICATED ON THE PLANS OR AS DESCRIBED WITHIN THIS REPORT. FOR FURTHER REFERENCE, SEE THE MAINE EROSION AND SEDIMENT CONTROL HANDBOOK FOR CONSTRUCTION: BEST MANAGEMENT PRACTICES.

A. TEMPORARY EROSION CONTROL MEASURES

THE FOLLOWING MEASURES ARE PLANNED AS TEMPORARY EROSION/SEDIMENTATION CONTROL MEASURES DURING

- 1. A TEMPORARY CRUSHED STONE-STABILIZED CONSTRUCTION ENTRANCE SHALL BE PLACED AT ANY ACCESS ROUTES OUT OF THE CONSTRUCTION ZONE.
- 2. SILTATION FENCE OR WOOD WASTE COMPOST BERMS SHALL BE INSTALLED DOWNGRADIENT OF ANY DISTURBED AREAS TO TRAP RUNOFF BORNE SEDIMENTS UNTIL THE SITE IS FULLY STABILIZED. THE SILT FENCE AND/OR THE WOOD WASTE COMPOST BERMS SHALL BE INSTALLED PER THE DETAILS PROVIDED IN THIS PACKAGE AND INSPECTED AT LEAST ONCE A WEEK AND BEFORE AND AFTER A STORM EVENT. REPAIRS SHALL BE MADE IF THERE ARE ANY SIGNS OF EROSION OR SEDIMENTATION BELOW THE FENCE OR BERM LINE. IF THERE ARE SIGNS OF UNDERCUTTING AT THE CENTER OR THE EDGES, OR IMPOUNDING OF LARGE VOLUMES OF WATER BEHIND FENCE OR BERM, THE BARRIER SHALL BE REPLACED WITH A STONE CHECK DAM.
- 3. STRAW OR HAY MULCH INCLUDING HYDROSEEDING IS INTENDED TO PROVIDE COVER FOR DENUDED OR SEEDED AREAS UNTIL REVEGETATION IS ESTABLISHED AND SHOULD BE APPLIED AT A RATE A 115 LBS / 1000 S.F. MULCH PLACED BETWEEN APRIL 15TH AND OCTOBER 15TH ON SLOPES OF LESS THEN 15 PERCENT SHALL BE ANCHORED BY APPLYING WATER; MULCH PLACED ON SLOPES OF EQUAL TO OR STEEPER THAN 15 PERCENT SHALL BE COVERED BY A FABRIC NETTING AND ANCHORED WITH STAPLES IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATION. MULCH PLACED BETWEEN OCTOBER 15TH AND APRIL 15TH ON SLOPES EQUAL TO OR STEEPER THAN 8 PERCENT SHALL BE COVERED WITH A FABRIC NETTING AND ANCHORED WITH STAPLES IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SLOPES STEEPER THAN 3:1. WHICH ARE TO BE REVEGETATED. SHALL RECEIVE CURLEX BLANKETS BY AMERICAN EXCELSIOR OR EQUAL. MULCH APPLICATION RATES ARE PROVIDED IN ATTACHMENT A OF THIS SECTION. MULCH SHALL NOT BE PLACED OVER SNOW.
- 4. TEMPORARY STOCKPILES OF STUMPS, GRUBBINGS, OR COMMON EXCAVATION WILL BE PROTECTED AS FOLLOWS:
- a. TEMPORARY STOCKPILES SHALL NOT BE LOCATED WITHIN 50 FEET OF ANY WETLANDS, WHICH WILL NOT BE DISTURBED AND ANY SLOPES WHICH EXCEED 15%.
- b. STOCKPILES SHALL BE STABILIZED WITHIN 7 DAYS BY EITHER TEMPORARILY SEEDING THE STOCKPILE BY A HYDROSEED METHOD CONTAINING AN EMULSIFIED MULCH TACKIFIER OR BY COVERING THE STOCKPILE WITH
- c. STOCKPILES SHALL BE SURROUNDED BY SILT FENCE OR OTHER SEDIMENT BARIER AT THE TIME OF FORMATION.
- 5. FOR WORK, WHICH IS CONDUCTED BETWEEN OCTOBER 15 AND APRIL 15 OF ANY CALENDAR YEAR, ALL DENUDED AREAS WILL BE COVERED WITH HAY MULCH, APPLIED AT TWICE THE NORMAL APPLICATION RATE AND ANCHORED WITH FABRIC NETTING. THE TIME PERIOD FOR APPLYING MULCH AS NOTED IN PARAGRAPH II.4.A.3, SHALL BE LIMITED TO 7 DAYS FOR ALL AREAS
- 6. DURING GRUBBING OPERATIONS, STONE CHECK DAMS SHALL BE INSTALLED AT ANY EVIDENT CONCENTRATED FLOW
- 7. SILT FENCING WITH A MINIMUM STAKE SPACING OF 6 FEET SHOULD BE USED, UNLESS THE FENCE IS SUPPORTED BY WIRE FENCE REINFORCEMENT OF MINIMUM 14 GAUGE AND WITH A MAXIMUM MESH SPACING OF 6 INCHES, IN WHICH CASE STAKES MAY BE SPACED A MAXIMUM OF 10 FEET APART. THE BOTTOM OF THE FENCE SHALL BE ANCHORED.
- 8. WOOD WASTE COMPOST/BARK BERMS MAY BE USED IN LIEU OF SILTATION FENCING. BERMS SHALL BE REMOVED AND SPREAD IN A LAYER NOT TO EXCEED 3" THICK ONCE UPGRADIENT AREAS ARE COMPLETED AND A 90% CATCH OF
- 9. TEMPORARY CATCH BASIN INLET BARRIERS SHALL BE INSTALLED THROUGHOUT THE COURSE OF CONSTRUCTION. ACCEPTABLE BARRIERS INCLUDE SILKSACKS, STONE SEDIMENT BARRIERS OR OTHER EQUIVALENT DEVICES.

B. PERMANENT EROSION CONTROL MEASURES

THE FOLLOWING PERMANENT EROSION CONTROL MEASURES HAVE BEEN DESIGNED AS PART OF THE **EROSION/SEDIMENTATION CONTROL PLAN:**

- 1. ALL STORM DRAIN PIPE OUTLETS SHALL HAVE RIPRAP APRONS AT THEIR OUTLET TO PROTECT THE OUTLET AND RECEIVING CHANNEL OF THE CULVERTS FROM SCOUR AND DETERIORATION. INSTALLATION DETAILS ARE INCLUDED WITHIN THE PLAN SET WITH THIS PACKAGE. THE APRONS SHALL BE INSTALLED AND STABILIZED TO THE EXTENT PRACTICABLE PRIOR TO DIRECTING RUNOFF TO THE TRIBUTARY PIPE OR CULVERT.
- 2. ALL AREAS DISTURBED DURING CONSTRUCTION, BUT NOT SUBJECT TO OTHER RESTORATION (PAVING, RIPRAP, ETC.) WILL BE LOAMED, LIMED, FERTILIZED, MULCHED, AND SEEDED, FABRIC NETTING, ANCHORED WITH STAPLES, SHALL BE PLACED OVER THE MULCH IN AREAS AS NOTED IN PARAGRAPH II.4.A.3. ALL AREAS WITHIN 50 FEET OF AN UNDISTURBED WETLAND SHALL BE MULCHED PRIOR TO ANY PREDICTED RAIN EVENT REGARDLESS OF THE 7-DAY WINDOW. NATIVE TOPSOIL SHALL BE STOCKPILED AND REUSED FOR FINAL RESTORATION WHEN IT IS OF SUFFICIENT

C. IMPLEMENTATION SCHEDULE

THE FOLLOWING CONSTRUCTION SEQUENCE SHALL BE REQUIRED FOR THE SITE DEVELOPMENT TO INSURE THE EFFECTIVENESS OF THE EROSION AND SEDIMENTATION CONTROL MEASURES ARE OPTIMIZED:

NOTE: FOR ALL GRADING ACTIVITIES, THE CONTRACTOR SHALL EXERCISE EXTREME CAUTION NOT TO OVEREXPOSE THE SITE BY

- LIMITING THE DISTURBED AREA. 1. INSTALL STABILIZED CONSTRUCTION ENTRANCE(S) SITE ENTRY/EXIST LOCATIONS.
- 2. CLEAR AREA NECESSARY FOR THE PARKING AREA/BUILDING CONSTRUCTION.
- 3. INSTALL PERIMETER SILTATION FENCE AND/OR WOOD WASTE BERMS PRIOR TO GRUBBING RESPECTIVE AREAS.
- 4. GRUB WORK AREA FOR PARKING LOTS AND BUILDINGS.
- 5. DURING GRUBBING OPERATIONS, INSTALL STONE CHECK DAMS AT ANY EVIDENT CONCENTRATED FLOW DISCHARGE
- 6. COMMENCE EARTHWORK AND GRADING TO SUBGRADE AS NECESSARY FOR THE PARKING LOTS AND WALKWAYS.
- 7. COMMENCE EARTHWORK AND GRADING TO SUBGRADE AS NECESSARY FOR THE BUILDING FOUNDATION.
- 8. COMMENCE INSTALLATION OF DRAINAGE APPURTENANCES, INCLUDING CATCH BASINS, MANHOLES, ETC.
- 9. COMMENCE INSTALLATION OF UNDERGROUND UTILITIES.
- 10. CONTINUE EARTHWORK AND GRADING TO SUBGRADE AS NECESSARY FOR CONSTRUCTION.
- 11. COMPLETE REMAINING EARTHWORK OPERATIONS.
- 12. COMMENCE BUILDING FOUNDATIONS.
- 13. INSTALL SUBBASE AND BASE COURSE GRAVELS WITHIN PARKING LOTS AND WALKWAYS.
- 14. COMPLETE INSTALLATION OF DRAINAGE APPURTENANCES AND UTILITY APPURTENANCES.
- 15. COMPLETE UNDERGROUND TANK INSTALLATION AND CONCRETE DISPENSER ISLAND.
- 16. INSTALL BASE COURSE PAVING FOR THE PARKING LOTS.
- 17. CONTINUE BUILDING CONSTRUCTION.
- 18. INSTALL SURFACE COURSE PAVING FOR THE PARKING, AND WALKWAY AREAS.
- 19. LOAM, LIME, FERTILIZE, SEED AND MULCH DISTURBED AREAS.
- 20. REMOVE ACCUMULATED SEDIMENT FROM AHEAD OF ANY SEDIMENT BARRIERS AS NECESSARY.
- 21. ONCE THE SITE IS STABILIZED AND A 90% CATCH OF VEGETATION HAS BEEN OBTAINED, REMOVE ALL TEMPORARY **EROSION CONTROL MEASURES**
- 22. TOUCH UP LOAM AND SEED.
- 23. COMPLETE PAVEMENT MARKINGS AND SIGNAGE

PRIOR TO CONSTRUCTION OF THE PROJECT, THE CONTRACTOR SHALL SUBMIT TO THE OWNER A SCHEDULE FOR THE COMPLETION OF THE WORK, WHICH WILL SATISFY THE FOLLOWING CRITERIA:

- 1. THE ABOVE CONSTRUCTION SEQUENCE SHOULD GENERALLY BE COMPLETED IN THE SPECIFIED ORDER; HOWEVER, SEVERAL SEPARATE ITEMS MAY BE CONSTRUCTED SIMULTANEOUSLY. WORK MUST ALSO BE SCHEDULED OR PHASED TO LIMIT THE EXTENT OF THE EXPOSED AREAS AS SPECIFIED BELOW. THE INTENT OF THIS SEQUENCE IS TO PROVIDE FOR EROSION CONTROL AND TO HAVE STRUCTURAL MEASURES SUCH AS SILT FENCE AND CONSTRUCTION ENTRANCES IN PLACE BEFORE LARGE AREAS OF LAND ARE DENUDED.
- 2. THE WORK SHALL BE CONDUCTED IN SECTIONS WHICH WILL:
- a. LIMIT THE AMOUNT OF EXPOSED AREA TO THOSE AREAS IN WHICH WORK IS EXPECTED TO BE UNDERTAKEN DURING THE PROCEEDING
- b. REVEGETATE DISTURBED AREAS AS RAPIDLY AS POSSIBLE. ALL AREAS SHALL BE PERMANENTLY STABILIZED WITHIN 7 DAYS OF FINAL GRADING OR BEFORE A STORM EVENT
- c. INCORPORATE PLANNED INLETS AND DRAINAGE SYSTEM AS EARLY AS POSSIBLE INTO THE CONSTRUCTION PHASE
- A. <u>EROSION, SEDIMENTATION AND STABILIZATION CONTROL PLAN</u>
- THE EROSION, SEDIMENTATION AND STABILIZATION CONTROL DETAILS ARE INCLUDED IN THE PLAN SET.
- B. <u>WINTER STABILIZATION PLAN</u>

THE WINTER CONSTRUCTION PERIOD IS FROM NOVEMBER 1 THROUGH APRIL 15. IF THE CONSTRUCTION SITE IS NOT STABILIZED WITH PAVEMENT, A ROAD GRAVEL BASE, 75% MATURE VEGETATION COVER OR RIPRAP BY NOVEMBER 15 THEN THE SITE NEEDS TO BE PROTECTED WITH OVER-WINTER STABILIZATION. AN AREA CONSIDERED OPEN IS ANY AREA NOT STABILIZED WITH PAVEMENT; VEGETATION, MULCHING, EROSION CONTROL MATS, RIPRAP OR GRAVEL BASE ON A PARKING LOT.

WINTER EXCAVATION AND EARTHWORK SHALL BE COMPLETED SUCH THAT NO MORE THAN 1 ACRE OF THE SITE IS WITHOUT STABILIZATION AT ANY ONE TIME. LIMIT THE EXPOSED AREA TO THOSE AREAS IN WHICH WORK IS EXPECTED TO BE UNDER TAKEN DURING THE PROCEEDING 15 DAYS AND THAT CAN BE MULCHED IN ONE DAY PRIOR TO ANY SNOW EVENT.

ALL AREAS SHALL BE CONSIDERED TO BE DENUDED UNTIL THE SUBBASE GRAVEL IS INSTALLED IN PARKING AREAS OR THE AREAS OF FUTURE LOAM AND SEED HAVE BEEN LOAMED, SEEDED AND MULCHED. HAY AND STRAW MULCH RATE SHALL BE A MINIMUM OF 150 LBS./1,000 S.F. (3 TONS/ACRE) AND SHALL BE PROPERLY ANCHORED.

THE CONTRACTOR SHALL INSTALL ANY ADDED MEASURES, WHICH MAY BE NECESSARY TO CONTROL EROSION/SEDIMENTATION FROM THE SITE DEPENDENT UPON THE ACTUAL SITE AND WEATHER CONDITIONS. CONTINUATION OF EARTHWORK OPERATIONS ON ADDITIONAL AREAS SHALL NOT BEGIN UNTIL THE EXPOSED SOIL SURFACE ON THE AREA BEING WORKED HAS BEEN STABILIZED, IN ORDER TO MINIMIZE AREAS WITHOUT EROSION CONTROL PROTECTION.

STOCKPILES OF SOIL OR SUBSOIL SHALL BE MULCHED FOR OVER WINTER PROTECTION WITH HAY OR STRAW AT TWICE THE NORMAL RATE OR AT 150 LBS/1,000 S.F. (3 TONS PER ACRE) OR WITH A FOUR-INCH LAYER OF WOODWASTE EROSION CONTROL MIX. THIS SHALL BE DONE WITHIN 24 HOURS OF STOCKING AND RE-ESTABLISHED PRIOR TO ANY RAINFALL OR SNOWFALL. ANY SOIL STOCKPILE SHALL NOT BE PLACED (EVEN COVERED WITH HAY OR STRAW) WITHIN 50 FEET FROM ANY NATURAL RESOURCES.

ANY AREAS WITHIN 50 FEET FROM ANY NATURAL RESOURCES. IF NOT STABILIZED WITH A MINIMUM OF 75% MATURE VEGETATION CATCH. SHALL BE MULCHED BY DECEMBER 1 AND ANCHORED WITH PLASTIC NETTING OR PROTECTED WITH EROSION CONTROL MATS. DURING WINTER CONSTRUCTION. A DOUBLE LINE OF SEDIMENT BARRIERS (I.E. SILT FENCE BACKED WITH HAY BALES OR EROSION CONTROL MIX) SHALL BE PLACED BETWEEN ANY NATURAL RESOURCE AND THE DISTURBED AREA. EXISTING PROJECTS NOT STABILIZED BY DECEMBER 1 SHALL BE PROTECTED WITH THE SECOND LINE OF SEDIMENT BARRIER TO ENSURE FUNCTIONALITY DURING THE SPRING THAW AND RAINS.

DURING FROZEN CONDITIONS, SEDIMENT BARRIERS SHALL CONSIST OF WOODWASTE FILTER BERMS AS FROZEN SOIL PREVENTS THE PROPER INSTALLATION OF HAY BALES AND SEDIMENT SILT FENCES.

AN AREA SHALL BE CONSIDERED DENUDED UNTIL AREAS OF FUTURE LOAM AND SEED HAVE BEEN LOAMED, SEEDED AND MULCHED. HAY AND STRAW MULCH SHALL BE APPLIED AT A RATE OF 150 LB. PER 1,000 SQUARE FEET OR 3 TONS/ACRE (TWICE THE NORMAL ACCEPTED RATE OF 75-LBS./1,000 S.F. OR 1.5 TONS/ACRE) AND SHALL BE PROPERLY ANCHORED. MULCH SHALL NOT BE SPREAD ON TOP OF SNOW. THE SNOW SHALL BE REMOVED DOWN TO A ONE-INCH DEPTH OR LESS PRIOR TO APPLICATION. AFTER EACH DAY OF FINAL GRADING. THE AREA SHALL BE PROPERLY STABILIZED WITH ANCHORED HAY OR STRAW OR EROSION CONTROL MATTING. AN AREA SHALL BE CONSIDERED TO HAVE BEEN STABILIZED WHEN EXPOSED SURFACES HAVE BEEN EITHER MULCHED WITH STRAW OR HAY AT A RATE OF 150 LB. PER 1,000 SQUARE FEET (3 TONS/ACRE) AND ADEQUATELY ANCHORED THAT GROUND SURFACE IS NOT VISIBLE THOUGH THE MULCH.

BETWEEN THE DATES OF NOVEMBER 1 AND APRIL 15, ALL MULCH SHALL BE ANCHORED BY EITHER PEG LINE, MULCH NETTING, ASPHALT EMULSION CHEMICAL, TRACK OR WOOD CELLULOSE FIBER. WHEN GROUND SURFACE IS NOT VISIBLE THROUGH THE MULCH THEN COVER IS SUFFICIENT. AFTER NOVEMBER 1ST, MULCH AND ANCHORING OF ALL BARE SOIL SHALL OCCUR AT THE END OF EACH FINAL GRADING

SLOPES SHALL NOT BE LEFT EXPOSED FOR ANY EXTENDED TIME OF WORK SUSPENSION UNLESS FULLY MULCHED AND ANCHORED WITH PEG AND NETTING OR WITH EROSION CONTROL BLANKETS. MULCHING SHALL BE APPLIED AT A RATE OF 230 LBS/1,000 S.F. ON ALL SLOPES

MULCH NETTING SHALL BE USED TO ANCHOR MULCH IN ALL DRAINAGE WAYS WITH A SLOPE GREATER THAT 3% FOR SLOPES EXPOSED TO DIRECT WINDS AND FOR ALL OTHER SLOPES GREATER THAT 8%. EROSION CONTROL BLANKETS SHALL BE USED IN LIEU OF MULCH IN ALL DRAINAGE WAYS WITH SLOPES 8% OR GREATER. EROSION CONTROL MIX CAN BE USED TO SUBSTITUTE EROSION CONTROL BLANKETS ON ALL SLOPES EXCEPT DITCHES.

BETWEEN THE DATES OF OCTOBER 15 AND APRIL 1ST. LOAM OR SEED WILL NOT BE REQUIRED. DURING PERIODS OF ABOVE FREEZING TEMPERATURES FINISHED AREAS SHALL BE FINE GRADED AND FITHER PROTECTED WITH MULICH OR TEMPORARILY SEEDED AND MULICHED. UNTIL SUCH TIME AS THE FINAL TREATMENT CAN BE APPLIED. IF THE DATE IS AFTER NOVEMBER 1ST AND IF THE EXPOSED AREA HAS BEEN LOAMED, FINAL GRADED WITH A UNIFORM SURFACE, THEN THE AREA MAY BE DORMANT SEEDED AT A RATE OF 3 TIMES HIGHER THAN SPECIFIED FOR PERMANENT SEED AND THEN MULCHED. DORMANT SEEDING MAY BE SELECTED TO BE PLACED PRIOR TO THE PLACEMENT OF MULCH AND FABRIC NETTING ANCHORED WITH STAPLES. IF DORMANT SEEDING IS USED FOR THE SITE, ALL DISTURBED AREAS SHALL RECEIVE 4" OF LOAM AND SEED AT AN APPLICATION RATE OF 5 LBS/1000 S.F. ALL AREAS SEEDED DURING THE WINTER SHALL BE INSPECTED. IN THE SPRING FOR ADEQUATE CATCH. ALL AREAS INSUFFICIENTLY VEGETATED (LESS THAN 75% CATCH) SHALL BE REVEGETATED BY REPLACING LOAM, SEED AND MULCH. IF DORMANT SEEDING IS NOT USED FOR THE SITE, ALL DISTURBED AREAS SHALL BE REVEGETATED IN THE SPRING.

WATER FROM CONSTRUCTION TRENCH DEWATERING SHALL PASS FIRST THROUGH A FILTER BAG OR SECONDARY CONTAINMENT STRUCTURE (E.G. HAY BALE LINED POOL) PRIOR TO DISCHARGE. THE DISCHARGE SITE SHALL BE SELECTED TO AVOID FLOODING, ICING, AND SEDIMENT DISCHARGES TO A PROTECTED RESOURCE. IN NO CASE SHALL THE FILTER BAG OR CONTAINMENT STRUCTURE BE LOCATED WITHIN 50 FEET OF A PROTECTED NATURAL RESOURCE.

MAINTENANCE MEASURES SHALL BE APPLIED AS NEEDED DURING THE ENTIRE CONSTRUCTION SEASON. AFTER EACH RAINFALL, SNOW STORM OR PERIOD OF THAWING AND RUNOFF. THE SITE CONTRACTOR SHALL PERFORM A VISUAL INSPECTION OF ALL INSTALLED EROSION CONTROL MEASURES AND PERFORM REPAIRS AS NEEDED TO INSURE THEIR CONTINUOUS FUNCTION. FOLLOWING THE TEMPORARY AND/OR FINAL SEEDING AND MULCHING, THE CONTRACTOR SHALL IN THE SPRING INSPECT AND REPAIR ANY DAMAGES AND/OR UNESTABLISHED SPOTS. ESTABLISHED VEGETATIVE COVER MEANS A MINIMUM OF 85% TO 90% OF AREAS VEGETATED WITH VIGOROUS GROWTH.

C. STANDARDS FOR TIMELY STABILIZATION OF CONSTRUCTION SITES DURING WINTER

1. STANDARD FOR THE TIMELY STABILIZATION OF DITCHES AND CHANNELS -- THE APPLICANT SHALL CONSTRUCT AND STABILIZE ALL STONE-LINED DITCHES AND CHANNELS ON THE SITE BY NOVEMBER 15. THE APPLICANT SHALL CONSTRUCT AND STABILIZE ALL GRASS-LINED DITCHES AND CHANNELS ON THE SITE BY SEPTEMBER 15. IF THE APPLICANT FAILS TO STABILIZE A DITCH OR CHANNEL TO BE GRASS-LINED BY SEPTEMBER 15, THEN THE APPLICANT WILL TAKE ONE OF THE FOLLOWING ACTIONS TO STABILIZE THE DITCH FOR LATE FALL AND WINTER.

- a. INSTALL A SOD LINING IN THE DITCH -- THE APPLICANT SHALL LINE THE DITCH WITH PROPERLY INSTALLED SOD BY OCTOBER 1. PROPER INSTALLATION INCLUDES THE APPLICANT PINNING THE SOD ONTO THE SOIL WITH WIRE PINS. ROLLING THE SOD TO GUARANTEE CONTACT BETWEEN THE SOD AND UNDERLYING SOIL, WATERING THE SOD TO PROMOTE ROOT GROWTH INTO THE DISTURBED SOIL, AND ANCHORING THE SOD WITH JUTE OR PLASTIC MESH TO PREVENT THE SOD STRIPS FROM SLOUGHING DURING FLOW CONDITIONS.
- b. INSTALL A STONE LINING IN THE DITCH -- THE APPLICANT SHALL LINE THE DITCH WITH STONE RIPRAP BY NOVEMBER 15. THE APPLICANT SHALL HIRE A REGISTERED PROFESSIONAL ENGINEER TO DETERMINE THE STONE SIZE AND LINING THICKNESS NEEDED TO WITHSTAND THE ANTICIPATED FLOW VELOCITIES AND FLOW DEPTHS WITHIN THE DITCH. IF NECESSARY, THE APPLICANT SHALL REGRADE THE DITCH PRIOR TO PLACING THE STONE LINING SO TO PREVENT THE STONE LINING FROM REDUCING THE DITCH'S CROSS-SECTIONAL

2. STANDARD FOR THE TIMELY STABILIZATION OF DISTURBED SLOPES -- THE APPLICANT SHALL CONSTRUCT AND STABILIZE STONE-COVERED SLOPES BY NOVEMBER 15. THE APPLICANT SHALL SEED AND MULCH ALL SLOPES TO BE VEGETATED BY SEPTEMBER 15. THE DEPARTMENT SHALL CONSIDER ANY AREA HAVING A GRADE GREATER THAN 15% (15H:1V) TO BE A SLOPE. IF THE APPLICANT FAILS TO STABILIZE ANY SLOPE TO BE VEGETATED BY SEPTEMBER 15, THEN THE APPLICANT SHALL TAKE ONE OF THE FOLLOWING ACTIONS TO STABILIZE THE SLOPE FOR LATE FALL AND WINTER.

- a. STABILIZE THE SOIL WITH TEMPORARY VEGETATION AND EROSION CONTROL MATS -- BY OCTOBER 1 THE APPLICANT SHALL SEED THE DISTURBED SLOPE WITH WINTER RYE AT A SEEDING RATE OF 3 POUNDS PER 1,000 SQUARE FEET AND APPLY EROSION CONTROL MATS OVER THE MULCHED SLOPE. THE APPLICANT SHALL MONITOR GROWTH OF THE RYE OVER THE NEXT 30 DAYS. IF THE RYE FAILS TO GROW AT LEAST THREE INCHES OR COVER AT LEAST 75% OF THE DISTURBED SLOPE BY NOVEMBER 1, THEN THE APPLICANT SHALL COVER THE SLOPE WITH A LAYER OF WOODWASTE COMPOST AS DESCRIBED IN ITEM III OF THIS STANDARD OR WITH STONE RIPRAP AS DESCRIBED IN ITEM IV OF THIS STANDARD.
- b. <u>STABILIZE THE SLOPE WITH SOD</u> -- THE APPLICANT SHALL STABILIZE THE DISTURBED SLOPE WITH PROPERLY INSTALLED SOD BY TOBER 1. PROPER INSTALLATION INCLUDES THE APPLICANT PINNING THE SOD ONTO THE SLOPE WITH WIRE PINS, ROLLING THE SOD TO GUARANTEE CONTACT BETWEEN THE SOD AND UNDERLYING SOIL, AND WATERING THE SOD TO PROMOTE ROOT GROWTH INTO THE DISTURBED SOIL. THE APPLICANT SHALL NOT USE LATE-SEASON SOD INSTALLATION TO STABILIZE SLOPES HAVING A GRADE GREATER

c. <u>STABILIZE THE SLOPE WITH WOODWASTE COMPOST</u> -- THE APPLICANT SHALL PLACE A SIX-INCH LAYER OF WOODWASTE COMPOST ON THE SLOPE BY NOVEMBER 15. PRIOR TO PLACING THE WOODWASTE COMPOST, THE APPLICANT WILL REMOVE ANY SNOW

ACCUMULATION ON THE DISTURBED SLOPE. THE APPLICANT SHALL NOT USE WOODWASTE COMPOST TO STABILIZE SLOPES HAVING

d. <u>STABILIZE THE SLOPE WITH STONE RIPRAP</u> -- THE APPLICANT SHALL PLACE A LAYER OF STONE RIPRAP ON THE SLOPE BY NOVEMBER 15. THE APPLICANT SHALL HIRE A REGISTERED PROFESSIONAL ENGINEER TO DETERMINE THE STONE SIZE NEEDED FOR STABILITY AND TO DESIGN A FILTER LAYER FOR UNDERNEATH THE RIPRAP.

GRADES GREATER THAN 50% (2H:1V) OR HAVING GROUNDWATER SEEPS ON THE SLOPE FACE.

3. STANDARD FOR THE TIMELY STABILIZATION OF DISTURBED SOILS -- BY SEPTEMBER 15 THE APPLICANT SHALL SEED AND MULCH ALL DISTURBED SOILS ON AREAS HAVING A SLOPE LESS THAN 15%. IF THE APPLICANT FAILS TO STABILIZE THESE SOILS BY THIS DATE, THEN THE APPLICANT SHALL TAKE ONE OF THE FOLLOWING ACTIONS TO STABILIZE THE SOIL FOR LATE FALL AND WINTER.

- a. STABILIZE THE SOIL WITH TEMPORARY VEGETATION -- BY OCTOBER 1 THE APPLICANT SHALL SEED THE DISTURBED SOIL WITH WINTER RYE AT A SEEDING RATE OF 3 POUNDS PER 1000 SQUARE FEET, LIGHTLY MULCH THE SEEDED SOIL WITH HAY OR STRAW AT 75 POUNDS PER 1000 SQUARE FEET, AND ANCHOR THE MULCH WITH PLASTIC NETTING. THE APPLICANT SHALL MONITOR GROWTH OF THE RYE OVER THE NEXT 30 DAYS. IF THE RYE FAILS GROW AT LEAST THREE INCHES OR COVER AT LEAST 75% OF THE DISTURBED SOIL BEFORE NOVEMBER 15, THEN THE APPLICANT SHALL MULCH THE AREA FOR OVER-WINTER PROTECTION AS DESCRIBED IN ITEM III OF THIS STANDARD.
- b. $\underline{\mathsf{STABILIZE}}$ THE SOIL WITH SOD -- THE APPLICANT SHALL STABILIZE THE DISTURBED SOIL WITH PROPERLY INSTALLED SOD BY OCTOBER 1. PROPER INSTALLATION INCLUDES THE APPLICANT PINNING THE SOD ONTO THE SOIL WITH WIRE PINS, ROLLING THE SOD TO GUARANTEE CONTACT BETWEEN THE SOD AND UNDERLYING SOIL, AND WATERING THE SOD TO PROMOTE ROOT GROWTH INTO THE DISTURBED SOIL.
- c. STABILIZE THE SOIL WITH MULCH -- BY NOVEMBER 15 THE APPLICANT SHALL MULCH THE DISTURBED SOIL BY SPREADING HAY OR STRAW AT A RATE OF AT LEAST 150 POUNDS PER 1000 SQUARE FEET ON THE AREA SO THAT NO SOIL IS VISIBLE THROUGH THE MULCH. PRIOR TO APPLYING THE MULCH, THE APPLICANT SHALL REMOVE ANY SNOW ACCUMULATION ON THE DISTURBED AREA. IMMEDIATELY AFTER APPLYING THE MULCH, THE APPLICANT WILL ANCHOR THE MULCH WITH PLASTIC NETTING TO PREVENT WIND FROM MOVING THE MULCH OFF THE DISTURBED SOIL.
- G. MAINTENANCE OF FACILITIES

THE STORMWATER FACILITIES WILL BE MAINTAINED BY THE APPLICANT, OR THEIR ASSIGNED HEIRS. THE CONTRACT DOCUMENTS WILL REQUIRE THE CONTRACTOR TO DESIGNATE A PERSON, WHO HAS KNOWLEDGE OF EROSION AND STORMWATER CONTROL INCLUDING THE STANDARDS AND CONDITIONS IN THE PERMIT, TO BE RESPONSIBLE FOR MAINTENANCE OF THE SEDIMENTATION CONTROL FEATURES DURING CONSTRUCTION AS REQUIRED BY THE EROSION CONTROL REPORT, DOCUMENTATION OF ANY INSPECTIONS, MAINTENANCE, AND ANY CORRECTIVE ACTIONS TAKEN SHALL BE SUMMARIZED IN A LOG (REPORT). THE LOG SHALL COMPLY WITH CHAPTER 500 APPENDIX B.1,C AND APPENDIX B.2.D. OF THE STATE OF MAINE DEP. STORMWATER MANAGEMENT

LONG-TERM OPERATION/MAINTENANCE RECOMMENDED FOR THE STORMWATER FACILITY IS PRESENTED BELOW. THE RESPONSIBLE PARTY MAY CONTRACT WITH SUCH PROFESSIONALS, AS MAY BE NECESSARY IN ORDER TO COMPLY WITH THIS PROVISION AND MAY RELY ON THE ADVICE OF SUCH PROFESSIONALS IN CARRYING OUT ITS DUTY HEREUNDER, PROVIDED, THAT THE FOLLOWING OPERATION AND MAINTENANCE PROCEDURES ARE HEREBY ESTABLISHED AS A MINIMUM FOR COMPLIANCE WITH THIS SECTION.

- PARKING LOTS CLEAR ACCUMULATIONS OF WINTER SAND AT LEAST TWICE A YEAR, PREFERABLY IN THE SPRING. EXCESS SHOULDER MATERIAL SHALL BE REMOVED MANUALLY OR BY FRONT-END LOADER. INSPECT PARKING LOT SIDE SLOPES ON AN ANNUAL BASIS FOR SLOUGHING OR UNDESIRABLE GROWTH.
- CATCH BASINS REMOVE SEDIMENT FROM CATCH BASINS WHEN SEDIMENT HAS ACCUMULATED TO WITHIN 6 INCHES OF THE OUTLET INVERT BUT NO LESS THAN ANNUALLY.
- STORMWATER BASIN INSPECT AND MAINTAIN ON A SEMI-ANNUAL BASIS INCLUDING, BUT NOT LIMITED TO, VEGETATION REMOVAL, EMBANKMENT INSPECTION, PIPES AND CONTROL STRUCTURES.
- H. IN ACCORDANCE WITH APPENDIX C OF THE MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION CHAPTER 500 STORMWATER MANAGEMENT REGULATIONS PERTAINING TO PROJECT HOUSEKEEPING, THE FOLLOWING SHALL APPLY:
- 1. SPILL PREVENTION. CONTROLS MUST BE USED TO PREVENT POLLUTANTS FROM CONSTRUCTION AND WASTE MATERIALS STORED ON SITE TO ENTER STORMWATER, WHICH INCLUDES STORAGE PRACTICES TO MINIMIZE EXPOSURE OF THE MATERIALS TO STORMWATER. THE SITE CONTRACTOR OR OPERATOR MUST DEVELOP, AND IMPLEMENT AS NECESSARY, APPROPRIATE SPILL PREVENTION, CONTAINMENT, AND RESPONSE PLANNING
- NOTE: ANY SPILL OR RELEASE OF TOXIC OR HAZARDOUS SUBSTANCES MUST BE REPORTED TO THE DEPARTMENT. FOR OIL SPILLS, CALL 1-800-482-0777 WHICH IS AVAILABLE 24 HOURS A DAY. FOR SPILLS OF TOXIC OR HAZARDOUS MATERIAL, CALL 1-800-452-4664 WHICH IS AVAILABLE 24 HOURS A DAY. FOR MORE INFORMATION. VISIT THE DEPARTMENT'S WEBSITE AT: HTTP://WWW.MAINE.GOV/DEP/SPILLS/EMERGSPILLRESP/
- 2. GROUNDWATER PROTECTION. DURING CONSTRUCTION, LIQUID PETROLEUM PRODUCTS AND OTHER HAZARDOUS MATERIALS WITH THE POTENTIAL TO CONTAMINATE GROUNDWATER MAY NOT BE STORED OR HANDLED IN AREAS OF THE SITE DRAINING TO AN INFILTRATION AREA AN "INFILTRATION AREA" IS ANY AREA OF THE SITE THAT BY DESIGN OR AS A RESULT OF SOILS. TOPOGRAPHY AND OTHER RELEVANT FACTORS ACCUMULATES RUNOFF THAT INFILTRATES INTO THE SOIL. DIKES, BERMS, SUMPS, AND OTHER FORMS OF SECONDARY CONTAINMENT THAT PREVENT DISCHARGE TO GROUNDWATER MAY BE USED TO ISOLATE PORTIONS OF THE SITE FOR THE PURPOSES OF STORAGE AND HANDLING OF THESE MATERIALS, ANY PROJECT PROPOSING INFILTRATION OF STORMWATER MUST PROVIDE ADEQUATE PRE-TREATMENT OF STORMWATER PRIOR TO DISCHARGE OF STORMWATER TO THE INFILTRATION AREA, OR PROVIDE FOR TREATMENT WITHIN THE INFILTRATION AREA, IN ORDER TO PREVENT THE ACCUMULATION OF FINES, REDUCTION IN INFILTRATION RATE, AND CONSEQUENT FLOODING AND DESTABILIZATION.
- 3. FUGITIVE SEDIMENT AND DUST, ACTIONS MUST BE TAKEN TO ENSURE THAT ACTIVITIES DO NOT RESULT IN NOTICEABLE EROSION OF SOILS OR FUGITIVE DUST EMISSIONS DURING OR AFTER CONSTRUCTION. OIL MAY NOT CONSTRUCTION ENTRANCE (SCE) SHOULD BE INCLUDED TO MINIMIZE TRACKING OF MUD AND SEDIMENT. IF OFF-SITE TRACKING OCCURS, PUBLIC ROADS SHOULD BE SWEPT IMMEDIATELY AND NO LESS THAN ONCE A WEEK AND PRIOR TO SIGNIFICANT STORM EVENTS. OPERATIONS DURING DRY MONTHS, THAT EXPERIENCE FUGITIVE DUST PROBLEMS, SHOULD WET DOWN UNPAVED ACCESS ROADS ONCE A WEEK OR MORE FREQUENTLY AS NEEDED WITH A WATER ADDITIVE TO SUPPRESS FUGITIVE SEDIMENT AND DUST.
- NOTE: DEWATERING A STREAM WITHOUT A PERMIT FROM THE DEPARTMENT MAY VIOLATE STATE WATER QUALITY STANDARDS AND THE NATURAL RESOURCES PROTECTION ACT.
- 4. DEBRIS AND OTHER MATERIALS, MINIMIZE THE EXPOSURE OF CONSTRUCTION DEBRIS, BUILDING AND LANDSCAPING MATERIALS. TRASH. FERTILIZERS. PESTICIDES. HERBICIDES. DETERGENTS. SANITARY WASTE AND OTHER MATERIALS TO PRECIPITATION AND STORMWATER RUNOFF. THESE MATERIALS MUST BE PREVENTED FROM BECOMING A POLLUTANT SOURCE.
- NOTE: TO PREVENT THESE MATERIALS FROM BECOMING A SOURCE OF POLLUTANTS, CONSTRUCTION AND POST-CONSTRUCTION ACTIVITIES RELATED TO A PROJECT MAY BE REQUIRED TO COMPLY WITH APPLICABLE PROVISION OF RULES RELATED TO SOLID, UNIVERSAL, AND HAZARDOUS WASTE, INCLUDING, BUT NOT LIMITED TO, THE MAINE SOLID WASTE AND HAZARDOUS WASTE MANAGEMENT RULES: MAINE HAZARDOUS WASTE MANAGEMENT RULES: MAINE OIL CONVEYANCE AND STORAGE RULES; AND MAINE PESTICIDE REQUIREMENTS.
- 5. EXCAVATION DE-WATERING. EXCAVATION DE-WATERING IS THE REMOVAL OF WATER FROM TRENCHES, FOUNDATIONS, COFFER DAMS, PONDS, AND OTHER AREAS WITHIN THE CONSTRUCTION AREA THAT RETAIN WATER AFTER EXCAVATION. IN MOST CASES THE COLLECTED WATER IS HEAVILY SILTED AND HINDERS CORRECT AND SAFE CONSTRUCTION PRACTICES. THE COLLECTED WATER REMOVED FROM THE PONDED AREA. EITHER THROUGH GRAVITY OR PUMPING. MUST BE SPREAD THROUGH NATURAL WOODED BUFFERS OR REMOVED TO AREAS THAT ARE SPECIFICALLY DESIGNED TO COLLECT THE MAXIMUM AMOUNT OF SEDIMENT POSSIBLE, LIKE A COFFERDAM SEDIMENTATION BASIN. AVOID ALLOWING THE WATER TO FLOW OVER DISTURBED AREAS OF THE SITE. EQUIVALENT MEASURES MAY BE TAKEN IF APPROVED BY THE DEPARTMENT
- NOTE: DEWATERING CONTROLS ARE DISCUSSED IN THE "MAINE EROSION AND SEDIMENT CONTROL BMPS, MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION."



482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355

Copyright Reserved

www.stantec.com

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay. The Copyrights to all designs and drawings are the property of

Stantec. Reproduction or use for any purpose other than that

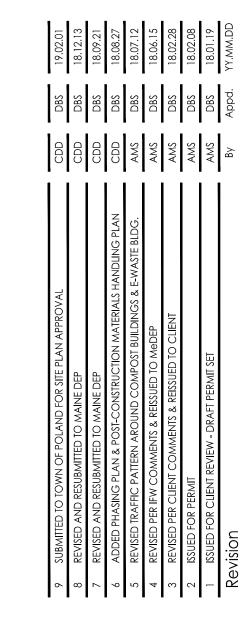
Consultants

authorized by Stantec is forbidden.

Westbrook, Maine 04092

Tel. 207.591.7000

Notes



File Name: 50465 details

Permit-Seal

Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION

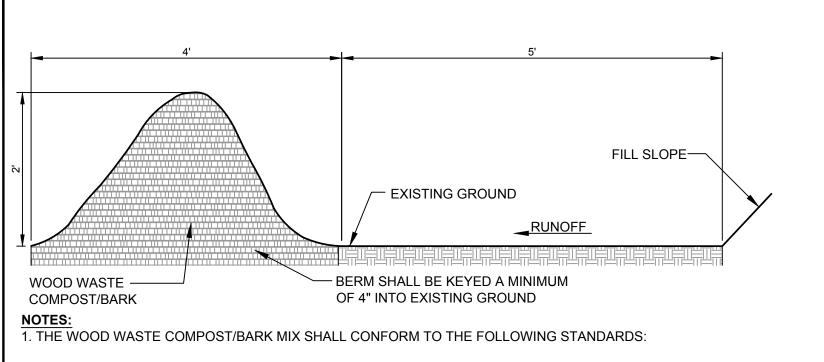
AUBURN, MAINE / POLAND, MAINE

Project No. 195350465

Sheet

Scale

N.T.S.



A. MOISTURE CONTENT - 30-60%

B. pH - 5.0-8.0 C. SCREEN SIZE - 100% LESS THAN 3", MAX. 70% LESS THAN 1"

D. NO LESS THAN 40% ORGANIC MATERIAL (DRY WEIGHT) BY LOSS OF IGNITION F. NO STONES LARGER THAN 2" IN DIAMETER

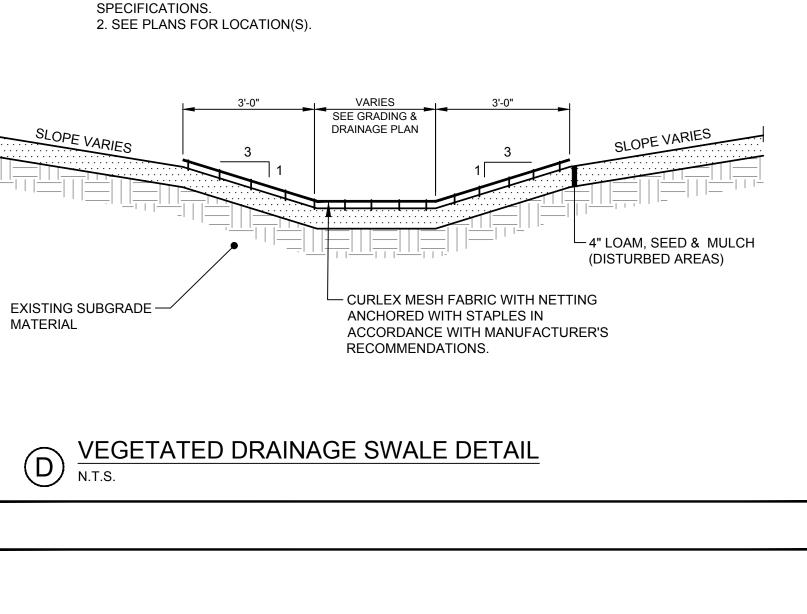
2. THE COMPOST BERM SHALL BE PLACED, UNCOMPACTED, ALONG A RELATIVELY LEVEL CONTOUR.

3. THE WOOD WASTE COMPOST/BARK FILTER BERM MAY BE USED IN LIEU OF SILTATION FENCE, AT THE TOE OF SHALLOW SLOPES, ON FROZEN GROUND, LEDGE OUT CROPS, VERY ROOTED FORESTED AREA OR AT THE EDGE OF GRAVEL PARKING AREAS.

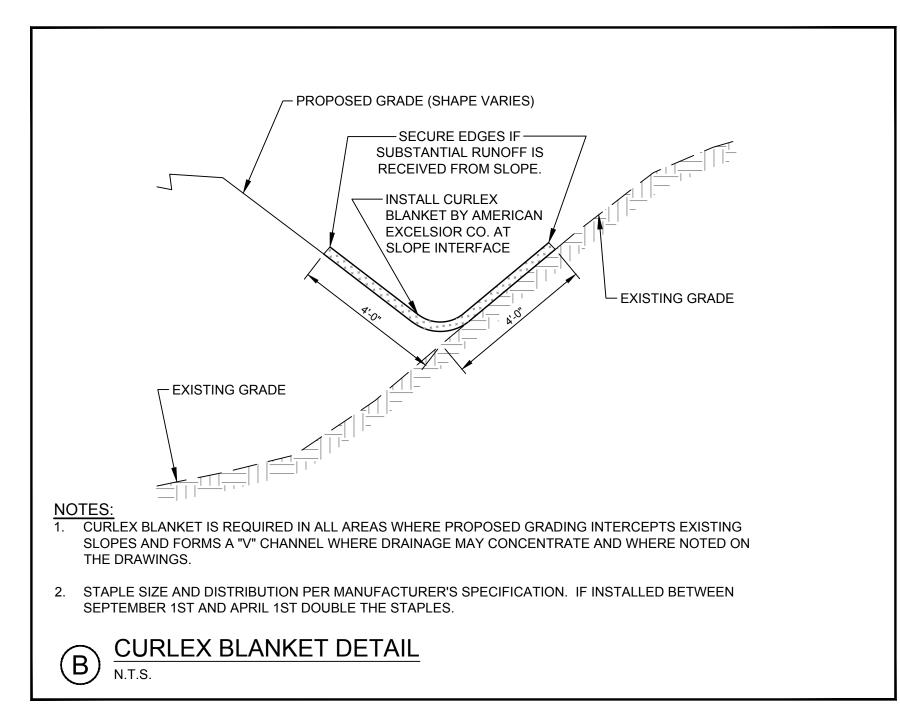
4. BERMS SHALL REMAIN IN PLACE UNTIL UPSTREAM AREA IS COMPLETED OR 70% CATCH OF VEGETATION IS ATTAINED. BERMS SHALL BE REMOVED BY SPREADING SUCH THAT THE NATIVE EARTH CAN BE SEEN BELOW.

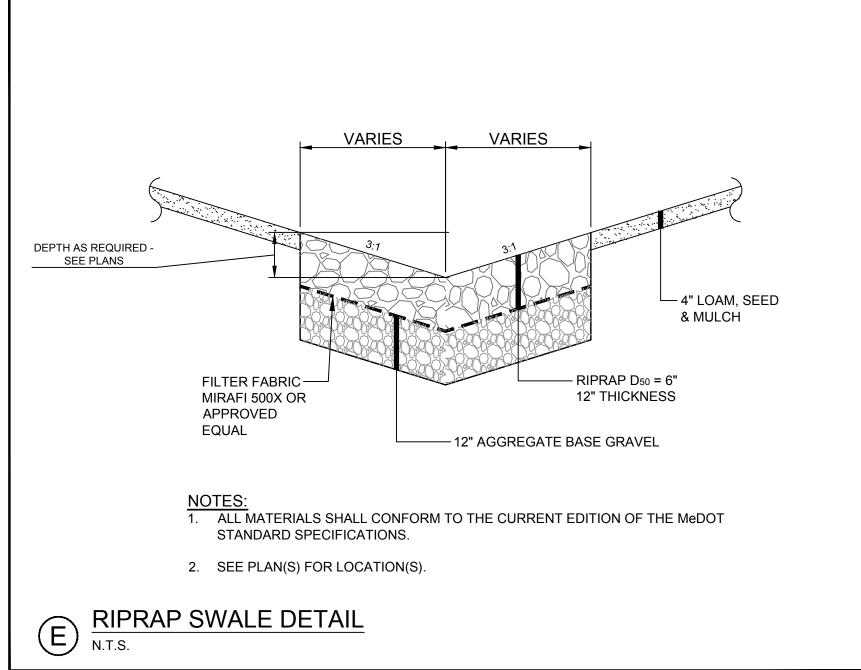
 $A \frac{VVO}{N.T.S.}$

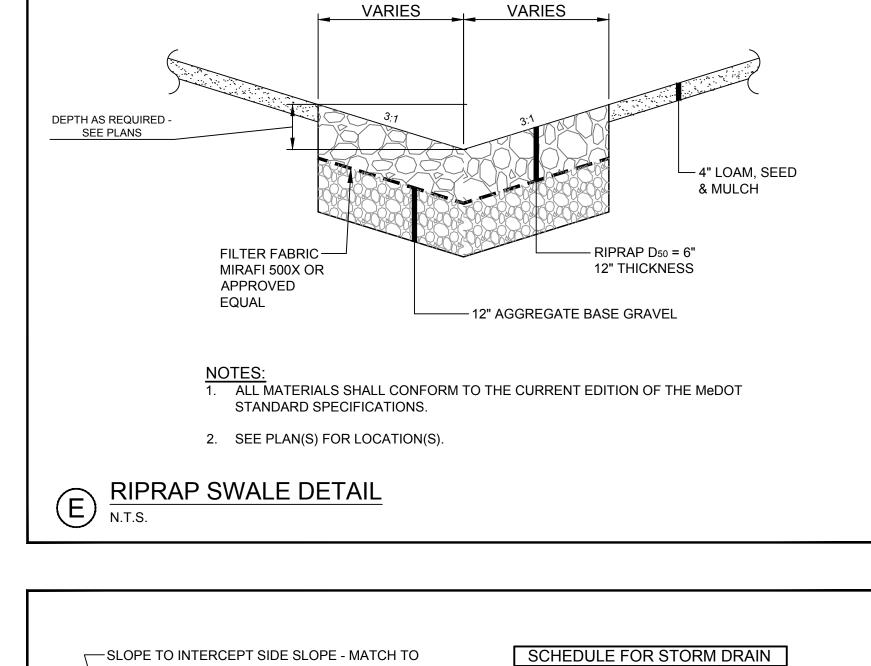
WOOD WASTE COMPOST / BARK FILTER BERM

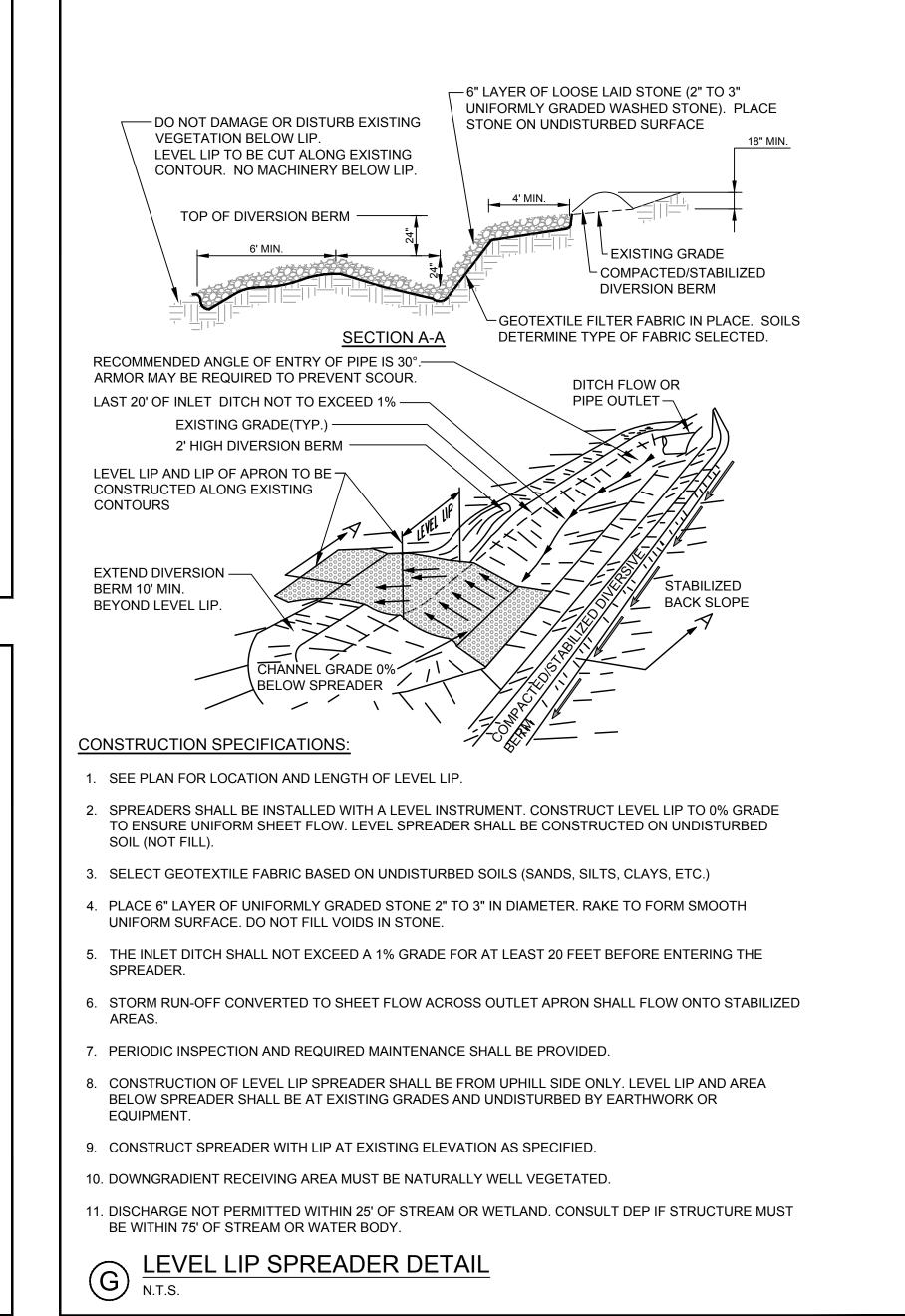


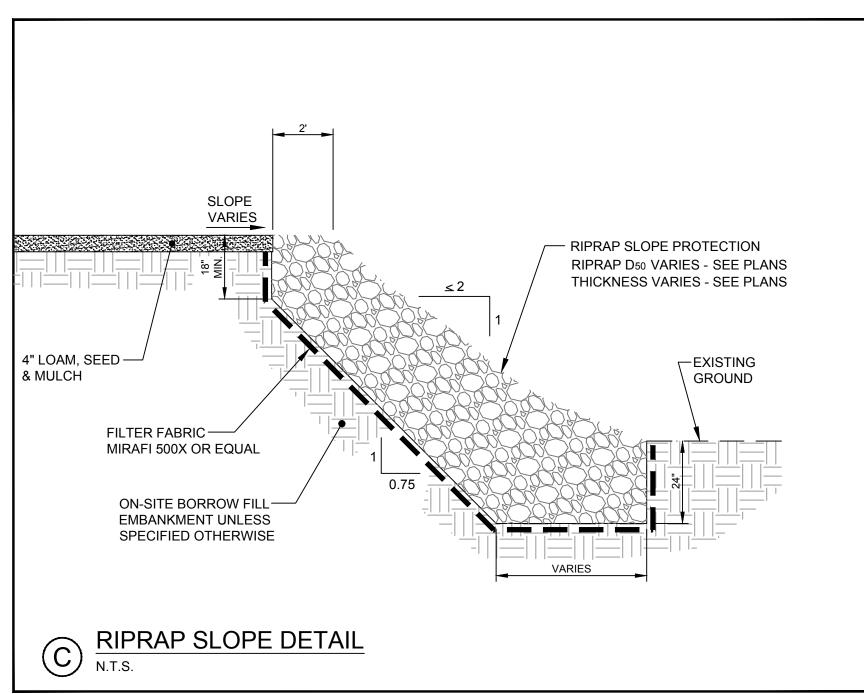
1. ALL MATERIALS SHALL CONFORM TO THE CURRENT EDITION OF THE MDOT STANDARD

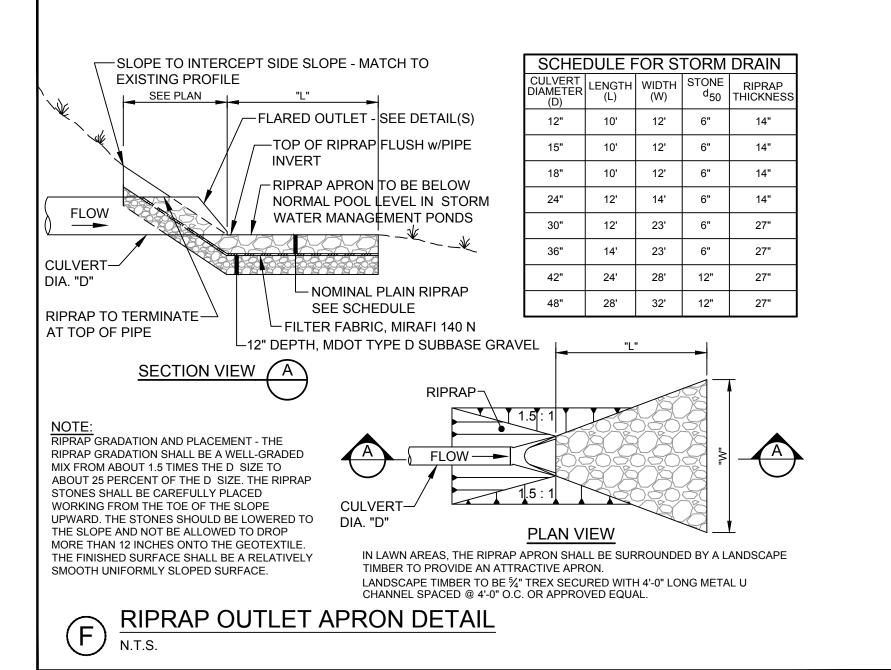


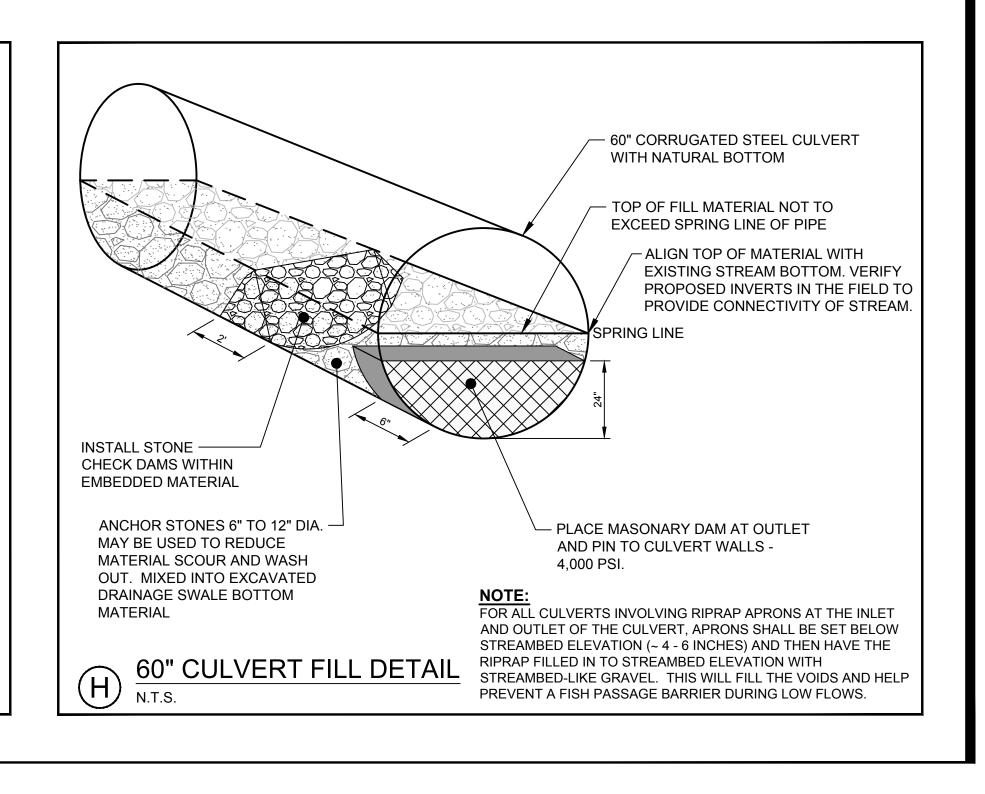














482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

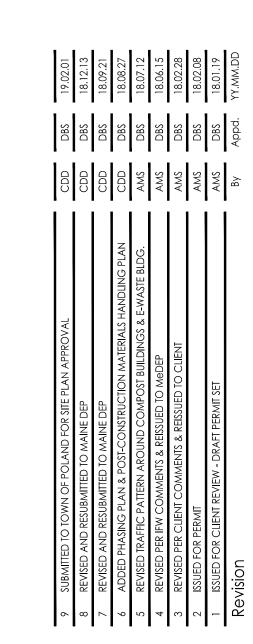
The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay. The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

Consultants

authorized by Stantec is forbidden.

846 Main Street Westbrook, Maine 04092 Tel. 207.591.7000

Notes



File Name: 50465 details Permit-Seal

Client/Project ST.GERMAIN COLLINS MB BARK, LLC

> MB BARK RECYCLING FACILITY EXPANSION

AUBURN, MAINE / POLAND, MAINE

SITE DETAILS

Project No. Scale 195350465 N.T.S. Sheet

C-7.1

LOCATION OF DIRTBAGS TO BE SELECTED BY THE CONTRACTOR BUT SHALL NOT BE SITED IN THE CRITICAL

SPECIFICATIONS AND REQUIREMENTS FOR DEWATERING

THIS PROJECT WILL REQUIRE THE DISCHARGE OF CONSTRUCTION DEWATERING AND TURBID LADEN RUNOFF FROM THE SITE TO BE DIRECTED AND DISCHARGED THROUGH A DIRTBAG. THIS DESCRIPTION ALSO CONTAINS APPENDED MATERIALS DESCRIBING THE **DIRTBAGS** REFERRED TO IN THIS NARRATIVE.

THE PROJECT WILL BENEFIT FROM A POND DESIGNED NOT ONLY FOR DETENTION BUT ALSO FOR USE AS SEDIMENTATION BASINS DURING CONSTRUCTION. HOWEVER, IT IS RECOGNIZED THAT WEATHER CONDITIONS ARE NOT ALWAYS PREDICTABLE; THERE MAY BE EXCEPTIONAL PERIODS WHEN CONSTRUCTION ACTIVITY RESULTS IN HIGHLY TURBID WATER WHICH IS NOT CONSIDERED DESIRABLE TO DISCHARGE TO THE PONDS, OR LIMITED ACTIVITY IS REQUIRED THAT MAY NOT BE EASILY ACCOMMODATED BY THE PONDS. TRADITIONALLY, MEDEP PERMITS HAVE HAD A STANDARD CONDITION WHICH STATES:

"THE APPLICANT SHALL TAKE ALL NECESSARY ACTIONS TO ENSURE THAT ITS ACTIVITIES OR THOSE OF ITS AGENTS DO NOT RESULT IN NOTICEABLE EROSION OF SOILS OR FUGITIVE DUST EMISSIONS ON THE SITE DURING THE CONSTRUCTION AND OPERATION OF THE PROJECT COVERED BY THIS APPROVAL."

THESE SPECIFICATIONS HAVE BEEN DEVELOPED FOR THE PURPOSE OF ADDRESSING CONSTRUCTION-DEWATERING ACTIVITIES WITH THE CONTINGENCY THAT UNPREDICTABLE WEATHER CAN CREATE. THE SPECIFICATION IS INTENDED TO "SHARE THE RISK" BETWEEN THE CONTRACTOR AND OWNER. IT IS ANTICIPATED THAT THIS METHOD WILL ALLOW THE BASE BID FOR THE PROJECT TO HAVE A REDUCED BUILT-IN CONTINGENCY COST FOR CERTAIN WEATHER-RELATED FACTORS.

THIS SPECIFICATION IS NOT INTENDED TO DIMINISH THE RECOGNIZED AND POTENTIAL AID OF THE PROPOSED SEDIMENT PONDS TO ACT AS THE PRIMARY DEVICE TO CAPTURE AND RETAIN SUSPENDED SEDIMENT. THIS BENEFIT IS A PRINCIPAL REASON WHY THE CONSTRUCTION OF THE PONDS EARLY IN THE PROJECT IS SO IMPORTANT.

ACCEPTABLE METHODS OF DISCHARGING CONSTRUCTION SITE RUNOFF:

DEWATERING OF THE CONSTRUCTION SITE SHALL BE ACCOMPLISHED USING ONE OF THE FOLLOWING MEASURES:

THE DIRECTION OF THE RUNOFF TO THE SEDIMENTATION BASIN BY GRAVITY FLOW.

THE PUMPING OF DIRTBAGS WITH A DISCHARGE TO THE POND.

THE PUMPING OF CONSTRUCTION SITE WATER AND COLLECTED RUNOFF TO A **DIRTBAG** (PATENTED PRODUCT BY ACF ENVIRONMENTAL PRODUCTS) WITH RELEASE THROUGH A VEGETATED BUFFER AT LEAST 50 FEET UPGRADIENT OF A WETLAND.

REQUIREMENTS FOR DIRTBAGS:

THE SITE CONTRACTOR SHALL INCLUDE THE PRICE OF INSTALLING, OPERATING, AND REMOVAL AND DISPOSAL OF FOUR DIRTBAG 55'S AS PART OF THE BASE BID. A UNIT PRICE SHALL BE PROVIDED FOR ADDITIONAL DIRTBAGS.

AT ALL TIMES THERE MUST BE AN UNUSED DIRTBAG AVAILABLE FOR EMERGENCY USE.

AT ALL TIMES (AFTER INITIAL SITE PREPARATION), THE CONTRACTOR SHALL HAVE ONE DIRTBAG ACTIVE OR READY FOR USE. THE DIRTBAGS SHALL BE FIELD LOCATED BY THE CONTRACTOR BUT ARE NOT TO BE INSTALLED IN ANY "CRITICAL" AREA. (THE SITE CRITICAL AREAS ARE SHOWN ON THE EROSION-SEDIMENT CONTROL PLAN.) THE **DIRTBAG** SHALL BE INSTALLED ON A PREPARED SUBGRADE. THIS SUBGRADE SHALL CONSIST OF THE INSTALLATION OF A LAYER OF MIRAFI 600X, AND 18 INCHES OF ¾ INCH CRUSHED STONE. THE PLAN DIMENSION OF THE CRUSHED STONE PAD SHALL EXCEED THE PLAN AREA OF THE **DIRTBAG** BY AT LEAST TWO FEET IN ALL DIRECTIONS. THE DIRTBAG SHALL NOT BE INSTALLED ON AN UNDERLYING SLOPE OF GREATER THAN 15 PERCENT.

CONSTRUCTION DEWATERING OPERATIONS:

ALL CONSTRUCTION-DEWATERING OPERATIONS ARE THE RESPONSIBILITY OF THE SITE CONTRACTOR. IT SHALL BE THE SITE CONTRACTOR WHO IS RESPONSIBLE FOR SELECTING THE SITE FOR THE DIRTBAG. THE SELECTION OF THE USE OF THE **DIRTBAG** OR THE SEDIMENTATION BASIN FOR DIRECTING DEWATERING, EXCEPT THAT THE OWNER MAY DIRECT THE SITE CONTRACTOR TO ALTER THE SELECTED OPERATION IF TURBID DISCHARGE TO A WETLAND OR EITHER CULVERT UNDER WEEKS ROAD IS OBSERVED.

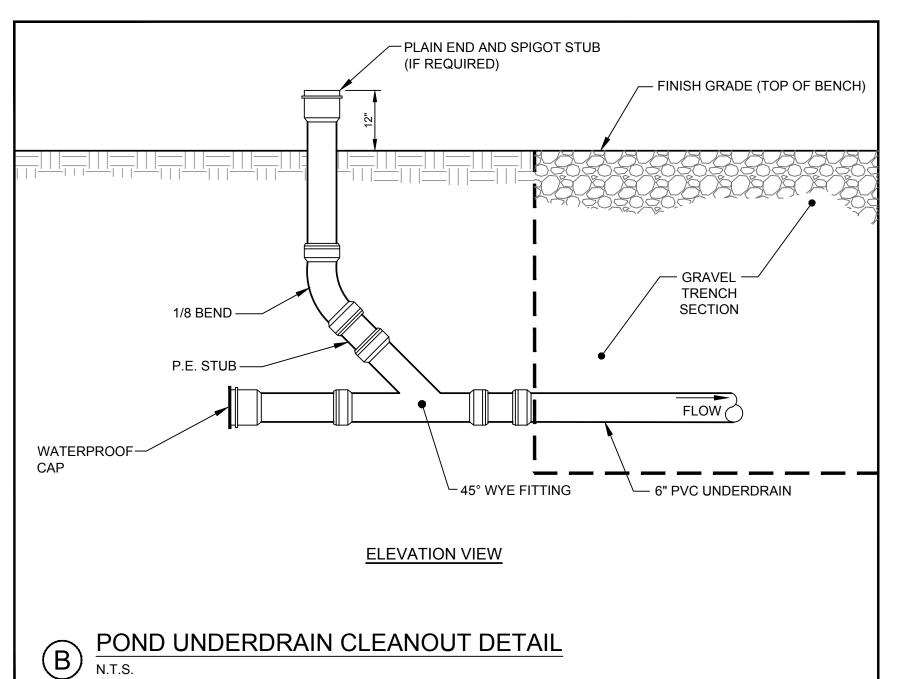
WINTER OPERATIONS:

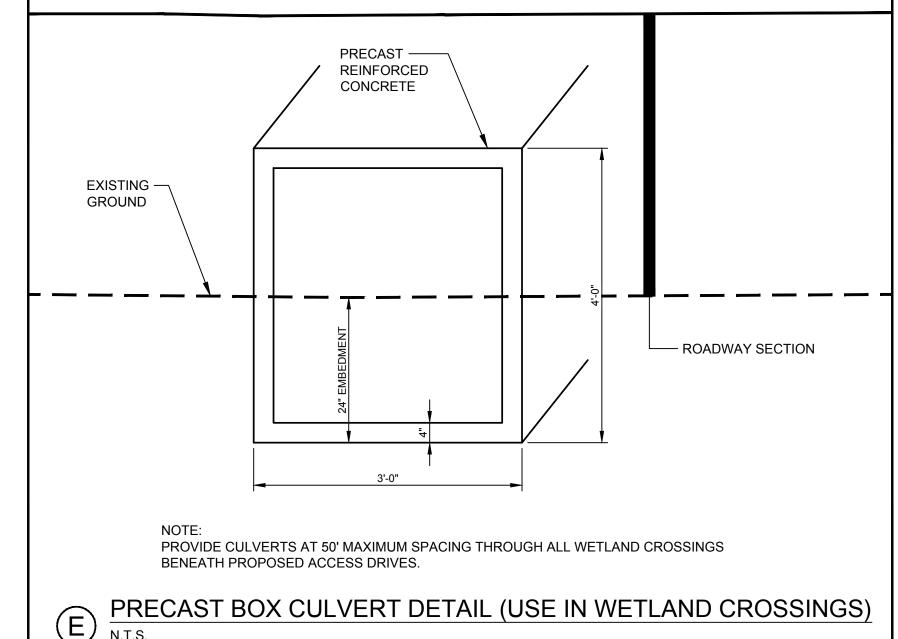
IN THE EVENT THAT WINTER OPERATIONS ARE REQUIRED, THE CONTRACTOR SHALL "POLY", ENCLOSE, AND PROVIDE TEMPORARY HEAT TO PREVENT THE **DIRTBAG** FROM SUBSTANTIAL FREEZING.

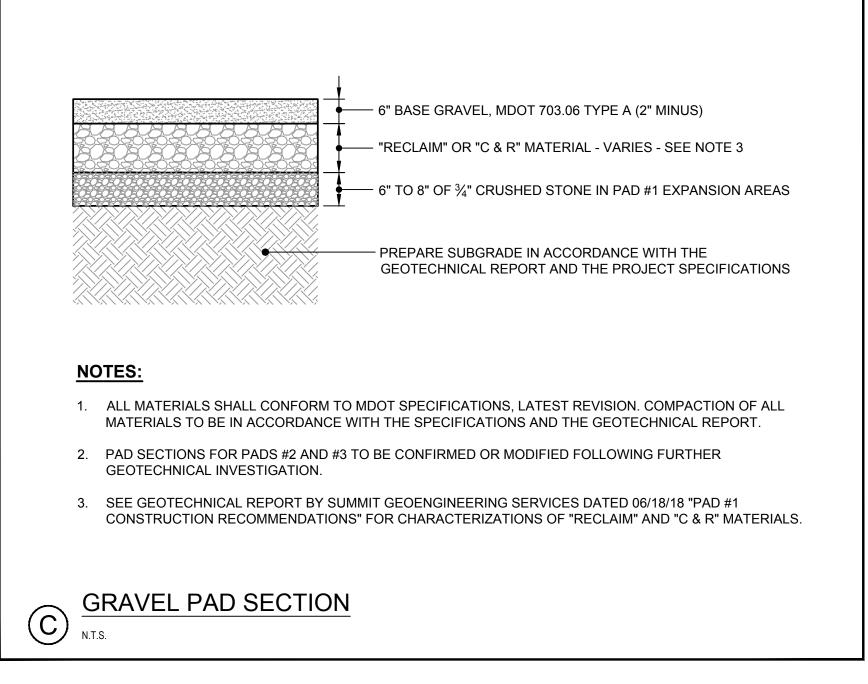
RECORD KEEPING

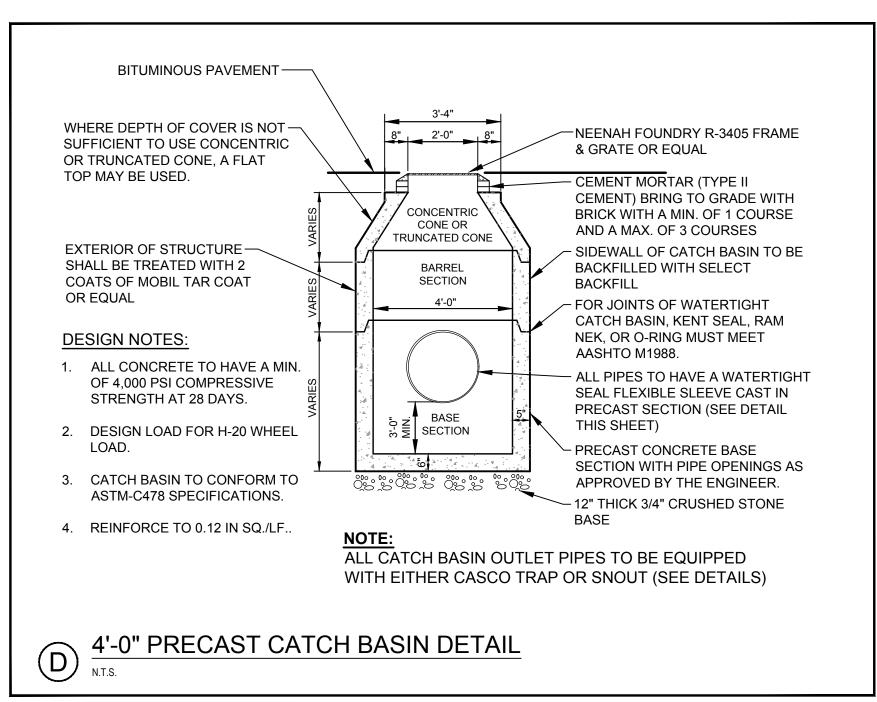
THE WEEKLY EROSION-SEDIMENT CONTROL REPORTS PREPARED IN ACCORDANCE WITH THE NPDES PERMIT SHALL MAINTAIN A LOG OF THE LOCATION, USE, AND REMOVAL OF **DIRTBAGS**. IN THE EVENT THAT THE STONE UNDER THE DIRTBAG BECOMES HIGHLY CONTAMINATED WITH FINES, THE NEXT DIRTBAG SHALL BE INSTALLED IN A DIFFERENT LOCATION.

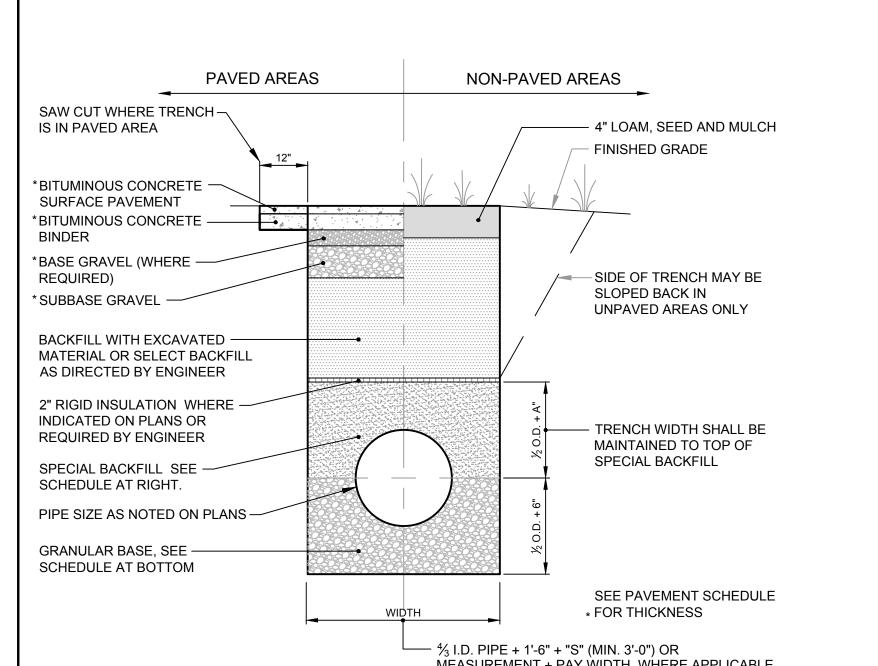
DIRTBAG® DETAIL AND SPECIFICATIONS











MEASUREMENT + PAY WIDTH, WHERE APPLICABLE

BRACING AND SHEETING OR OTHER TRENCH PROTECTION TO BE PROVIDED TO MEET APPLICABLE

2. WHERE APPLICABLE, SERVICES TO BE SIMILAR EXCEPT MINIMUM PAY WIDTH IS 2 1/2 FT.

RESPONSIBILITY OF THE CONTRACTOR.

STATE AND O.S.H.A. SAFETY STANDARDS. ALL SUCH TRENCH PROTECTION TO BE THE

SCHEDULE OF BASE BACKFILL						
TYPE OF PIPE	BEDDING MATERIAL	SPECIAL BACKFILL	SPECIAL BACKFILL COVER "A" (IN)	SELECT BACKFILL		
CONC.	3/4" CRUSHED STONE	CLEAN SAND	12	CLEAN SAND		
PVC	3/4" CRUSHED STONE	CLEAN SAND	6	CLEAN SAND		
CMP	3/4" CRUSHED STONE	CLEAN SAND	6	CLEAN SAND		
HDPE	3/4" CRUSHED STONE	CLEAN SAND	6	CLEAN SAND		

TYPICAL STORM DRAIN PIPE TRENCH SECTION DETAIL



482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay. The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that authorized by Stantec is forbidden.

Consultants

846 Main Street Westbrook, Maine 04092 Tel. 207.591.7000

Notes

File Name: 50465 details

Permit-Seal

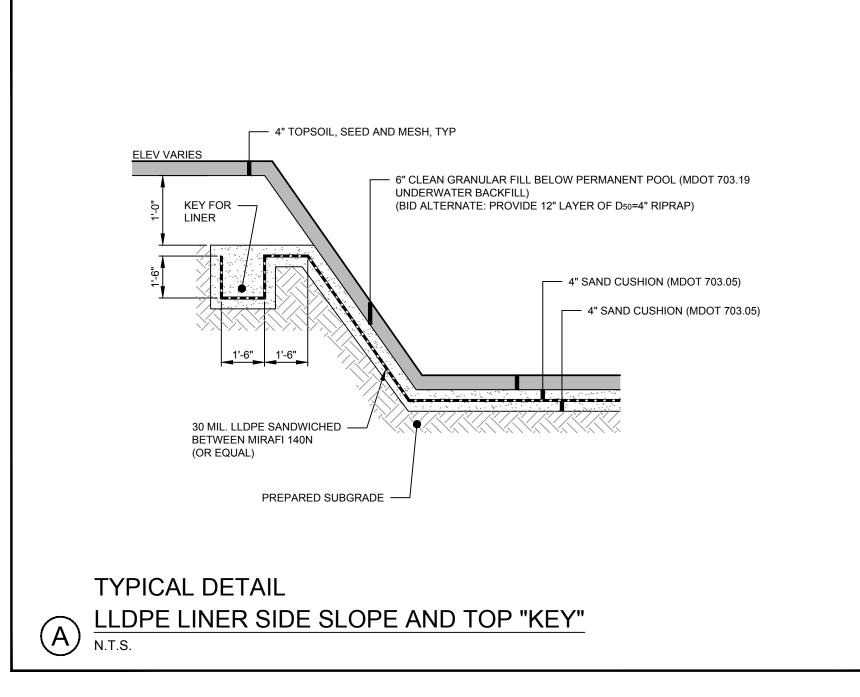
Client/Project ST.GERMAIN COLLINS MB BARK, LLC

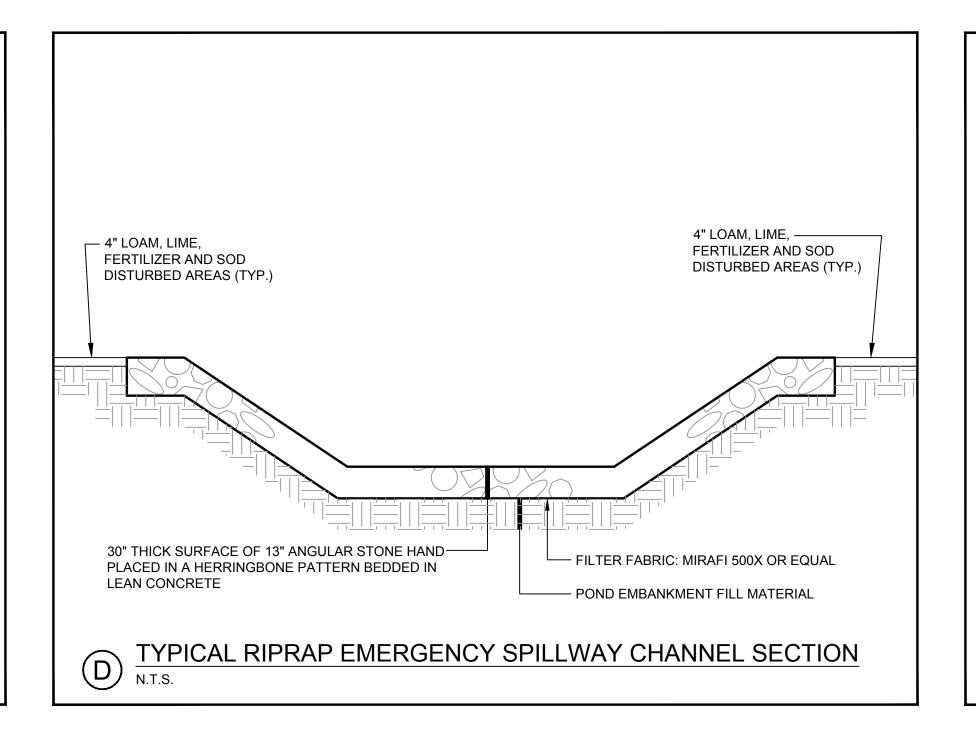
MB BARK RECYCLING FACILITY EXPANSION

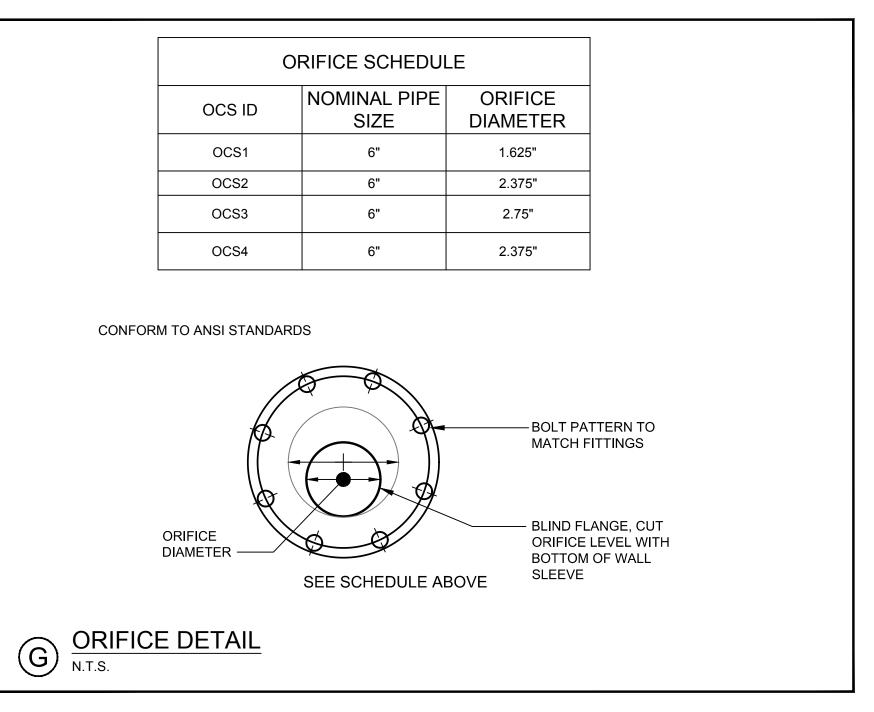
AUBURN, MAINE / POLAND, MAINE

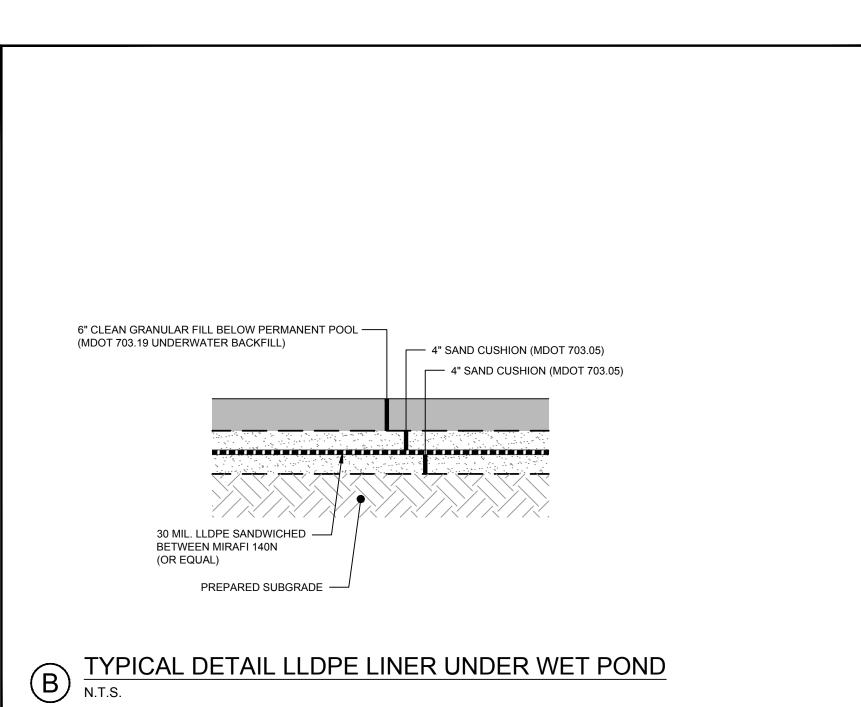
SITE DETAILS

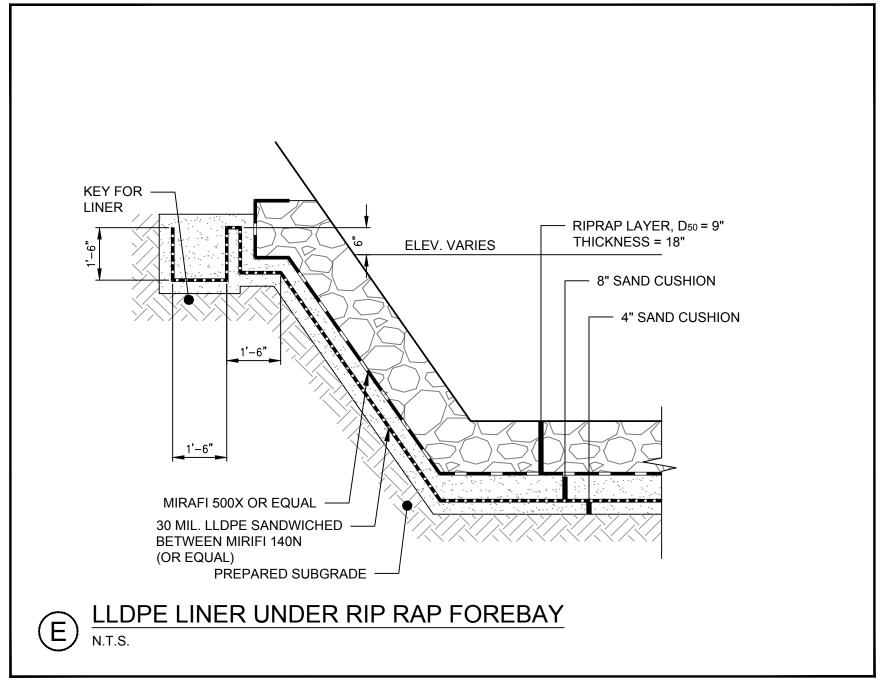
Project No. Scale 195350465 N.T.S. Sheet

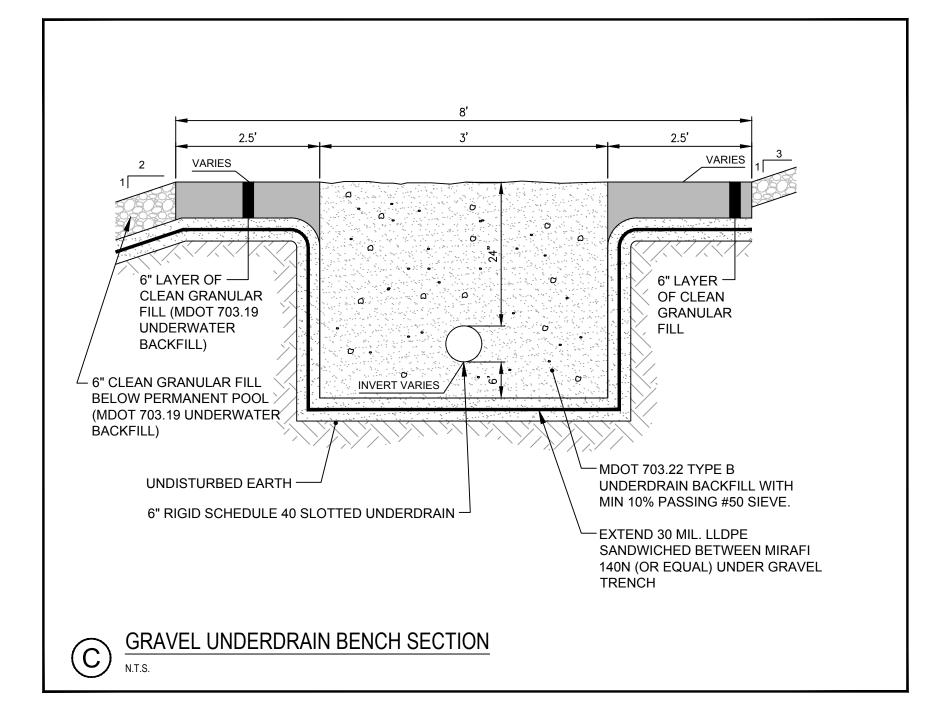


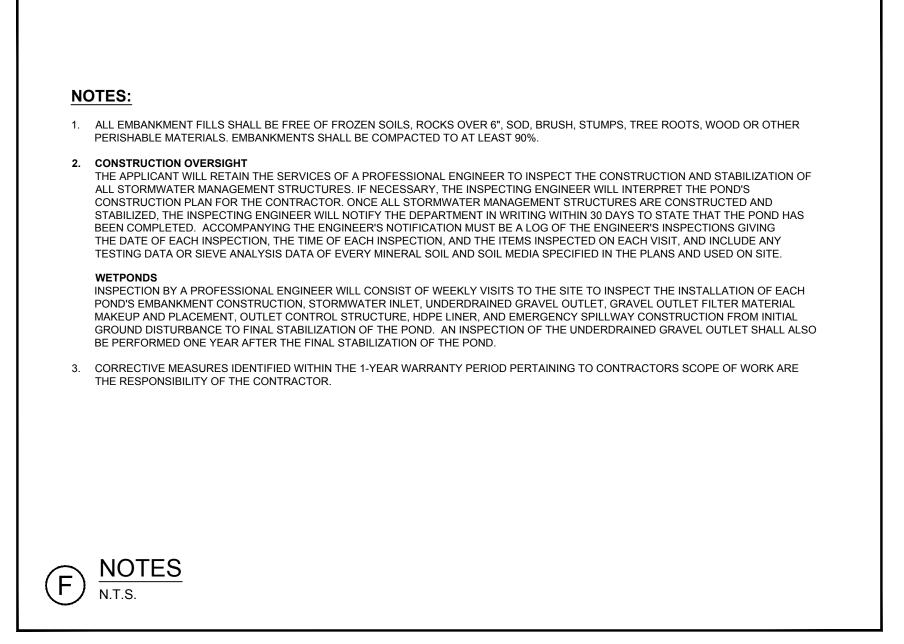


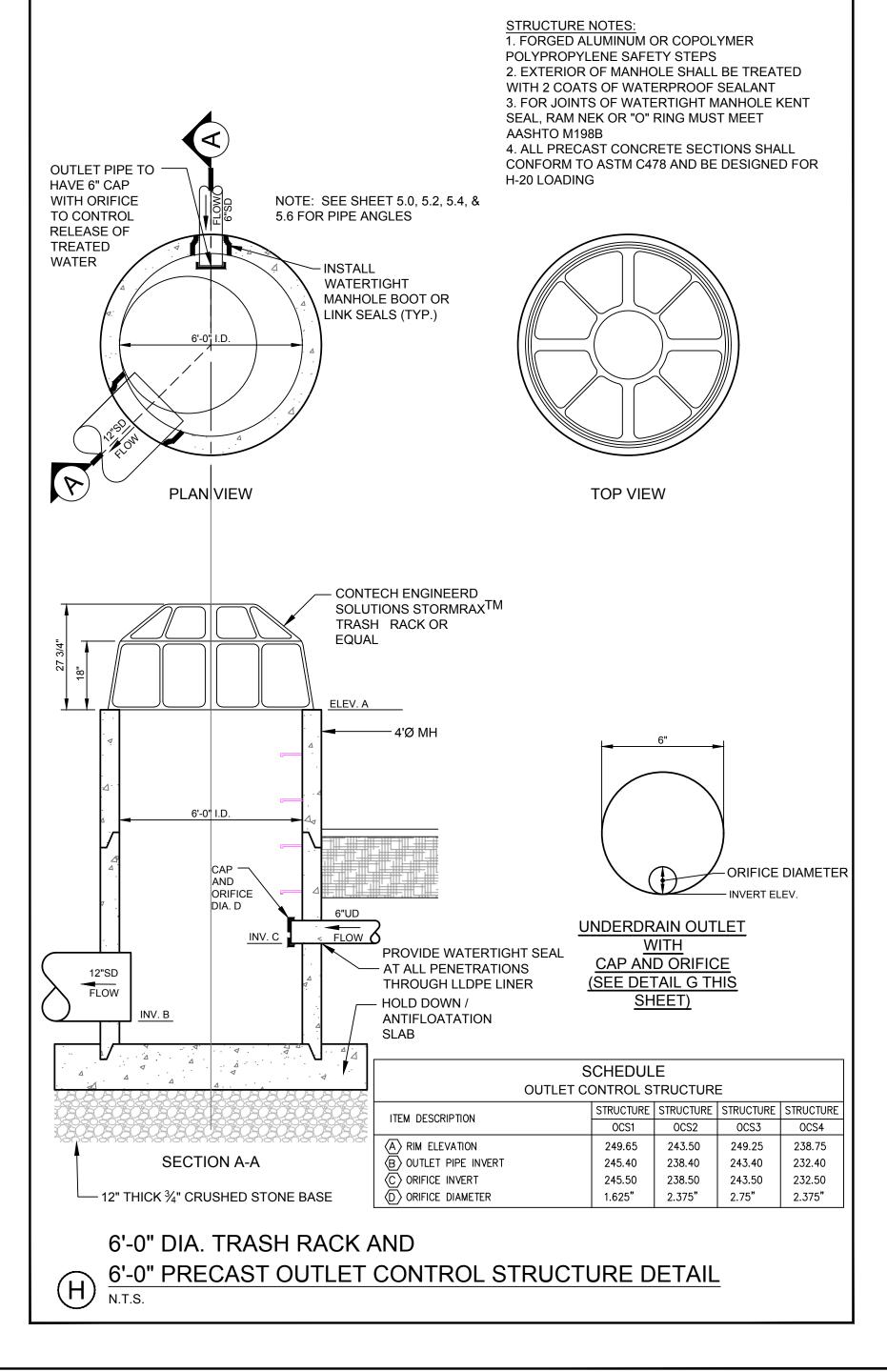














Stante 482 Par

482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

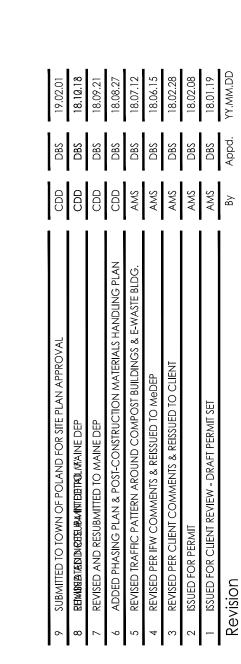
Consultants

St.Germain Collins

846 Main Street Westbrook, Maine 04092 Tel. 207.591.7000

authorized by Stantec is forbidden.

Notes



Permit-Seal

Darrin B.

1518
02.01.19
CENS

Client/Project

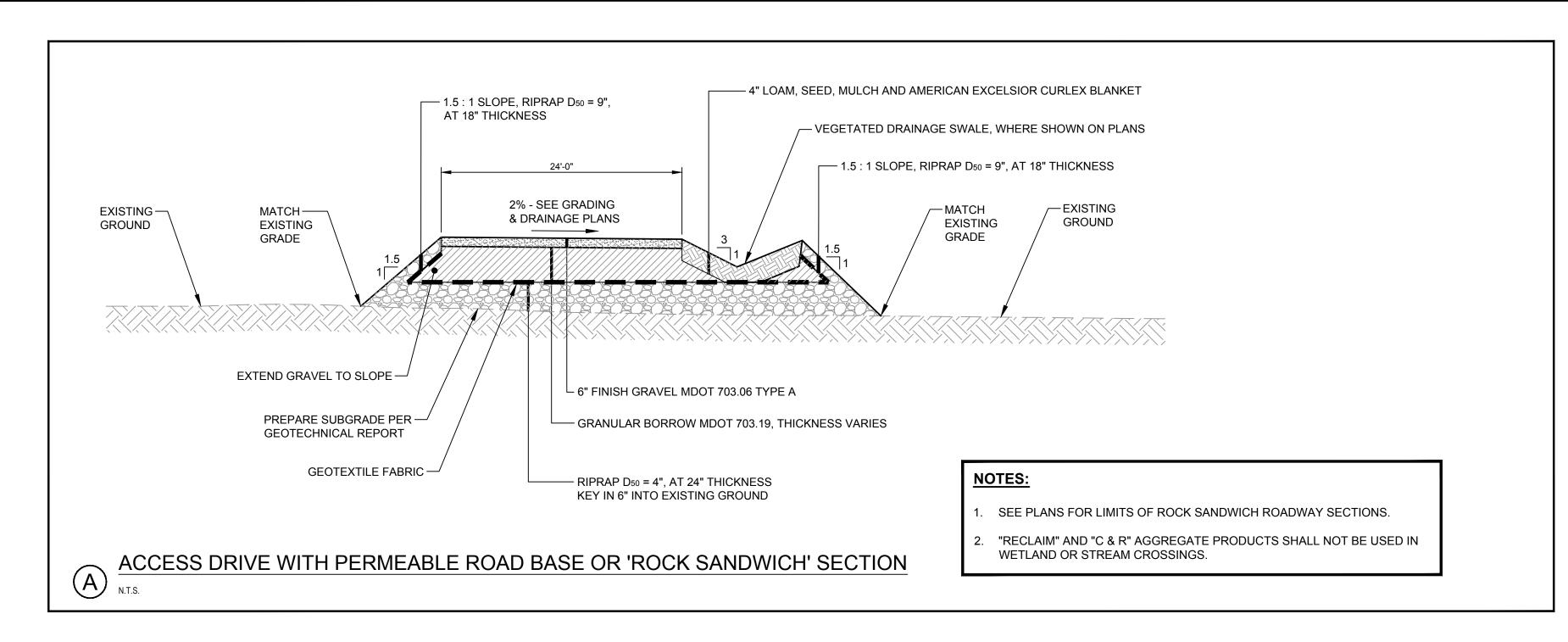
ST.GERMAIN COLLINS MB BARK, LLC

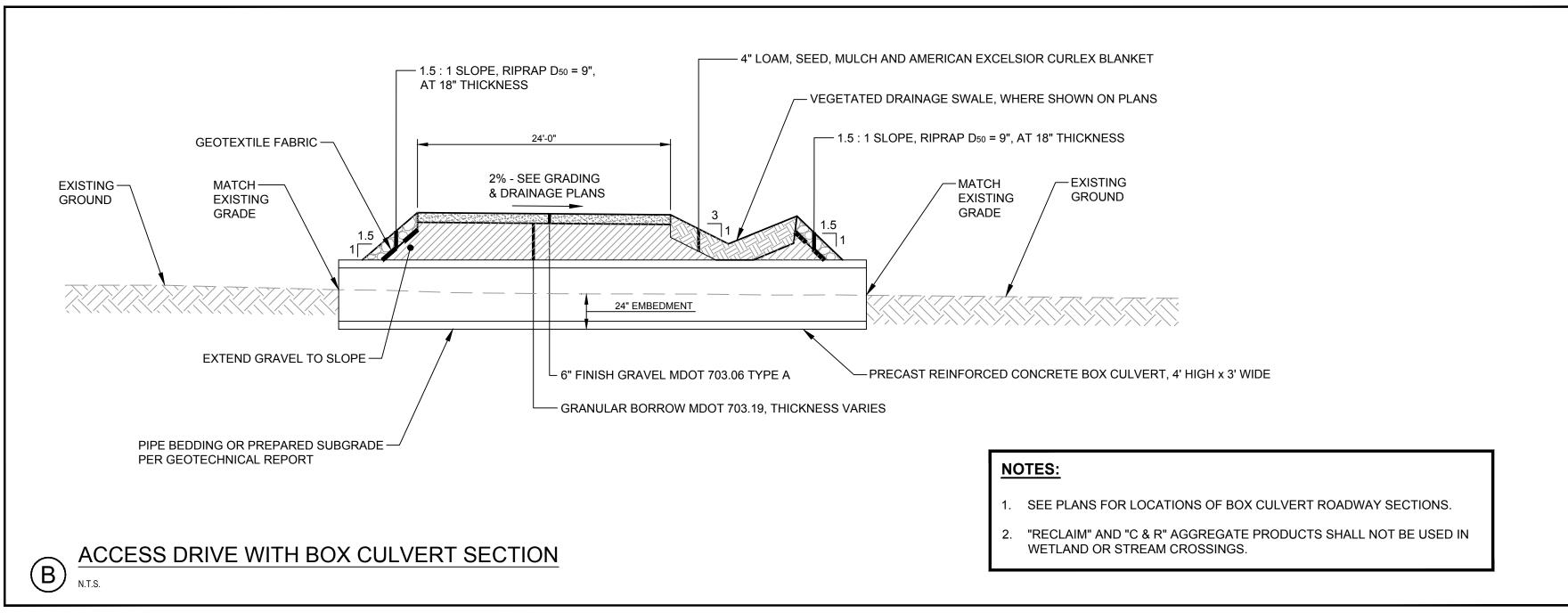
MB BARK RECYCLING FACILITY EXPANSION
AUBURN, MAINE / POLAND, MAINE

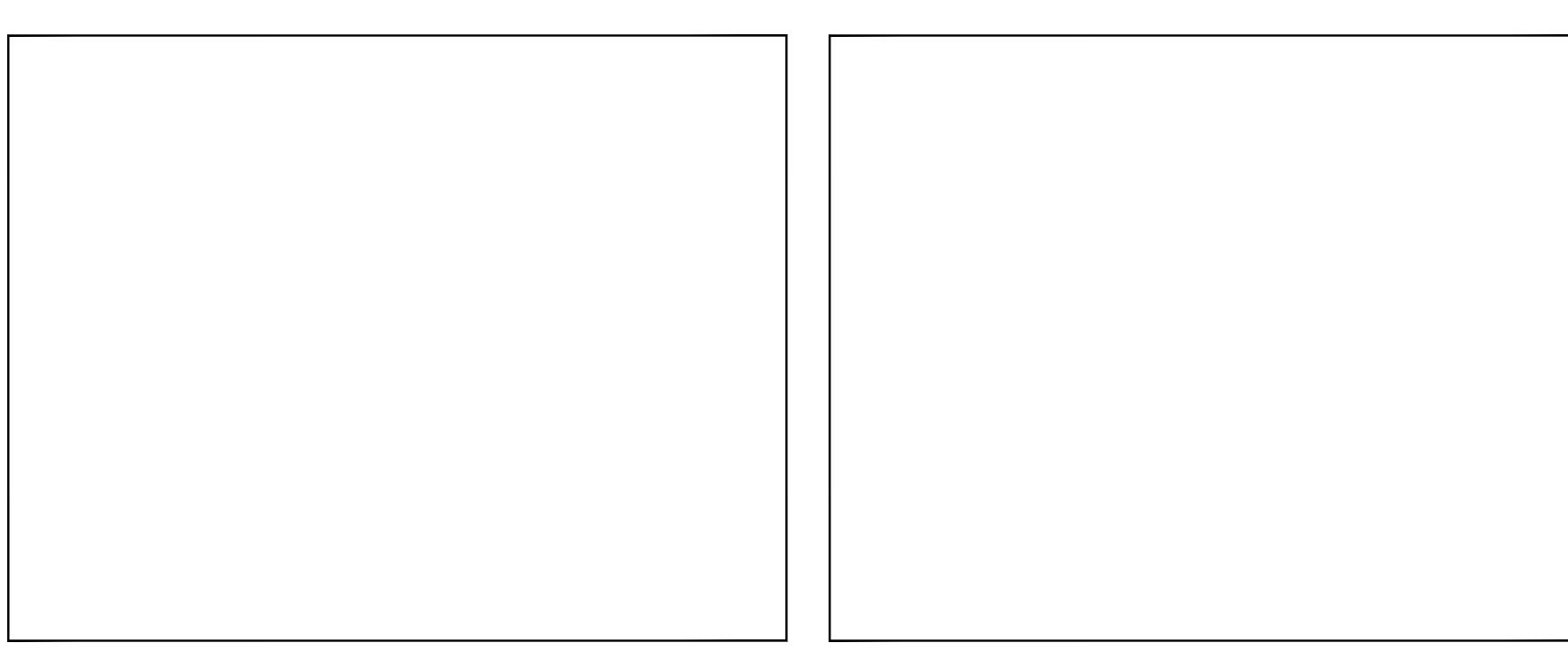
SITE DETAILS

Project No. Scale
195350465 N.T.S.
Sheet

C-7.3









Stanted

482 Payne Road Scarborough Court Scarborough, Maine 04074-8929 Tel. 207.883.3355 www.stantec.com

Copyright Reserved

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to Stantec without delay.

The Copyrights to all designs and drawings are the property of Stantec. Reproduction or use for any purpose other than that

Consultants

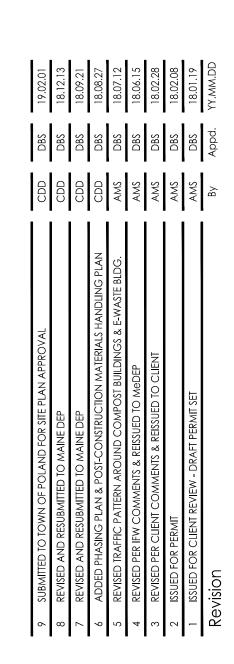
St Germain Collins

846 Main Street Westbrook, Maine 04092

Tel. 207.591.7000

authorized by Stantec is forbidden.

Notes



File Name: 50465_details

Darrin B.

Darrin B.

15118
02.01.19
CENSE

Client/Project

ST.GERMAIN COLLINS MB BARK, LLC

MB BARK RECYCLING FACILITY EXPANSION

AUBURN, MAINE / POLAND, MAINE

Title

Sheet

SITE DETAILS

Project No. Scale 195350465 N.T.S.

V:\1953\active\195350465\civil\cadd\permit\dwg\50465_details.c

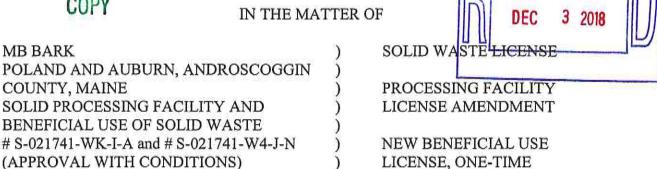
C-7.4



STATE OF MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION STATE HOUSE STATION 17 AUGUSTA, MAINE 04333

OFFICE COPY

DEPARTMENT ORDER



Pursuant to the provisions of the Maine Hazardous Waste, Septage, and Solid Waste Management Act, 38 M.R.S. §§ 1301-1319-Y, the Department's Solid Waste Management Regulations, General Provisions, 06-096 C.M.R. ch. 400 (last amended April 6, 2015), Transfer Stations and Storage Sites for Solid Waste, 06-096 C.M.R. ch. 402 (last amended April 12, 2015), Water Quality Monitoring, Leachate Monitoring, and Waste Characterization, 06-096 C.M.R. ch. 405 (last amended April 12, 2015), Processing Facilities, 06-096 C.M.R. ch. 409 (last amended July 27, 2014), Beneficial Use of Solid Wastes, 06-096 C.M.R. ch. 418 (last amended July 8, 2018), and the Department's Stormwater Management Rules, 06-096 C.M.R. ch. 500 (last revised August 12, 2015), the Department of Environmental Protection ("Department") has considered the application of MB BARK ("MB Bark") with its supportive data and other related materials on file and FINDS THE FOLLOWING FACTS:

1. APPLICATION SUMMARY

- A. <u>Application</u>: MB Bark has submitted an application for amendment of its processing facility license, Department license # S-021741-WK-A-N, issued January 30, 1998. The application includes a request for after-the-fact approval of changes made at the facility prior to submission of the application. MB Bark has also submitted an application for one-time beneficial use of secondary material as construction fill.
- B. <u>History</u>: Department license # S-021741-WK-A-N was issued to Morse Brothers, Inc. on January 30, 1998, and approved construction and operation of a processing facility for the manufacture of bark mulch. The license was appealed and upheld with additional conditions related to noise on November 21, 1998. On February 28, 2005, Morse Brothers, Inc. received a license revision to allow storage of short paper fiber from Cascades-Auburn Fiber. The licenses were transferred from Morse Brothers, Inc. to MB Bark on February 17, 2006 (Department license # S-021741-WX-C-T), following the Chapter 11 bankruptcy of Morse Brothers, Inc. On July 27, 2006, MB Bark received approval of a minor revision application (Department license # S-021741-WK-D-M); this license approved regrading the site to improve drainage, paving much of the waste

MB BARK	2	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

handling area and perimeter road, and acceptance and storage of biomass fuel materials. MB Bark received approval of another license minor revision (Department license # S-021741-WK-E-M, dated September 27, 2007), permitting acceptance and storage of inert material and old corrugated containers ("OCC"). MB Bark received a permit-by-rule (Department license # S-021741-WL-F-P, issued November 20, 2007) for the use of emulsified asphalt encapsulated oil contaminated soil as construction fill. On October 27, 2011, MB Bark received another license minor revision (Department license # S-021741-WK-G-M); this revision approved acceptance and processing of creosote treated wood for making boiler fuel. MB Bark received a license (Department license # S-021741-CG-H-N, dated March 19, 2013) for a compost facility for Type 1A and 1B residuals.

C. Summary of Proposal: MB Bark now requests approval to amend the processing facility license to expand the facility, and accept and process asphalt shingles. MB Bark also requests after-the-fact approval of: acceptance and storage of panel glass from processors of cathode ray tubes ("CRT glass"); grading changes; expansion of the facility; changes to the extent of pavement at the site; and acceptance and processing of wood pallets. MB Bark also requests approval of one-time beneficial use of secondary materials as construction fill.

2. PROJECT DESCRIPTION

The site is currently arranged inside a perimeter access road. A wetland lies inside the perimeter road on the northwestern side of the site. The remainder of the area inside the perimeter road is developed for buildings, material handling and storage, and includes 6 stormwater management ponds, 3 of which are intended to provide stormwater treatment in addition to detention. One stormwater detention pond and 2 ponds intended to provide stormwater treatment are located outside the perimeter road. The site also includes a gatehouse, office building, truck scales, a 10,000-gallon diesel above ground storage tank, and parking areas. MB Bark currently accepts wood wastes and pallets, creosote treated wood, CRT glass, inert materials, and Types 1A and 1B organic materials. It is also licensed to accept short paper fiber from Cascades-Auburn Fiber, and OCC. MB Bark has accepted and stored utility poles for a contractor needing a local storage area in the past, but has stated it will no longer accept this material. Wood wastes and bark are processed into mulch and erosion control mix, and sold. Creosote treated wood is processed into fuel chips and sold to a facility licensed to use it as a substitute fuel. CRT glass is sent to the CPRC processing facility in Scarborough ("CPRC") for processing and use. Short paper fiber from Cascades-Auburn Fiber has been stored on site for up to

MB BARK	3	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

one year before sending off site for utilization; although Cascades-Auburn Fiber is no longer in business, MB Bark wants to maintain the ability to accept this waste. Types 1A and 1B materials are composted, and the compost is sold. Inert materials (soil, sand, rock, brick, concrete, porcelain, sand from winter sand cleanup, and non-CRT glass) have been processed on site into loam and base aggregates, and sold; MB Bark recently revised its application to say it no longer intends to crush inert materials on site, but will send them to CPRC for processing. Loam, sand, and compost are used to manufacture topsoil, which is sold. MB Bark has stated that it no longer intends to accept OCC or utility poles. MB Bark proposes to accept and store manufactured shingle tear-offs ("asphalt shingles"), grind them on site, and sell the product to hot mix plants or send it to CPRC for use in products.

MB Bark proposes to increase the size of the originally licensed facility (hereafter referred to as the "existing site") and to develop two new storage pads, pads 2 and 3, on a different part of the property (hereafter referred to as the "new area"). An access road will be constructed to the new area from the existing site. Since the facility was first licensed, the existing site's waste handling area has been extended outward. MB Bark requests after-the-fact approval for expansion that has already occurred, and approval for additional expansion.

As originally licensed, the facility's waste handling area encompassed 50.3 acres, of which approximately 38.5 acres is usable for waste handling activity; after completion of phases 1, 2, and 3, it will be 74.8 acres, of which 66.6 acres will be at the existing site and 8.2 acres at the new area. The new access road will add about 1.4 acres of developed area. Approximately 20 acres of the existing site will be regraded. Stormwater management features have been proposed to handle additional runoff. Department license # S-021741-WK-D-M approved paving a large material storage and handling area on the existing site, as well as the perimeter road; these areas have not been paved, and MB Bark proposes to leave them unpaved.

A portion of one on site building is leased to another company and used for truck maintenance, and another is leased to an electronics de-manufacturer. MB Bark also proposes to construct a rail spur in the future, after completing the construction proposed in this amendment application. Design information for the rail spur has not yet been provided.

MB Bark estimates 269,480 cy of fill (compacted volume) will be needed for the proposed expansion and regrading; approximately 2/3 of this volume will be used to regrade the existing site. Some of the fill volume (approximately 22,862 cy) will be

MB BARK	4	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

supplied from cut material on site. MB Bark estimates 283,611 cy of new material will be needed (before compaction), and proposes to use secondary materials consisting of 36,417 cy of reclaim manufactured by CPRC, 26,833 cy of modified reclaim manufactured by CPRC, and 220,361 cy of C&R manufactured by CPRC. Reclaim is a mixture of emulsified asphalt encapsulated contaminated soils mixed with other solid wastes. Modified reclaim is made with soils that meet the 06-096 C.M.R. ch. 418, § 1definition of petroleum contaminated soil, encapsulated with emulsified asphalt. C&R is made of inert materials, and may be mixed with ground asphalt shingles and with ground CRT glass; if the asphalt shingles and/or CRT glass are added, the C&R is not an inert material. CPRC holds licenses allowing it to manufacture these products, although as of September 1, 2018, it ceased manufacturing reclaim. Some reclaim previously manufactured remains stockpiled at the CPRC site. Reclaim and modified reclaim are hereafter referred to as "reclaim".

MB Bark has divided the development into 3 phases of construction. Phase 1 will include all work at the existing site, referred to as pad 1 and the runway area, with associated stormwater management features. Phase 2 will include the new access road and pad 3, with its associated stormwater management features. Phase 3 will include pad 2 and its stormwater management features. Phase 1 is expected to be completed in the first year of construction. Phases 2 and 3 are expected to be completed in the second year of construction. The application states construction will not be continuous, but in total will take 4000 hours from 4 employees over the two-year period.

VARIANCE REQUEST

MB Bark requests a variance to the provisions of 06-096 C.M.R. ch. 409, § 2(A)(3), which prohibits locating the waste handling area at a processing facility within 100 feet of a protected natural resource. The proposed waste handling area is within 100 feet of wetland areas in several locations. Wetland mapping revealed that the western side of the existing site is also within 100 feet of wetlands. The proposed access road to the new area will result in some filled wetlands, due to the need to cross wetlands in 3 locations and cross a stream and a wetland in 1 location; in addition, wetlands will be filled in 2 locations for pads 2 and 3. MB Bark has received a permit under the Natural Resources Protection Act (Department license numbers # L-19467-TG-C-N and # L-19467-D-N, dated June 19, 2018), and has received approval from the U.S. Army Corps of Engineers for the proposed wetland and stream alterations. The Department finds that MB Bark has presented clear and convincing evidence that its proposal will be compliant with State law and the purpose and intent of 06-096 C.M.R. ch. 409, § 2(A)(3).

MB BARK	5	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

BENEFICIAL USE

Reclaim and C&R will be used to provide the majority (approximately 90%) of the fill needed for the project. Analysis of reclaim and C&R by MB Bark's geotechnical engineering consultant showed that the material will perform as a substitute for virgin construction fill material. CPRC's licenses do not permit it to accept hazardous waste. MB Bark has proposed to meet erosion and sedimentation control best management practices. General licensing criteria are addressed in subsequent Findings of Fact in this license. Reclaim and C&R made with asphalt shingles may potentially exceed some of the screening standards in 06-096 C.M.R. ch. 418, appendix A. CRS, CPRC's predecessor in Scarborough, performed evaluations of the risk posed by reclaim and C&R, and proposed risk management measures to limit risk; the risk assessment information was included in MB Bark's beneficial use application by reference. The risk management measures proposed include limiting the percentage of shingles in C&R; annual analysis of shingles for benzo[a]pyrene and the total metals arsenic, barium, cadmium, chromium, mercury, lead, selenium, and silver; limiting use of reclaim to nonresidential settings; and covering reclaim and C&R containing CRT glass with a concrete or asphalt paved surface or 6 inches of compacted soil. MB Bark's application shows that the enlarged existing waste handling area and the new waste handling areas will be covered with a 6-inch layer of compacted gravel. Department staff ("staff") comments that the 6-inch compacted gravel layer should not contain secondary materials. In addition, the 2017 cut and fill plan states that a portion of the fill volume will be soil and gravel cover for the waste storage and handling areas. These changes were not accounted for in MB Bark's estimate of the volume of waste derived product to be used. Staff comments that this approval is to construct the facility to the proposed grades in MB Bark's proposed 2-year timeframe, and that if there is insufficient secondary material to complete the work in the specified time frame, appropriate virgin material will need to be used, unless otherwise licensed by the Department. In addition, if MB Bark has access to or takes delivery of more secondary material than is needed, or the volume estimates are too high, the additional unneeded material must be removed when construction is complete.

MB Bark has stated that it will file a deed notice and restriction as described in 06-096 C.M.R. ch. 418, § 10(B) and (C). Staff comments that a draft of the notice and restriction should be submitted to the Department for review and approval prior to the start of construction, which includes: grubbing; blasting; excavation; delivering C&R or reclaim to the site; placement of fill; regrading; or moving fill, stockpiled C&R, or reclaim. The notice and restriction should be filed at the Androscoggin County Registry of Deeds

MB BARK	6	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

within 60 days of Department approval, and a copy of the filed information, including book and page number, should be provided to the Department as soon as it is available.

MB Bark's proposal included use of C&R and reclaim as fill wherever fill was needed for the project. Plan sheet C-3.2 in MB Bark's application shows an area for storage of C&R after completion of construction. Staff comments that the application materials do not address storage or distribution of C&R. Staff comments that MB Bark will need a beneficial use license to distribute C&R, but this has not been addressed in this application. Staff comments that any C&R or reclaim remaining on site after completion of phases 1, 2 and 3 should be removed within 30 days of completion of construction and returned to CPRC or disposed in a landfill, unless otherwise licensed by the Department. No additional C&R or reclaim may be stored on site until MB Bark has a beneficial use license to distribute it.

An application for beneficial use of solid waste must demonstrate that the proposed beneficial use is not disposal or a means of discard. MB Bark provided a submittal on October 5, 2018 that detailed its need for additional storage, handling, and processing areas. Briefly, MB Bark expects demand for its bark mulch, creosote treated wood chip, and loam products to increase over the next 3-5 years, and states it will need more space to accommodate more material. In addition, MB Bark expects future demand for recycled aggregate to increase, and has included storage area for C&R on pad 1. Pads 2 and 3 will be needed for the proposed new activity of shingle storage and processing. Pad 3 will include areas for: storage of incoming shingles for 3 days while awaiting asbestos analytical results; storage of post-test shingles; storage of ½ minus ground shingles; storage of inert material; and allowance for access and processing equipment. Pad 2 includes area for: a feed stockpile of ½ ground shingles; a pile of ¼ minus ground shingles destined for hot mix plants; an enclosed container for storage of asbestos-containing material in case any is detected; and space for a loadout ramp, access, and processing equipment.

Since the quantity of secondary material used as fill is determined by both area and depth, MB Bark's design consultant, Stantec Consulting Services, Inc. ("Stantec"), provided a discussion of how it arrived at the proposed grades. Factors considered included: roadway grades and widths appropriate for operation of heavy equipment and trucks; keeping stormwater conveyances at the surface to minimize maintenance needs; keeping stormwater treatment facilities to a manageable size; providing for sufficient elevation change to allow stormwater to flow to the ponds and to outfall from the ponds; appropriate slope of the working surface of pads 1, 2, and 3; avoidance of wetlands and vernal pool buffer areas; and desire to connect smaller working areas.

MB BARK	7	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

MB Bark's justification of the size of the area needed included existing and proposed specific acreage needs for various materials. MB Bark also provided a post-development plan showing the location of the various material handling and processing areas. Staff comments that there are discrepancies between the sizes of the areas on plan sheet C-3.2 (titled "Post-Construction Materials Handling Plan", last revised September 21, 2018) and the proposed sizes contained in MB Bark's October 5, 2018 submittal explaining the need for more handling and processing area at the site. Staff comment that sheet C-3.2 will need to be revised to match the area sizes described in MB Bark's October 5, 2018 letter.

The Department finds that use of reclaim, modified reclaim, and C&R as construction fill for this project, as described in the application, is a beneficial use, provided: the 6-inch compacted gravel layer covering all construction fill does not contain secondary materials; a draft of the deed notice and restriction is submitted to the Department for review and approval, and the finalized document is filed at the Androscoggin County Registry of Deeds within 60 days after Department approval, and a copy of the filed information, including book & page number, is provided to the Department as soon as it is available; any C&R or reclaim remaining on site after completion of phases 1, 2 and 3 is removed within 30 days of completion of construction and returned to CPRC or disposed in a landfill, unless otherwise licensed by the Department; no additional C&R is stored on site until MB Bark has a beneficial use license to distribute it; and application drawing sheet C-3.2 is revised and resubmitted to show material storage, handling, and processing areas with sizes that match the area sizes MB Bark states it needs in the October 5, 2018 letter.

TITLE, RIGHT, OR INTEREST

The property is owned by MB Investment Properties, LLC and leased to MB Bark, LLC. A copy of the lease was provided. The Department finds that MB Bark has provided adequate evidence of title, right, or interest in the property.

FINANCIAL CAPACITY

MB Bark estimates the cost of the project to be \$403,000, including the wetland compensation fee, blasting, tree clearing, materials, construction labor and construction oversight. Design expenses have already been paid. Material costs are low because MB Bark will not need to buy fill. MB Bark is a subsidiary of CPRC, who will supply the reclaim, modified reclaim, and C&R without charging MB Bark. Costs will be paid out

MB BARK	8	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

of operational cash flow and, if necessary, through working capital that MB Bark has access to through the Bank of America. Staff comments that the cost estimate does not include environmental monitoring, additional construction oversight required in this approval, additional permitting costs to date, cost of a noise assessment, or a contingency factor. However, the amount of working capital MB Bark has access to is well in excess of the cost estimate provided, so staff recommends approval regardless. The Department finds that MB Bark has provided evidence of availability of sufficient funds to complete the proposed amendment and operate the facility in a manner consistent with state environmental standards.

TECHNICAL ABILITY

MB Bark has worked with 3 consultants: St.Germain Collins, Stantec, and Summit Geoengineering Services, Inc. ("Summit"). St.Germain Collins provided permitting and project management services; the 2 individuals involved have over 30 years combined permitting and oversight experience with solid waste facilities. Stantec provided design and engineering services, wetland mapping, surveyed the site for rare plants, and provided permitting services under the Natural Resources Protection Act and with the U.S. Army Corps of Engineers. The 2 professional engineers leading Stantec's team have over 40 years of combined experience with civil and environmental engineering projects including Department solid waste facility permitting. Summit provided geotechnical support. Summit's professional engineer has 30 years of experience with geotechnical, construction materials, civil, and structural engineering, and site permitting.

Construction will be performed by MB Bark and CPRC personnel using company-owned equipment. The employees are experienced in use of heavy equipment, and have performed grading and stormwater treatment structure construction at the MB Bark site since 2007, including construction of the MB Bark compost facility in 2013. MB Bark's operations are overseen by its General Manager, who has been with MB Bark since the change in ownership in 2006. Summit and a professional engineer from St.Germain Collins will provide construction oversight and post-construction certification. MB Bark states that construction will occur sporadically, as its employees have time to do it, rather than continuously, and proposes construction oversight consisting of weekly inspections by a professional engineer during times of active construction. Staff comments that the previous regrading effort at the site resulted in overfilling and unlicensed expansion, and recommends frequent inspections by a professional engineer during all times that construction is occurring. Staff also comments that no design information was provided for the rail spur that MB Bark intends to construct in the future, and that design

MB BARK	9	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

information should be provided to the Department for review and approval prior to construction of the rail spur.

The Department finds that MB Bark has submitted adequate evidence of technical ability to design, construct, operate, and maintain the facility in accordance with state environmental requirements and its permits provided:

- a professional engineer conducts weekly inspections during all times that construction is occurring;
- weekly reports are submitted electronically to the Department each Tuesday for the preceding week:
 - when construction occurred, the report must include personnel working on the project, a description of work completed, the type and quantity of material moved or placed, and engineering inspection reports; and
 - when no construction occurred, the report may be an email documenting there was no construction activity, from anyone with knowledge of the construction activities;
- clearing limits and earthwork limits are marked in the field by a surveyor or professional engineer prior to undertaking any clearing or earthwork and the markings are maintained until the Department concurs the construction was completed as licensed;
- areas requiring construction, as specified in the application materials, may not be
 used until a professional engineer has certified construction is complete and was
 done as approved by the Department and the Department has accepted this
 certification; and
- rail spur design information must be provided to the Department for review and approval prior to construction of the rail spur.

DISCLOSURE

MB Bark submitted a complete civil and criminal disclosure statement. The Department finds no reason to withhold this license based on civil or criminal record.

TRAFFIC MOVEMENT

The original license for the facility stated that up to 160 truck trips and 100 passenger vehicle trips were expected per day. MB Bark provided 2017 traffic count information for the MB Bark facility, which showed an average of 19 truck trips and 13 other vehicle trips per day, and a maximum of 69 truck trips and 13 other vehicle trips per day. An

MB BARK	10	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

increase in traffic is expected due to the new waste stream proposed to be accepted (shingles); MB Bark provided traffic count data from the CPRC facility in Scarborough, where shingles as well as other wastes are currently accepted. All of the CPRC shingle traffic is expected to go to MB Bark once it begins accepting shingles. Average trips per day at CPRC for all wastes was 36; the highest day was 99. The average number of trips per day in and out of MB Bark is expected to be 68, and the maximum less than or equal to 181. This is less than the 260-maximum expected in the original facility license. Haul route information was provided; roads with weight limits or congested locations are not proposed to be used. No high accident locations were identified at nearby intersections. The facility is open 5 days per week except during busy times of the year, when it is open 7 days per week. Interior roads are 2-way, gravel-surfaced, and 24 feet wide. Traffic circulation patterns were identified on facility drawings. The Department finds that MB Bark has made adequate provisions for safe and effective traffic movement into, out of, and within the proposed facility.

10. FITTING HARMONIOUSLY INTO THE ENVIRONMENT

MB Bark proposes expansion of the existing site and proposes new development in a currently wooded part of the site, to the west of the existing site. Stantec mapped wetlands and surveyed the property for rare, threatened, or endangered species. A deer wintering area was identified to the north and northwest of the new area. A perennial stream and 3 wetlands are located to the northeast of the existing site. An intermittent stream and 8 wetlands are located to the west of the existing site, 1 wetland is northwest of the existing site, 3 wetlands are south of the existing site, and 1 wetland exists in the interior of the existing site. Several vernal pools, none of which were significant vernal pools, were mapped to the west and south of the existing site. Of the 15 identified wetlands, 7 are wetlands of special significance. One threatened plant species, Swamp White Oak, was found to the northeast of the existing site. New and expanded waste handling areas will be at least 100 feet from property lines. Approximate clearing limits are shown on the project plans. Areas outside the clearing limits are proposed to be left undisturbed. Finding of Fact #3, above, describes MB Bark's request for a variance to the requirement for a 100-foot setback to protected natural resources. Staff comments that the western and southwestern edge of the existing site, where the railroad spur is proposed to be constructed, is already within 100 feet of wetlands. Grading for the railroad spur has not yet been proposed, but the revised waste handling area boundary is at the edge of fill on this side. Staff recommends that no additional expansion occur in this area.

MB BARK	11	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

The Maine Natural Areas Program ("MNAP") recommended no expansion or disturbance occur within or adjacent to the forested riparian areas where Swamp White Oak was found, and that a 75-foot no disturbance buffer be maintained between Swamp White Oaks and the erosion control mix berm at the edge of the proposed earthwork area.

The Maine Department of Inland Fisheries and Wildlife ("MDIFW") commented that only a small portion of the proposed development would intersect with the deer wintering area and that therefore it would not be significant to the deer population, and that minimal impacts to wildlife are expected as long as vernal pools are avoided as described in the application. MDIFW recommends 100-foot riparian buffers be maintained between intermittent and perennial streams and project disturbances in order to protect habitat for wild brook trout. Part of pad 3 would have encroached into this buffer, so MB Bark revised its design to include the buffer, and submitted revised drawings (sheets C-4.4 and C-4.5, revised September 21, 2018).

The proposed new access road to pads 2 and 3 will cross wetlands, as described in Finding of Fact #3, above. The road at these locations has been designed to maintain hydraulic connectivity of both sides of crossed wetlands.

The Department finds that the proposed new and expanded areas will fit harmoniously into the natural environment provided that: no additional expansion at the west and southwest edge of the developed site; no disturbance (including mowing) occurs within 75 feet of Swamp White Oaks; and a 100-foot riparian buffer is maintained between intermittent and perennial streams and project disturbances.

11. EXISTING USES, SCENIC CHARACTER

The expanded facility will extend no closer to the nearest airport, the Auburn Regional Airport. An historical site was identified when the processing facility was first licensed; archaeological work at that site has concluded. The Maine Historic Preservation Commission ("MHPC") required a Phase I prehistoric archaeological survey be performed in the western part of the property, where the new area is proposed. The survey was completed, and no archaeological sites were found. MHPC found that no historic or archaeological properties would be affected. MB Bark's application states that the facility will not unreasonably interfere with views from established public viewing areas because forested buffers will be retained around the new area (pads 2 and 3). Adjacent properties are zoned rural residential or farm/forest. The existing site has been a processing facility for approximately 20 years. MB Bark states that it has implemented various practices to reduce the impact on residential neighbors, including planting

MB BARK	12	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

vegetation and limiting equipment noise. Noise readings were taken from a shingle grinder in operation at CPRC to assess potential noise levels at MB Bark. A noise level of 75 dBA was recorded at a distance of 250 feet from the grinder; MB Bark proposed to locate the shingle grinder at least 300 feet from the property line to ensure noise would not exceed 75 dBA at the property line.

Staff comments that 06-096 C.M.R. ch. 400, § 4(F)(2)(a)(ii) limits noise levels to 60 dBA for daytime hours and 50 dBA for nighttime hours at any protected location in an area for which the zoning is not predominantly commercial or industrial. A "protected location" is defined in 06-096 C.M.R. ch. 400, § 1(Ii) as any location within a parcel of property that contains a residential subdivision or a residence. The noise limit at a protected location effectively applies up to the property line. A residential subdivision is located on the adjacent parcel across the property line that would be 300 feet from the shingle grinder; since the noise readings provided by MB Bark showed a noise level of 65 dBA at 500 feet from the shingle grinder, the limit of 60 dBA at a protected location would be exceeded. Staff comments that the noise survey performed when the processing facility was first licensed only included bark processing equipment. Since then, other noise sources have been present at the site, including the grinder that processes rail ties and pallets. Staff comments that noise from all sources must be evaluated and an estimate of maximum daytime and nighttime noise levels at all nearby protected locations must be provided. Staff comments that, at this time, MB Bark has not shown its operations will meet the noise standard in 06-096 C.M.R. ch. 400, § 4(F)(2)(a)(ii). Staff recommends an evaluation of all non-exempt noise sources at the facility (existing and proposed) be performed by a qualified consultant, and an estimate of the maximum daytime and nighttime noise levels at all nearby protected locations be submitted to the Department for review and approval prior to beginning any work associated with this application. Staff also recommends follow-up noise level monitoring after construction, during operation of all non-exempt equipment, to verify that actual noise levels meet the regulatory standard. Staff comments that, if actual noise levels exceed the standard, the activity producing the noise will need to cease until MB Bark demonstrates it can meet the standard.

The Department finds that MB Bark's proposed expansion and new development will not present a bird hazard to aircraft, have an unreasonable adverse effect on the preservation of historical sites, or unreasonably interfere with views from established public viewing areas. The Department finds that MB Bark has not demonstrated its noise levels will meet the standards in 06-096 C.M.R. ch. 400, § 4(F)(2)(a)(ii). The Department further finds that an evaluation of all non-exempt noise sources at the facility (existing and proposed) must be performed by a qualified consultant, and an estimate of the maximum

MB BARK	13	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

daytime and nighttime noise levels at all nearby protected locations must be submitted to the Department for review and approval prior to beginning any work associated with this application, and follow-up noise monitoring must be performed by a qualified consultant after construction, during operation of all non-exempt equipment, to verify that actual noise levels meet the standards in 06-096 C.M.R. ch. 400, § 4(F)(2)(a)(ii). In the event noise levels exceed the standards in 06-096 C.M.R. ch. 400, § 4(F)(2)(a)(ii), MB Bark must cease use of the equipment causing the exceedance until it demonstrates it can meet the standards.

12. AIR QUALITY

MB Bark states that no air emissions licenses are required for the facility, and there will be no new sources of odors. Dust may be produced by processing equipment, from material stockpiles, or due to travel on facility roadways and storage areas. MB Bark proposes to use a water truck as necessary on roadways and storage areas. Processing equipment typically includes spray bars to control dust during processing. Dust from material stockpiles will be controlled by wetting. The facility operations manual includes dust control measures. The Department finds that the processing facility will not unreasonably adversely affect air quality.

SOIL SUITABILITY

MB Bark hired Summit to assess the suitability of site soils and proposed fill materials for the project. Summit excavated 6 test pits; 5 were in the area proposed for expansion of the existing site ("pad 1") and 1 was excavated into an area already filled with reclaim. Summit concluded that both the native soils and proposed fill materials would be suitable for use in constructing pad 1 and that the native soils would support the pad construction, pending determination of material conditioning, handling, placement, and compaction procedures. As proposed, pad 1 would have a fill slope 25 feet high at the thickest point. Summit next performed a global stability analysis and concluded that the embankment would have an acceptable factor of safety, assuming its pad construction recommendations were followed. Summit expects its evaluations and recommendations for pad 1 will be applicable to pads 2 and 3, but states that subsurface investigations in the pads 2 and 3 area will need to be done to confirm this. Additional information on the subgrade conditions in the proposed roadway area will also be needed to determine subgrade preparation needed and type and thickness of materials needed.

Staff reviewed the geotechnical reports and commented in a memo dated August 21, 2018 that additional information is needed. Staff recommends that no material

MB BARK	14	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

movement or placement for phase 1 construction occur until all Department concerns regarding soil suitability and site stability have been resolved. In addition, staff recommends that subsurface investigations be performed in the area of the proposed road and pads 2 and 3, and that a geotechnical report be submitted to determine what material conditioning, handling, placement, and compaction procedures will be needed, and to determine road subgrade preparation needed and type and thickness of materials needed for road construction. The report must be submitted for review and approval, and no earthwork or material movement or placement may be done for the phase 2 or 3 construction (road, or pads 2 or 3) until the Department has approved commencement of construction.

The Department finds that MB Bark has not yet demonstrated the expansion or the new development will be constructed on suitable soils. The Department finds that no material movement or placement for phase 1 construction may occur until all Department concerns, as detailed in the staff memorandum dated August 21, 2018, regarding soil suitability and site stability have been resolved. The Department also finds that subsurface investigations must be performed in the area of the proposed road and pads 2 and 3, and that a geotechnical report must be prepared describing what material conditioning, handling, placement, and compaction procedures will be needed, and to determine road subgrade preparation needed and type and thickness of materials needed for road construction. The report must be submitted for review and approval, and no earthwork or material movement or placement may be done for the phase 2 or 3 construction (road or pads 2 or 3) until the Department has approved commencement of construction.

14. STORMWATER, FLOODING, AND EROSION AND SEDIMENTATION CONTROL

Stantec prepared a stormwater management report for the project. Peak stormwater discharge was evaluated for the 2, 10, and 25 year, 24-hour storm events. Stormwater management features were incorporated into the design to direct and detain flow as needed. Four wet ponds are proposed to provide water quality treatment. Staff reviewed the report and comment that stormwater quality and quantity will be improved following completion, and that if constructed as proposed, no downstream flooding will result.

The project is not located in a 100-year floodplain.

The application states that temporary and permanent erosion control measures are proposed as described in the Maine DEP Erosion and Sediment Control BMP manual. Erosion and sedimentation control notes are included on drawings prepared by Stantec

MB BARK	15	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

for the project. MB Bark will be responsible for maintenance of permanent stormwater conveyance and treatment systems. The application states that inspection, maintenance, and housekeeping will comply with Appendix B of 06-096 C.M.R. ch. 500. Staff comments that the applicant must comply with the provisions in the most recent versions of the BMPs, which are the Maine Erosion and Sediment Control Practices Field Guide for Contractors, March 2015, and the Maine Erosion and Sediment Control Best Management Practices (BMPs) Manual for Designers and Engineers, October 2016.

The Department finds that the processing facility will not unreasonably cause or increase flooding onsite or on adjacent properties nor create an unreasonable flood hazard to a structure, and that MB Bark has made adequate provisions for erosion and sedimentation control, provided it implements, inspects, and maintains temporary and permanent erosion control measures as described, and provided MB Bark, its agents, and its consultants comply with the provisions in the most recent versions of the BMPs.

SURFACE WATER QUALITY AND GROUND WATER QUALITY

The processing facility is not in the direct watershed of a waterbody most at risk from new development. The facility has a Multi-Sector General Permit, and collects surface water samples from 4 outfalls under the industrial stormwater program. These sampling requirements have been included in the facility operations manual. Staff comments that wastes have been stored outside and uncovered on soil base pads at this site for years, and waste materials have been used as fill, and staff have recommended a pore water monitoring program be implemented to characterize water quality. MB Bark has agreed to do this, and has included monitoring provisions in its operations manual. Staff comments that the procedures in 06-096 C.M.R. ch. 405, § 2(C) and (D) must be followed when evaluating water quality monitoring results. The Department finds that MB Bark has made adequate provisions for implementing a water quality monitoring program in order to determine whether or not surface water quality or ground water quality has been or will be affected, provided the procedures in 06-096 C.M.R. ch. 405, § 2(C) and (D) are followed when evaluating water quality monitoring results.

MB BARK	16	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

16. UTILITIES

No new access to utilities will be needed for the project. Waste materials that may be produced include metal removed from pallets, rail ties, or shingles; plastic or wood bypass removed during shingle processing; plastic or wood removed from compostable material; and unusable residue from cleanup of material storage and handling areas. Metal and bypass are proposed to be sent to recycling facilities or to Riverside Recycling in Portland. Unusable residue from material storage and handling area cleanup will be taken directly to a landfill. MB Bark has provided a statement from Waste Management-Crossroads Landfill that it is willing and has the capacity to accept the expected annual quantity of waste processing residue. The Department finds that MB Bark has made adequate provisions for utilities, including solid waste disposal, provided it disposes of unusable residue from material storage and handling area cleanup at the Waste Management-Crossroads Landfill or other facility licensed to accept this waste.

OTHER PROCESSING FACILITY CRITERIA

The amendment application requests approval to process pallets and asphalt shingles, and to accept and store CRT glass. MB Bark states it will not crush inert material on site, and will send CRT glass and inert material to CPRC for processing. Only unpainted, untreated pallets with no visible contamination are accepted. Pallets are ground and combined with land clearing debris and used to make mulch and erosion control mix. These products are sold for landscaping or erosion control purposes. Fasteners are removed by magnets in the processing equipment, and are sent off site for recycling as metal.

Asphalt shingles are ground to ½" minus and screened to separate the ¼" minus fraction. Plastic and other trash are removed prior to processing and sent to the Riverside Recycling transfer station. Nails are removed by magnets in the processing equipment and sent off site for recycling as metal. The ¼" to ½" ground material will be sent to CPRC to be made into C&R. The ¼" minus fraction will either be sent to hot mix asphalt plants licensed to accept it for use as a raw material substitute or be sent to CPRC for use in C&R. Each load of incoming asphalt shingles will be tested for asbestos prior to being added to the stockpile awaiting processing. After processing, one sample per 250 tons will be tested for asbestos, and one sample per year of ground shingles will be analyzed for the total metals arsenic, barium, cadmium, chromium, lead, mercury, selenium, and silver, and for benzo[a]pyrene. In addition, MB Bark states that as required by customers, ground shingles will be analyzed by TCLP for metals, volatile organic

MB BARK	17	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

compounds and semi-volatile organic compounds. CPRC's shingle beneficial use license contains limits on the levels of total metals and benzo[a]pyrene in C&R. At least one hot mix plant requires analysis for the TCLP parameters. MB Bark has included the material sampling requirements in its operations manual.

CRT glass is produced by electronics demanufacturing facilities. MB Bark will only accept CRT glass from electronics demanufacturing facilities that have a Department-approved sampling and analytical work plan for the CRT glass, and MB Bark will only accept a batch after it has been tested and found to be acceptable. This information has also been included in the operations manual. CRT glass will not be processed, and all of it will be sent off site to CPRC, who uses it to make C&R.

Material storage and processing areas will be cleaned annually, the surfaces inspected and repaired as needed, and elevations checked annually to ensure the proper elevations are maintained. Residues are reused to the extent possible. Reuse of rail tie processing residues may not be possible; in that event, it will be sent off site for disposal. This is discussed in Finding of Fact #16, above.

Staff comments that MB Bark has outlets for all stored materials and processed materials. Regarding residuals, staff comments that pallet and land clearing debris residuals cleaned up on the paved surface may be usable as mulch. The same residuals on gravel surfaces may be usable as erosion control material or possibly mixed into topsoil. Residuals from shingle processing cleaned up on the gravel surface may be usable in making C&R. Residuals from CRT glass storage cleaned up on the gravel surface may be usable in making C&R. Residuals from rail tie processing cleaned up on the gravel surface may be too dirty to use as fuel, and may not be used as mulch, and therefore must be taken to a landfill for disposal. Staff comments that MB Bark has made provisions for distribution or disposal of all materials stored or processed at the site and for residuals generated at the site. Staff comments that no materials may be stored on site for more than two years, which includes time before and after processing.

The Department finds that MB Bark has addressed other processing facility criteria, including material analytical requirements, material distribution plans, and management of residuals.

18. OPERATIONS

MB Bark has submitted a revised facility operations manual addressing existing and proposed aspects of the operation. The Department finds that MB Bark has submitted an

MB BARK	18	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

operations manual that addresses all applicable operating requirements provided a complete manual including all revisions and appendices is submitted for review and approval.

SOLID WASTE HIERARCHY

New or expanded processing facilities that generate residues requiring disposal must demonstrate that the facility will recycle or process into fuel for combustion all waste accepted at the facility to the maximum extent practicable, but in no case at a rate less than 50%, and that the facility is consistent with the recycling provisions of the state waste management and recycling plan, in order to show that their purpose and practices are consistent with the State's solid waste management hierarchy.

MB Bark expects to store, test and process up to 37,500 tons of shingles. In recent years, MB Bark has reported it received a maximum of 45,706 tons of land clearing debris, 2,080 tons of CRT glass, 1,416 tons of lumber/pallets, 14,953 tons of creosote treated wood, and 556 tons of inert materials annually. Shingles are proposed to be ground and sent to either hot mix asphalt plants for use in asphalt pavement production or sent to CPRC for use in making C&R. Land clearing debris, bark, and pallets are ground and made into bark mulch. CRT glass will be shipped to CPRC for use in making C&R. Creosote treated wood is ground to make boiler fuel. Inert material (soil, sand, rock, brick, porcelain, and non-CRT glass) is resold as fill or topsoil, or will be sent to CPRC for processing. Residues and bypass generated during processing include metal, plastic, and wood. Metal is recycled; plastic and wood will be disposed. No residue has been reported as disposed in recent years. Residue from cleaning material storage areas will be reused if possible; rail tie cleanup debris (estimated at 10 to 15 tractor-trailer loads, per year) will likely need to be disposed. The bypass and cleanup debris together would be a small percentage of all wastes received at the facility. MB Bark's operations manual addresses annual reporting, which will include reporting of the quantity of material received, processed, and disposed, allowing verification of the recycling percentage. Staff comments that it is very likely that well over 50% of the wastes received at MB Bark will be recycled, reused, sent off site for recycling or reuse elsewhere, or processed into fuel for combustion, but that the annual reports will need to verify the percentage. The Department finds that the purposes and practices of MB Bark are consistent with the State's solid waste management hierarchy, provided MB Bark completely addresses annual reporting requirements sufficient for the Department to verify that the reuse and recycling percentage meets the requirements of 06-096 C.M.R. ch. 409, § 2(C)(1).

MB BARK	19	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

BASED on the above Findings of Fact, and subject to the Conditions listed below, the Department makes the following Conclusions:

- MB Bark has presented clear and convincing evidence that its proposal will be compliant
 with State law and the purpose and intent of 06-096 C.M.R. ch. 409, § 2(A)(3).
- Use of reclaim and C&R as construction fill for this project, as described in the 2. application, is a beneficial use, provided: the 6-inch compacted gravel layer covering all construction fill does not contain secondary materials; a draft of the deed notice and restriction is submitted to the Department for review and approval prior to the start of construction, the finalized document is filed at the Androscoggin County Registry of Deeds within 60 days of Department approval, and a copy of the filed information, including book and page number, is provided to the Department as soon as it is available; any C&R or reclaim remaining on site after completion of phases 1, 2 and 3 is removed within 30 days of completion of construction and returned to CPRC or disposed in a landfill, unless otherwise licensed by the Department; no additional C&R is stored on site until MB Bark has a beneficial use license to distribute it; and application drawing sheet C-3.2 is revised to show material storage, handling, and processing areas with sizes that match the area sizes MB Bark states it needs in the October 5, 2018 letter, and is submitted to the Department for review and approval within 15 days of the effective date of this license.
- MB Bark has provided adequate evidence of title, right, or interest in the property.
- 4. MB Bark has provided adequate evidence of availability of sufficient funds to complete the proposed amendment and operate the facility in a manner consistent with state environmental standards.
- 5. MB Bark has provided adequate evidence of technical ability to design, construct, operate, and maintain the facility in accordance with state environmental requirements and its permits provided:
 - a professional engineer conducts weekly inspections during all times that construction is occurring;
 - weekly reports are submitted electronically to the Department each Tuesday for the preceding week:
 - when construction occurred, the report shall include personnel working on the project, a description of work completed, the type and quantity of material moved or placed, and engineering inspection reports; and

MB BARK	20	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

- when no construction occurred, the report may be an email documenting there was no construction activity from anyone with knowledge of the construction activities;
- clearing limits and earthwork limits are marked in the field by a surveyor or
 professional engineer prior to undertaking any clearing or earthwork and the
 markings are maintained until the Department concurs the construction was
 completed as licensed;
- areas requiring construction, as specified in the application materials, may not be used until a professional engineer has certified construction is complete and was done as approved by the Department and the Department has accepted this certification; and
- rail spur design information shall be provided to the Department for review and approval prior to construction of the rail spur.
- There is no reason to withhold this license based on civil or criminal record.
- MB Bark has made adequate provisions for safe and effective traffic movement into, out
 of, and within the proposed facility.
- 8. The proposed new and expanded areas will fit harmoniously into the natural environment provided that: no additional expansion at the west or southwest edge of the existing site; no disturbance (including mowing) occurs within 75 feet of Swamp White Oaks; and a 100-foot riparian buffer is maintained between intermittent and perennial streams and project disturbances.
- MB Bark's proposed expansion and new development will not present a bird hazard to aircraft, have an unreasonable adverse effect on the preservation of historical sites, or unreasonably interfere with views from established public viewing areas.
- 10. MB Bark has not demonstrated its noise levels will meet the standards in 06-096 C.M.R. ch. 400, § 4(F)(2)(a)(ii). An evaluation of all non-exempt noise sources at the facility (existing and proposed) must be performed by a qualified consultant, and an estimate of the maximum daytime and nighttime noise levels at all nearby protected locations must be submitted to the Department for review and approval prior to beginning any work associated with this application, and follow-up noise monitoring must be performed by a qualified consultant after construction, during operation of all non-exempt equipment, to verify that actual noise levels meet the standards in 06-096 C.M.R. ch. 400, § 4(F)(2)(a)(ii). In the event noise levels exceed the standards in 06-096 C.M.R. ch. 400, §

MB BARK	21	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN	·)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

4(F)(2)(a)(ii), MB Bark must cease use of the equipment causing the exceedance until it demonstrates it can meet the standards.

- 11. The processing facility will not unreasonably adversely affect air quality.
- 12. MB Bark has not yet demonstrated the expansion or the new development will be constructed on suitable soils. No material movement or placement for phase 1 construction may occur at the site until all Department concerns, as detailed in the staff memorandum dated August 21, 2018, regarding soil suitability and site stability have been resolved. Subsurface investigations must be performed in the area of the proposed road and pads 2 and 3, and a geotechnical report must be prepared describing what material conditioning, handling, placement, and compaction procedures will be needed, and to determine road subgrade preparation needed and type and thickness of materials needed for road construction. The report must be submitted for review and approval, and no earthwork or material movement or placement may be done for the phase 2 or 3 construction (road or pads 2 or 3) until the Department has approved commencement of construction of phases 2 and 3.
- 13. The processing facility will not unreasonably cause or increase flooding on site or on adjacent properties nor create an unreasonable flood hazard to a structure, and MB Bark has made adequate provisions for erosion and sedimentation control, provided it implements, inspects, and maintains temporary and permanent erosion control measures as described, and provided MB Bark, its agents and its consultants comply with the provisions in the most recent versions of the BMPs.
- 14. The Department finds that MB Bark has made adequate provisions for implementing a water quality monitoring program in order to determine whether or not surface water quality or ground water quality has been or will be affected, provided the procedures in 06-096 C.M.R. ch. 405, § 2(C) and (D) are followed when evaluating water quality monitoring results.
- 15. MB Bark has made adequate provisions for utilities, including solid waste disposal, provided it disposes of unusable residue from material storage and handling area cleanup at the Waste Management-Crossroads Landfill, or other facility licensed to accept this waste.
- 16. MB Bark has addressed other processing facility criteria, including material analytical requirements, material distribution plans, and management of residuals.

MB BARK	22	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

- 17. MB Bark has submitted an operations manual that addresses all applicable operational requirements provided a complete manual including all revisions and appendices is submitted to the Department for review and approval.
- 18. The purposes and practices of MB Bark are consistent with the State's solid waste management hierarchy, provided MB Bark completely addresses annual reporting requirements sufficient for the Department to verify that the reuse and recycling percentage meets the requirements of 06-096 C.M.R. ch. 409, § 2(C)(1).
- All other findings, conclusions, and conditions remain as approved in Department license #S-021741-WK-A-N and subsequent licenses.

THEREFORE the Department APPROVES MB Bark's request for a variance to 06-096 C.M.R. ch. 409, § 2(A)(3) of the Department's regulations and APPROVES the above noted application of MB Bark SUBJECT TO THE ATTACHED CONDITIONS, and all applicable standards and regulations.

- The Standard Conditions of Approval, a copy attached as Appendix A.
- MB Bark shall take all necessary actions to ensure that its activities or those of its agents
 do not result in unnecessary or noticeable erosion of soils on site during operation of the
 facility.
- The 6-inch compacted gravel surface layer placed over all construction fill shall not contain secondary materials.
- 4. A draft of the deed notice and restriction shall be submitted to the Department for review and approval prior to the start of construction, shall be filed in the Androscoggin County Registry of Deeds within 60 days of Department approval, and a copy of the filed information, including book and page number, shall be provided to the Department as soon as it is available.
- 5. Any C&R or reclaim remaining on site after completion of phases 1, 2 and 3 shall be removed within 30 days of completion of construction and returned to CPRC or disposed in a landfill, unless otherwise licensed by the Department. No additional C&R shall be stored on site until MB Bark has a beneficial use license to distribute it.

MB BARK	23	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS)	Ó	LICENSE, ONE-TIME

- 6. Within 15 days of the effective date of this license, MB Bark shall submit to the Department for review and approval application drawing sheet C-3.2, revised to show material storage, handling, and processing areas with sizes that match the area sizes MB Bark states it needs in the October 5, 2018 letter.
- A professional engineer shall conduct weekly inspections during all times that construction is occurring.
- 8. Weekly reports shall be submitted electronically to the Department each Tuesday for the preceding week. When construction occurred, the report shall include personnel working on the project, a description of work completed, the type and quantity of material moved or placed, and engineering inspection reports. When no construction occurred, the report may be an email from anyone with knowledge of the construction activities, documenting there was no construction activity.
- Rail spur design information shall be provided to the Department for review and approval prior to construction of the rail spur.
- 10. Clearing limits and earthwork limits shall be marked in the field by a surveyor or professional engineer prior to undertaking any clearing or earthwork, and the markings shall be maintained until the Department concurs the construction was completed as licensed.
- 11. Areas where construction occurred shall not be used until a professional engineer has certified construction is complete and was done as approved by the Department, and the Department has accepted this certification.
- 12. No additional expansion shall occur on the west and southwest edge of the existing site.
- 13. No disturbance, including mowing, shall occur within 75 feet of Swamp White Oaks.
- 14. A 100-foot riparian buffer shall be maintained between intermittent and perennial streams and project disturbances.
- 15. An evaluation of all non-exempt noise sources at the facility (existing and proposed) shall be performed by a qualified consultant, and an estimate of the maximum daytime and nighttime noise levels at all nearby protected locations shall be submitted to the Department for review and approval prior to beginning any work associated with this application.

MB BARK	24	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

- 16. Noise monitoring shall be performed by a qualified consultant after construction of each phase, and during operation of all non-exempt equipment, to verify that actual noise levels meet the standards in 06-096 C.M.R. ch. 400, § 4(F)(2)(a)(ii). If noise levels exceed the standards, MB Bark shall cease using the noise-producing equipment until it demonstrates it can meet the standard.
- 17. No material movement or placement for phase 1 construction shall occur at the site until all Department concerns, as detailed in the staff memorandum dated August 21, 2018, regarding soil suitability and site stability have been resolved.
- 18. Subsurface investigations shall be performed in the area of the proposed road and pads 2 and 3, and a geotechnical report shall be prepared describing what material conditioning, handling, placement, and compaction procedures will be needed, and to determine road subgrade preparation needed and type and thickness of materials needed for road construction. The report shall be submitted for review and approval.
- 19. No earthwork or material movement or placement may be done for the phase 2 or 3 construction (road or pads 2 or 3) until the Department has approved commencement of construction of phases 2 and 3.
- MB Bark shall implement, inspect, and maintain temporary and permanent erosion control measures as described in the application materials.
- MB Bark and its consultants shall comply with the provisions in the most recent versions of the BMPs.
- 22. Within 15 days of the effective date of this license, MB Bark shall submit a complete operations manual including all revisions and appendices to the Department for review and approval.
- 23. MB Bark and its consultant shall follow the procedures in 06-096 C.M.R. ch. 405, § 2(C) and (D) when evaluating water quality monitoring results.
- 24. MB Bark shall dispose of unusable residue from material storage and handling area cleanup at the Waste Management-Crossroads Landfill, or other facility licensed to accept this waste.

MB BARK	25	SOLID WASTE LICENSE
POLAND AND AUBURN, ANDROSCOGGIN)	
COUNTY, MAINE)	PROCESSING FACILITY
SOLID PROCESSING FACILITY AND)	LICENSE AMENDMENT
BENEFICIAL USE OF SOLID WASTE)	
# S-021741-WK-I-A and # S-021741-W4-J-N)	NEW BENEFICIAL USE
(APPROVAL WITH CONDITIONS))	LICENSE, ONE-TIME

- 25. MB Bark shall completely address annual reporting requirements sufficient for the Department to verify that the reuse and recycling percentage meets the requirements of 06-096 C.M.R. ch. 409, § 2(C)(1).
- 26. The one-time beneficial use approval shall lapse 2 years from the effective date of this license. No placement of secondary materials as fill may occur after that date without additional approval.
- 27. The invalidity or unenforceability of any provision, or part thereof, of this license shall not affect the remainder of the provision or any other provisions. This license shall be construed and enforced in all respects as if such invalid or unenforceable provision or part thereof had been omitted.

DONE AND DATED AT AUGUSTA, MAINE, THIS ______DAY

OF NOVEMBER , 2018.

DEPARTMENT OF ENVIRONMENTAL PROTECTION

BY: XWef (New S 57)
MELANIE LOYZIM, ACTING COMMUSIONER

PLEASE NOTE ATTACHED SHEET FOR GUIDANCE ON APPEAL PROCEDURES.

Date of initial receipt of amendment application, # S-021741-WK-I-A: March 9, 2018

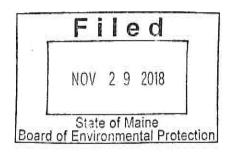
Date of acceptance of processing facility application: March 30, 2018

Date of initial receipt of beneficial use application, #S-021741-W4-J-N: September 10, 2018

Date of acceptance of beneficial use application: September 12, 2018

Date filed with Board of Environmental Protection:

xkk82900 & xkk83544



STANDARD CONDITIONS TO ALL SOLID WASTE FACILITY LICENSES

STRICT CONFORMANCE WITH THE STANDARD AND SPECIAL CONDITIONS OF THIS APPROVAL IS NECESSARY FOR THE PROJECT TO MEET THE STATUTORY CRITERIA FOR APPROVAL. VIOLATIONS OF THE CONDITIONS UNDER WHICH A LICENSE IS ISSUED SHALL CONSTITUTE A VIOLATION OF THAT LICENSE AGAINST WHICH ENFORCEMENT ACTION MAY BE TAKEN, INCLUDING REVOCATION.

- Approval of Variations from Plans. The granting of this approval is dependent upon and limited to the proposals and plans contained in the application and supporting documents submitted and affirmed by the license. Any consequential variation from these plans, proposals, and supporting documents is subject to review and approval prior to implementation.
- Compliance with All Applicable Laws. The licensee shall secure and comply with all
 applicable federal, state, and local licenses, permits, authorizations, conditions, agreements, and
 orders prior to or during construction and operation, as appropriate.
- 3. Compliance with All Terms and Conditions of Approval. The licensee shall submit all reports and information requested by the Department demonstrating that the licensee has complied or will comply with all terms and conditions of this approval. All preconstruction terms and conditions must be met before construction begins.
- 4. Transfer of License. The licensee may not transfer the solid waste facility license or any portion thereof without approval of the Department.
- 5. Initiation of Construction or Development Within Two Years. If the construction or operation of the solid waste facility is not begun within two years of issuance of within 2 years after any administrative and judicial appeals have been resolved, the license lapses and the licensee must reapply to the Department for a new license unless otherwise approved by the Department.
- 6. Approval Included in Contract Bids. A copy of the approval must be included in or attached to all contract bid specifications for the solid waste facility.
- 7. Approval Shown to Contractors. Contractors must be shown the licensee before commencing work on the solid waste facility.
- 8. Background of key individuals. A licensee may not knowingly hire as an officer, director or key solid waste facility employee, or knowingly acquire an equity interest or debt interest in, any person convicted of a felony or found to have violated a State or federal environmental law or rule without first obtaining the approval of the Department.
- Fees. The licensee must comply with annual license and annual reporting fee requirements of the Department's rules.
- 10. Recycling and Source Reduction Determination for Solid Waste Disposal Facilities. This condition does not apply to the expansion of a commercial solid waste disposal facility that accepts only special waste for landfilling.

Appendix A

STANDARD CONDITIONS TO ALL SOLID WASTE FACILITY LICENSES

The solid waste disposal facility shall only accept solid waste that is subject to recycling and source reduction programs, voluntary or otherwise, at least as effective as those imposed by 38 MRSA Chapter 13.

- 11. Deed Requirements for Solid Waste Disposal Facilities. Whenever any lot of land on which an active, inactive, or closed solid waste disposal facility is located is being transferred by deed, the following must be expressly stated in the deed:
 - A. The type of facility located on the lot and the dates of its establishment and closure.
 - B. A description of the location and the composition, extent, and depth of the waste deposited.
 - C. The disposal location coordinates of asbestos wastes must be identified.



DEP INFORMATION SHEET

Appealing a Department Licensing Decision

Dated: November 2018

Contact: (207) 287-2452

SUMMARY

There are two methods available to an aggrieved person seeking to appeal a licensing decision made by the Department of Environmental Protection's (DEP) Commissioner: (1) an administrative process before the Board of Environmental Protection (Board); or (2) a judicial process before Maine's Superior Court. An aggrieved person seeking review of a licensing decision over which the Board had original jurisdiction may seek judicial review in Maine's Superior Court.

A judicial appeal of final action by the Commissioner or the Board regarding an application for an expedited wind energy development (35-A M.R.S. § 3451(4)) or a general permit for an offshore wind energy demonstration project (38 M.R.S. § 480-HH(1)) or a general permit for a tidal energy demonstration project (38 M.R.S. § 636-A) must be taken to the Supreme Judicial Court sitting as the Law Court.

This information sheet, in conjunction with a review of the statutory and regulatory provisions referred to herein, can help a person to understand his or her rights and obligations in filing an administrative or judicial appeal.

I. ADMINISTRATIVE APPEALS TO THE BOARD

LEGAL REFERENCES

The laws concerning the DEP's Organization and Powers, 38 M.R.S. §§ 341-D(4) & 346; the Maine Administrative Procedure Act, 5 M.R.S. § 11001; and the DEP's Rules Concerning the Processing of Applications and Other Administrative Matters ("Chapter 2"), 06-096 C.M.R. ch. 2.

DEADLINE TO SUBMIT AN APPEAL TO THE BOARD

The Board must receive a written appeal within 30 days of the date on which the Commissioner's decision was filed with the Board. Appeals filed more than 30 calendar days after the date on which the Commissioner's decision was filed with the Board will be dismissed unless notice of the Commissioner's license decision was required to be given to the person filing an appeal (appellant) and the notice was not given as required.

HOW TO SUBMIT AN APPEAL TO THE BOARD

Signed original appeal documents must be sent to: Chair, Board of Environmental Protection, 17 State House Station, Augusta, ME 04333-0017. An appeal may be submitted by fax or e-mail if it contains a scanned original signature. It is recommended that a faxed or e-mailed appeal be followed by the submittal of mailed original paper documents. The complete appeal, including any attachments, must be received at DEP's offices in Augusta on or before 5:00 PM on the due date; materials received after 5:00 pm are not considered received until the following day. The risk of material not being received in a timely manner is on the sender, regardless of the method used. The appellant must also send a copy of the appeal documents to the Commissioner of the DEP; the applicant (if the appellant is not the applicant in the license proceeding at issue); and if a hearing was held on the application, any intervenor in that hearing process. All of the information listed in the next section of this information sheet must be submitted at the time the appeal is filed.

INFORMATION APPEAL PAPERWORK MUST CONTAIN

Appeal materials must contain the following information at the time the appeal is submitted:

- Aggrieved Status. The appeal must explain how the appellant has standing to maintain an appeal. This
 requires an explanation of how the appellant may suffer a particularized injury as a result of the
 Commissioner's decision.
- The findings, conclusions, or conditions objected to or believed to be in error. The appeal must
 identify the specific findings of fact, conclusions regarding compliance with the law, license conditions,
 or other aspects of the written license decision or of the license review process that the appellant
 objects to or believes to be in error.
- 3. The basis of the objections or challenge. For the objections identified in Item #2, the appeal must state why the appellant believes that the license decision is incorrect and should be modified or reversed. If possible, the appeal should cite specific evidence in the record or specific licensing requirements that the appellant believes were not properly considered or fully addressed.
- The remedy sought. This can range from reversal of the Commissioner's decision on the license or permit to changes in specific permit conditions.
- 5. All the matters to be contested. The Board will limit its consideration to those matters specifically raised in the written notice of appeal.
- 6. Request for hearing. If the appellant wishes the Board to hold a public hearing on the appeal, a request for public hearing must be filed as part of the notice of appeal, and must include an offer of proof in accordance with Chapter 2. The Board will hear the arguments in favor of and in opposition to a hearing on the appeal and the presentations on the merits of an appeal at a regularly scheduled meeting. If the Board decides to hold a public hearing on an appeal, that hearing will then be scheduled for a later date.
- 7. New or additional evidence to be offered. If an appellant wants to provide evidence not previously provided to DEP staff during the DEP's review of the application, the request and the proposed evidence must be submitted with the appeal. The Board may allow new or additional evidence, referred to as supplemental evidence, to be considered in an appeal only under very limited circumstances. The proposed evidence must be relevant and material, and (a) the person seeking to add information to the record must show due diligence in bringing the evidence to the DEP's attention at the earliest possible time in the licensing process; or (b) the evidence itself must be newly discovered and therefore unable to have been presented earlier in the process. Specific requirements for supplemental evidence are found in Chapter 2 § 24.

OTHER CONSIDERATIONS IN APPEALING A DECISION TO THE BOARD

- Be familiar with all relevant material in the DEP record. A license application file is public
 information, subject to any applicable statutory exceptions, and is made easily accessible by the DEP.
 Upon request, the DEP will make application materials available during normal working hours, provide
 space to review the file, and provide an opportunity for photocopying materials. There is a charge for
 copies or copying services.
- Be familiar with the regulations and laws under which the application was processed, and the
 procedural rules governing your appeal. DEP staff will provide this information on request and
 answer general questions regarding the appeal process.
- 3. The filing of an appeal does not operate as a stay to any decision. If a license has been granted and it has been appealed, the license normally remains in effect pending the processing of the appeal. Unless a stay of the decision is requested and granted, a license holder may proceed with a project pending the outcome of an appeal, but the license holder runs the risk of the decision being reversed or modified as a result of the appeal.

WHAT TO EXPECT ONCE YOU FILE A TIMELY APPEAL WITH THE BOARD

The Board will formally acknowledge receipt of an appeal, and will provide the name of the DEP project manager assigned to the specific appeal. The notice of appeal, any materials accepted by the Board Chair as supplementary evidence, any materials submitted in response to the appeal, and relevant excerpts from the DEP's application review file will be sent to Board members with a recommended decision from DEP staff. The appellant, the license holder if different from the appellant, and any interested persons are notified in advance of the date set for Board consideration of an appeal or request for public hearing. The appellant and the license holder will have an opportunity to address the Board at the Board meeting. With or without holding a public hearing, the Board may affirm, amend, or reverse a Commissioner decision or remand the matter to the Commissioner for further proceedings. The Board will notify the appellant, the license holder, and interested persons of its decision.

II. JUDICIAL APPEALS

Maine law generally allows aggrieved persons to appeal final Commissioner or Board licensing decisions to Maine's Superior Court (see 38 M.R.S. § 346(1); 06-096 C.M.R. ch. 2; 5 M.R.S. § 11001; and M.R. Civ. P. 80C). A party's appeal must be filed with the Superior Court within 30 days of receipt of notice of the Board's or the Commissioner's decision. For any other person, an appeal must be filed within 40 days of the date the decision was rendered. An appeal to court of a license decision regarding an expedited wind energy development, a general permit for an offshore wind energy demonstration project, or a general permit for a tidal energy demonstration project may only be taken directly to the Maine Supreme Judicial Court. See 38 M.R.S. § 346(4).

Maine's Administrative Procedure Act, DEP statutes governing a particular matter, and the Maine Rules of Civil Procedure must be consulted for the substantive and procedural details applicable to judicial appeals.

ADDITIONAL INFORMATION

If you have questions or need additional information on the appeal process, for administrative appeals contact the Board's Executive Analyst at (207) 287-2452, or for judicial appeals contact the court clerk's office in which your appeal will be filed.

Note: The DEP provides this INFORMATION SHEET for general guidance only; it is not intended for use as a legal reference. Maine law governs an appellant's rights.



STATE OF MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION 17 STATE HOUSE STATION AUGUSTA, MAINE 04333-0017

DEPARTMENT ORDER

IN THE MATTER OF

) NATURAL RESOURCES PROTECTION ACT
) FRESHWATER WETLAND ALTERATION
) RIVER, STREAM, BROOK ALTERATION
) WATER QUALITY CERTIFICATION
) FINDINGS OF FACT AND ORDER

Pursuant to the provisions of 38 M.R.S. §§ 480-A–480-JJ, Section 401 of the Federal Water Pollution Control Act (33 U.S.C. § 1341), Chapters 310, 315, and 335 of Department rules, the Department of Environmental Protection has considered the application of MB BARK, LLC, with the supportive data, agency review comments, and other related materials on file and FINDS THE FOLLOWING FACTS:

1. <u>PROJECT DESCRIPTION</u>:

A. History: In Department Order #L-19467-31-B-N, dated December 3, 1997, the Department approved 65,423 square feet of freshwater wetland alteration for the development of a bark mulch processing facility. The project included two stream crossings for an access road into the facility. Compensation for the loss of wetland areas and functions was conducted on site in the form of enhancement and creation along an unnamed stream, and in the form of preservation of an upland buffer adjacent to an emergent wetland. In 2006, the Department determined that the wetland enhancement and creation sites were successful. The project site is located on Bark Mulch Drive off Hardscrabble Road in the Town of Poland.

In Department Order #S-21741-WK-A-N, dated January 30, 1998, the Bureau of Remediation and Waste Management (BRWM) approved the construction of a solid waste facility at this location. Since that time, BRWM has approved several modifications to the permit.

B. Summary: The applicant proposes to permanently alter 26,853 square feet of forested freshwater wetlands to expand an existing bark mulch processing facility to accommodate additional types of solid waste materials. The expansion will include the construction of three new processing and handling pads, two 24-foot wide access roads and associated stormwater management structures. One of the proposed pads will be located on existing developed area; the other two proposed pads will be three acres and five acres in size, and located in a forested, undeveloped area. The proposed project will result in 26,853 square feet of permanent wetland alteration to construct the access roads and handling pads, and 3,760 square feet of temporary wetland alteration to clear trees to access and construct the proposed roads. The proposed wetland impacts include three road crossings, where the applicant proposes to install a total of six three-foot wide by

four-foot high box culverts at 40-foot intervals. The project will also include a stream crossing under one of the proposed access roads. The proposed stream crossing will consist of a 60-inch diameter, 40-foot long corrugated metal pipe culvert with riprap aprons, resulting in 60 linear feet of stream alteration. The culvert will be greater than 1.2 times bank-full width, embedded in the stream channel and backfilled with two feet of streambed material. The proposed project can be seen on a set of plans entitled, "Site Development Plans for MB Bark Recycling Facility Expansion," Sheets C-1.0 to C-7.4, prepared by Stantec Consulting Services, Inc. and dated February 8, 2018, with a latest revision date of June 6, 2018.

The applicant also submitted an application to BRWM to amend its existing Solid Waste license (#S-21741-WK-I-A) to construct the proposed expansion. BRWM accepted the application for review on March 30, 2018. BRWM will review the solid waste and stormwater management aspects of the proposed project.

C. Current Use of the Site: The site of the proposed project is located on two parcels that make up approximately 172-acres of land. The parcels contain an existing bark mulch processing facility consisting of an access drive, graveled and paved work areas, three buildings, three sheds, and associated infrastructure. The undeveloped portions of the parcel contain forested uplands, forested wetlands, and two unnamed streams. A portion of the facility is located in the Town of Auburn; the majority of the facility is located in the Town of Poland. The parcels are identified as Lots 15-2 and 17 on Map 4 of the Town of Poland's tax maps.

2. <u>EXISTING SCENIC, AESTHETIC, RECREATIONAL OR NAVIGATIONAL USES:</u>

The Natural Resources Protection Act (NRPA), in 38 M.R.S. §480-D(1), requires the applicant to demonstrate that the proposed project will not unreasonably interfere with existing scenic, aesthetic, recreational and navigational uses.

In accordance with Chapter 315, Assessing and Mitigating Impacts to Scenic and Aesthetic Uses (06-096 C.M.R. ch. 315, effective June 29, 2003), the applicant submitted a copy of the Department's Visual Evaluation Field Survey Checklist as Appendix A to the application along with a description of the property and the proposed project. The applicant also submitted several photographs of the proposed project site and surroundings.

The proposed project is not located in, on, or over a waterbody used by the general public. It is located within multiple freshwater wetlands and a stream located on the applicant's property. The wetlands and stream are not scenic resources as defined in Chapter 315 §5(H). The nearest scenic resource that is visited by the general public, in part, for the use, observation, enjoyment and appreciation of its natural and cultural visual qualities, is the Little Androscoggin River. The proposed project at its closest point is located approximately 0.4 miles from the Little Androscoggin River. Because of vegetation between the project site and the scenic resource, the proposed project site is not visible from the Little Androscoggin River.

The Department determined that based on the nature of the proposed project and its location, there are no existing recreational or navigational uses of the resource that would be unreasonably impacted.

The Department finds that the proposed activity will not unreasonably interfere with existing scenic, aesthetic, recreational or navigational uses of the Little Androscoggin River.

3. SOIL EROSION:

The NRPA, in 38 M.R.S. §480-D(2), requires the applicant to demonstrate that the proposed project will not cause unreasonable erosion of soil or sediment nor unreasonably inhibit the natural transfer of soil from the terrestrial to the marine or freshwater environment.

The applicant submitted a construction plan and an erosion and sedimentation control plan (Attachments 7 and 8), as well as erosion control details (Sheet C-7.0 of the plans referenced in Finding 1B), which describe specific measures that will be utilized pre-, during, and post-construction, in accordance with the MDEP *Erosion and Sediment Control Best Management Practices Field Guide*. Erosion control barriers will be installed downslope of all areas prior to ground disturbance, grubbing, and site grading. Disturbed areas, side slopes, and exposed soils will be stabilized with loam, mulch, and seed as necessary during and following construction. Temporary erosion and sedimentation controls will be maintained until disturbed soils are stable. The applicant stated they will retain a civil engineer to provide construction oversight and erosion control monitoring throughout the construction process.

The Department finds that the activity will not cause unreasonable erosion of soil or sediment nor unreasonably inhibit the natural transfer of soil from the terrestrial to the marine or freshwater environment.

4. HABITAT CONSIDERATIONS:

The NRPA, in 38 M.R.S. §480-D(3), requires the applicant to demonstrate that the proposed project will not unreasonably harm significant wildlife habitat, freshwater wetland plant habitat, threatened or endangered plant habitat, aquatic or adjacent upland habitat, travel corridor, freshwater, estuarine or marine fisheries or other aquatic life.

The project site contains forested uplands, forested wetlands, and an intermittent, unnamed stream that flows north into Davis Brook and the Little Androscoggin River. An additional unnamed stream is located on the northeast side of the development, outside of the proposed project area. The property contains several vernal pool depressions. The applicant submitted a report entitled, "Vernal Pool Survey Results," prepared by Stantec Consulting Services, Inc. and dated May 29, 2018. The report, based

on surveys conducted in April and May of 2018, found that none of the pools met the definition of significance under the NRPA.

The Maine Department of Inland Fisheries and Wildlife (MDIFW) reviewed the proposed project and stated that there are no Essential Habitats mapped within the project area. MDIFW recommend that a 100-foot wide riparian buffer be maintained along the stream and associated wetlands. Except for the proposed stream crossing, the project has been designed to accommodate the recommended buffer. MDIFW further recommended that all in-stream work take place between July 15 and October 1, and commented that the riprap aprons at the proposed stream crossing should be installed at four to six inches below the streambed elevation, and filled in to streambed elevation with gravel similar to that of the native streambed. The applicant agreed to the in-stream work window of July 15 to October 1, and revised the culvert details on Plan Sheet C-7.1 to incorporate MDIFW's recommendations.

The Department finds that the activity will not unreasonably harm any significant wildlife habitat, freshwater wetland plant habitat, threatened or endangered plant habitat, aquatic or adjacent upland habitat, travel corridor, freshwater, estuarine or marine fisheries or other aquatic life.

5. WATER QUALITY CONSIDERATIONS:

As discussed in Finding 3, the applicant proposes to use erosion and sedimentation control during construction to minimize impacts to water quality from siltation.

The Department does not anticipate that the proposed project will violate any state water quality law, including those governing the classification of the State's waters.

6. WETLANDS AND WATERBODIES PROTECTION RULES:

The applicant proposes to fill 26,853 square feet of forested freshwater wetlands to construct the proposed access roads and handling pads. The applicant proposes to temporarily alter 3,760 square feet of forested freshwater wetlands to clear trees to access and construct the proposed roads. The applicant proposes to alter 60 linear feet of a stream to install a culvert and riprap aprons at the proposed road crossing.

The Wetlands and Waterbodies Protection Rules, 06-096 C.M.R. ch. 310 (last amended January 26, 2009), interpret and elaborate on the NRPA criteria for obtaining a permit. The rules guide the Department in its determination of whether a project's impacts would be unreasonable. A proposed project would generally be found to be unreasonable if it would cause a loss in wetland area, functions and values and there is a practicable alternative to the project that would be less damaging to the environment. Each application for a NRPA permit that involves a freshwater wetland alteration or an alteration to a river, stream, or brook must provide an analysis of alternatives in order to demonstrate that a practicable alternative does not exist.

- Avoidance. An applicant must submit an analysis of whether there is a practicable alternative to the project that would be less damaging to the environment and this analysis is considered by the Department in its assessment of the reasonableness of any impacts. The applicant submitted an alternatives analysis for the proposed project completed by Stantec Consulting Services, Inc. and dated March 5, 2018. The purpose of the proposed project is to expand the existing facility in Poland/Auburn to accommodate a portion of the operations currently located at a facility in the Town of Scarborough. The expansion at the Poland/Auburn site will allow the facility to process asphalt shingles, gypsum board, and panel glass as well as bark mulch. The Scarborough facility will continue to process these materials as well as concrete, brick, dredge spoils, and petroleum-contaminated soils, but on a gradually reduced scale. The applicant considered keeping both facilities the same size, but determined that the Scarborough facility would be unable to support an anticipated increase in demands. The applicant considered expanding only the Scarborough facility, but determined that the facility is constrained by the parcel size and by adjacent development. The applicant considered consolidating the Scarborough operations within the footprint of the existing Poland/Auburn facility, but determined that the existing footprint is too small to accommodate the variety of products processed by the two operations. The applicant considered moving the Scarborough operation to an entirely new location, but was unable to locate a parcel that was financially and logistically feasible. The applicant considered other expansion designs of the Poland/Auburn facility and determined that all other designs would result in a greater amount of wetland impact. In light of these considerations, the applicant stated that there is no practicable alternative to the proposed project that can meet the applicant's needs and avoids impact to the wetlands and stream.
- В. Minimal Alteration. In support of an application and to address the analysis of the reasonableness of any impacts of a proposed project, an applicant must demonstrate that the amount of freshwater wetland and stream to be altered will be kept to the minimum amount necessary for meeting the overall purpose of the project. The applicant minimized impacts to wetlands by maximizing the use of upland areas on the parcel, and by designing the proposed roads and handling pads to avoid the majority of wetlands and to be greater than 250 feet away from the vernal pools on the parcel. The wetland crossings are located at narrow points in the wetlands, and designed with narrowed side slopes to minimize intrusion into the wetlands. The wetland crossings are designed with box culverts and with large rock base materials to maintain hydrologic connectivity in the wetlands. The stream crossing is designed to an appropriate width as recommended by MDIFW, and will be embedded and backfilled with streambed material to maintain habitat connectivity for aquatic organisms. The applicant stated that the proposed project minimizes impacts to the freshwater wetlands and stream to the greatest extent practicable.

C. Compensation. In accordance with Chapter 310 §5(C)(6)(a), compensation is required to achieve the goal of no net loss of freshwater wetland functions and values. The applicant submitted a plan that identifies the wetland areas within the project area entitled, "Proposed Wetland Impact Map," prepared by Stantec Consulting Services, Inc. and last revised May 17, 2018. The applicant also submitted a functions and values assessment entitled, "Functions and Values Assessment Report – MB Bark Recycling Facility," prepared by Stantec Consulting Services, Inc. and dated March 2, 2018. The assessment identified seven wetlands within the vicinity of the proposed development, all of which are predominantly palustrine forested wetlands (PFO). Only three wetlands will be directly impacted by the proposed project. The assessment identified sediment/toxicant retention, nutrient removal, shoreline stabilization, and wildlife habitat as the principal wetland functions that were impacted as a result of the project.

The applicant proposes to compensate for lost functions and values of the freshwater wetlands through a contribution to the In-Lieu-Fee (ILF) Program of the Maine Natural Resource Conservation Program (MNRCP) in the amount of \$101,505.00, payable to "Treasurer, State of Maine," and directly to the attention of the ILF Program Administrator at 17 State House Station, Augusta, Maine 04333. The ILF payment must be received by the Department prior to the start of construction.

The proposed compensatory plan meets the requirements for the restoration, enhancement and preservation of freshwater wetland impacts outlined in Chapter 310. Therefore, the Department finds that the applicant has avoided and minimized waterbody and wetland impacts to the greatest extent practicable, and that the proposed project represents the least environmentally damaging alternative that meets the overall purpose of the project, provided that prior to project construction, the applicant submits the ILF payment as described above.

7. OTHER CONSIDERATIONS:

The Department finds, based on the design, proposed construction methods, and location, the proposed project will not inhibit the natural transfer of soil from the terrestrial to the marine environment, will not interfere with the natural flow of any surface or subsurface waters, and will not cause or increase flooding. The proposed project is not located in a coastal sand dune system, is not a crossing of an outstanding river segment, and does not involve dredge spoils disposal or the transport of dredge spoils by water.

BASED on the above findings of fact, and subject to the conditions listed below, the Department makes the following conclusions pursuant to 38 M.R.S. §§ 480-A–480-JJ and Section 401 of the Federal Water Pollution Control Act:

A. The proposed activity will not unreasonably interfere with existing scenic, aesthetic, recreational, or navigational uses.

- B. The proposed activity will not cause unreasonable erosion of soil or sediment.
- C. The proposed activity will not unreasonably inhibit the natural transfer of soil from the terrestrial to the marine or freshwater environment.
- D. The proposed activity will not unreasonably harm any significant wildlife habitat, freshwater wetland plant habitat, threatened or endangered plant habitat, aquatic or adjacent upland habitat, travel corridor, freshwater, estuarine, or marine fisheries or other aquatic life provided that the applicant submits a payment to the ILF program as described in Finding 6.
- E. The proposed activity will not unreasonably interfere with the natural flow of any surface or subsurface waters.
- F. The proposed activity will not violate any state water quality law including those governing the classifications of the State's waters.
- G. The proposed activity will not unreasonably cause or increase the flooding of the alteration area or adjacent properties.
- H. The proposed activity is not on or adjacent to a sand dune.
- I. The proposed activity is not on an outstanding river segment as noted in 38 M.R.S. § 480-P.

THEREFORE, the Department APPROVES the application of MB BARK, LLC to expand an existing solid waste facility as described in Finding 1, SUBJECT TO THE ATTACHED CONDITIONS, and all applicable standards and regulations:

- 1. Standard Conditions of Approval, a copy attached.
- 2. The applicant shall take all necessary measures to ensure that its activities or those of its agents do not result in measurable erosion of soil on the site during the construction of the project covered by this approval.
- 3. Severability. The invalidity or unenforceability of any provision, or part thereof, of this License shall not affect the remainder of the provision or any other provisions. This License shall be construed and enforced in all respects as if such invalid or unenforceable provision or part thereof had been omitted.

4. Prior to the start of construction, the applicant shall submit a payment in the amount of \$101,505.00, payable to "Treasurer, State of Maine," to the attention of the ILF Program Administrator at 17 State House Station, Augusta, Maine 04333.

THIS APPROVAL DOES NOT CONSTITUTE OR SUBSTITUTE FOR ANY OTHER REQUIRED STATE, FEDERAL OR LOCAL APPROVALS NOR DOES IT VERIFY COMPLIANCE WITH ANY APPLICABLE SHORELAND ZONING ORDINANCES.

DONE AND DATED IN AUGUSTA, MAINE, THIS 19TH DAY OF JUNE , 2018

DEPARTMENT OF ENVIRONMENTAL PROTECTION

BY: Mach Bycon

JUN 1 9 2018

State of Maine
Board of Environmental Protection

PLEASE NOTE THE ATTACHED SHEET FOR GUIDANCE ON APPEAL PROCEDURES.

JM/L19467CNDN/ATS#82887,82948



Natural Resources Protection Act (NRPA) Standard Conditions

THE FOLLOWING STANDARD CONDITIONS SHALL APPLY TO ALL PERMITS GRANTED UNDER THE NATURAL RESOURCES PROTECTION ACT, 38 M.R.S. § 480-A ET SEQ., UNLESS OTHERWISE SPECIFICALLY STATED IN THE PERMIT.

- A. <u>Approval of Variations From Plans.</u> The granting of this permit is dependent upon and limited to the proposals and plans contained in the application and supporting documents submitted and affirmed to by the applicant. Any variation from these plans, proposals, and supporting documents is subject to review and approval prior to implementation.
- B. <u>Compliance With All Applicable Laws.</u> The applicant shall secure and comply with all applicable federal, state, and local licenses, permits, authorizations, conditions, agreements, and orders prior to or during construction and operation, as appropriate.
- C. <u>Erosion Control.</u> The applicant shall take all necessary measures to ensure that his activities or those of his agents do not result in measurable erosion of soils on the site during the construction and operation of the project covered by this Approval.
- D. <u>Compliance With Conditions</u>. Should the project be found, at any time, not to be in compliance with any of the Conditions of this Approval, or should the applicant construct or operate this development in any way other the specified in the Application or Supporting Documents, as modified by the Conditions of this Approval, then the terms of this Approval shall be considered to have been violated.
- E. <u>Time frame for approvals.</u> If construction or operation of the activity is not begun within four years, this permit shall lapse and the applicant shall reapply to the Board for a new permit. The applicant may not begin construction or operation of the activity until a new permit is granted. Reapplications for permits may include information submitted in the initial application by reference. This approval, if construction is begun within the four-year time frame, is valid for seven years. If construction is not completed within the seven-year time frame, the applicant must reapply for, and receive, approval prior to continuing construction.
- F. <u>No Construction Equipment Below High Water.</u> No construction equipment used in the undertaking of an approved activity is allowed below the mean high water line unless otherwise specified by this permit.
- G. <u>Permit Included In Contract Bids.</u> A copy of this permit must be included in or attached to all contract bid specifications for the approved activity.
- H. <u>Permit Shown To Contractor.</u> Work done by a contractor pursuant to this permit shall not begin before the contractor has been shown by the applicant a copy of this permit.



DEP INFORMATION SHEET

Appealing a Department Licensing Decision

Dated: March 2012 Contact: (207) 287-2811

SUMMARY

There are two methods available to an aggrieved person seeking to appeal a licensing decision made by the Department of Environmental Protection's ("DEP") Commissioner: (1) in an administrative process before the Board of Environmental Protection ("Board"); or (2) in a judicial process before Maine's Superior Court. An aggrieved person seeking review of a licensing decision over which the Board had original jurisdiction may seek judicial review in Maine's Superior Court.

A judicial appeal of final action by the Commissioner or the Board regarding an application for an expedited wind energy development (35-A M.R.S.A. § 3451(4)) or a general permit for an offshore wind energy demonstration project (38 M.R.S.A. § 480-HH(1)) or a general permit for a tidal energy demonstration project (38 M.R.S.A. § 636-A) must be taken to the Supreme Judicial Court sitting as the Law Court.

This INFORMATION SHEET, in conjunction with a review of the statutory and regulatory provisions referred to herein, can help a person to understand his or her rights and obligations in filing an administrative or judicial appeal.

I. ADMINISTRATIVE APPEALS TO THE BOARD

LEGAL REFERENCES

The laws concerning the DEP's *Organization and Powers*, 38 M.R.S.A. §§ 341-D(4) & 346, the *Maine Administrative Procedure Act*, 5 M.R.S.A. § 11001, and the DEP's *Rules Concerning the Processing of Applications and Other Administrative Matters* ("Chapter 2"), 06-096 CMR 2 (April 1, 2003).

HOW LONG YOU HAVE TO SUBMIT AN APPEAL TO THE BOARD

The Board must receive a written appeal within 30 days of the date on which the Commissioner's decision was filed with the Board. Appeals filed after 30 calendar days of the date on which the Commissioner's decision was filed with the Board will be rejected.

HOW TO SUBMIT AN APPEAL TO THE BOARD

Signed original appeal documents must be sent to: Chair, Board of Environmental Protection, c/o Department of Environmental Protection, 17 State House Station, Augusta, ME 04333-0017; faxes are acceptable for purposes of meeting the deadline when followed by the Board's receipt of mailed original documents within five (5) working days. Receipt on a particular day must be by 5:00 PM at DEP's offices in Augusta; materials received after 5:00 PM are not considered received until the following day. The person appealing a licensing decision must also send the DEP's Commissioner a copy of the appeal documents and if the person appealing is not the applicant in the license proceeding at issue the applicant must also be sent a copy of the appeal documents. All of the information listed in the next section must be submitted at the time the appeal is filed. Only the extraordinary circumstances described at the end of that section will justify evidence not in the DEP's record at the time of decision being added to the record for consideration by the Board as part of an appeal.

WHAT YOUR APPEAL PAPERWORK MUST CONTAIN

Appeal materials must contain the following information at the time submitted:

- 1. *Aggrieved Status*. The appeal must explain how the person filing the appeal has standing to maintain an appeal. This requires an explanation of how the person filing the appeal may suffer a particularized injury as a result of the Commissioner's decision.
- 2. The findings, conclusions or conditions objected to or believed to be in error. Specific references and facts regarding the appellant's issues with the decision must be provided in the notice of appeal.
- 3. *The basis of the objections or challenge*. If possible, specific regulations, statutes or other facts should be referenced. This may include citing omissions of relevant requirements, and errors believed to have been made in interpretations, conclusions, and relevant requirements.
- 4. *The remedy sought.* This can range from reversal of the Commissioner's decision on the license or permit to changes in specific permit conditions.
- 5. All the matters to be contested. The Board will limit its consideration to those arguments specifically raised in the written notice of appeal.
- 6. Request for hearing. The Board will hear presentations on appeals at its regularly scheduled meetings, unless a public hearing on the appeal is requested and granted. A request for public hearing on an appeal must be filed as part of the notice of appeal.
- 7. New or additional evidence to be offered. The Board may allow new or additional evidence, referred to as supplemental evidence, to be considered by the Board in an appeal only when the evidence is relevant and material and that the person seeking to add information to the record can show due diligence in bringing the evidence to the DEP's attention at the earliest possible time in the licensing process or that the evidence itself is newly discovered and could not have been presented earlier in the process. Specific requirements for additional evidence are found in Chapter 2.

OTHER CONSIDERATIONS IN APPEALING A DECISION TO THE BOARD

- 1. Be familiar with all relevant material in the DEP record. A license application file is public information, subject to any applicable statutory exceptions, made easily accessible by DEP. Upon request, the DEP will make the material available during normal working hours, provide space to review the file, and provide opportunity for photocopying materials. There is a charge for copies or copying services.
- 2. Be familiar with the regulations and laws under which the application was processed, and the procedural rules governing your appeal. DEP staff will provide this information on request and answer questions regarding applicable requirements.
- 3. The filing of an appeal does not operate as a stay to any decision. If a license has been granted and it has been appealed the license normally remains in effect pending the processing of the appeal. A license holder may proceed with a project pending the outcome of an appeal but the license holder runs the risk of the decision being reversed or modified as a result of the appeal.

WHAT TO EXPECT ONCE YOU FILE A TIMELY APPEAL WITH THE BOARD

The Board will formally acknowledge receipt of an appeal, including the name of the DEP project manager assigned to the specific appeal. The notice of appeal, any materials accepted by the Board Chair as supplementary evidence, and any materials submitted in response to the appeal will be sent to Board members with a recommendation from DEP staff. Persons filing appeals and interested persons are notified in advance of the date set for Board consideration of an appeal or request for public hearing. With or without holding a public hearing, the Board may affirm, amend, or reverse a Commissioner decision or remand the matter to the Commissioner for further proceedings. The Board will notify the appellant, a license holder, and interested persons of its decision.

II. JUDICIAL APPEALS

Maine law generally allows aggrieved persons to appeal final Commissioner or Board licensing decisions to Maine's Superior Court, see 38 M.R.S.A. § 346(1); 06-096 CMR 2; 5 M.R.S.A. § 11001; & M.R. Civ. P 80C. A party's appeal must be filed with the Superior Court within 30 days of receipt of notice of the Board's or the Commissioner's decision. For any other person, an appeal must be filed within 40 days of the date the decision was rendered. Failure to file a timely appeal will result in the Board's or the Commissioner's decision becoming final.

An appeal to court of a license decision regarding an expedited wind energy development, a general permit for an offshore wind energy demonstration project, or a general permit for a tidal energy demonstration project may only be taken directly to the Maine Supreme Judicial Court. See 38 M.R.S.A. § 346(4).

Maine's Administrative Procedure Act, DEP statutes governing a particular matter, and the Maine Rules of Civil Procedure must be consulted for the substantive and procedural details applicable to judicial appeals.

ADDITIONAL INFORMATION

If you have questions or need additional information on the appeal process, for administrative appeals contact the Board's Executive Analyst at (207) 287-2452 or for judicial appeals contact the court clerk's office in which your appeal will be filed.

Note: The DEP provides this INFORMATION SHEET for general guidance only; it is not intended for use as a legal reference. Maine law governs an appellant's rights.



IN-LIEU-FEE (ILF) PROJECT COPY FOR DEP

Applicant:

Please attach your check to this entire packet and forward it to the DEP as stated in Special condition # 6 of your permit.

MAINE IN-LIEU-FEE (ILF) PROJECT IMPACT WORKSHEET

DEP Invoice #		Filled in by	v ILF Administrator in	Augusta
Project name:	Morse Brothers Bark, LLC			
Permittee(s):	Moody's Collision Center			
DEP/Corps permit	#: L -19467-TG-C-N /NA	E-2018-00135	Attach a copy of th	e permit
DEP/Corps Project	t Manager: Jami MacNe	il / Rodney Howe		
ILF Fee Amount:	\$101,505.00			
Check Date:		Filled in l	by ILF Administrator in	n Augusta
Project address:	Bark Mulch Drive Auburn, M	laine	Attach a l	ocus map
Biophysical region	- Section:	Central Interior & Mid Coast		
Biophysical region	- Subsection:	Central Maine Embayment Subs	section	_
Total impact area s	ubject to compensation:	26,853 SF (.61 acres)		
Resource(s) impacte	ed:			
Resource Types (list all that apply)	Functions & Values (for wetland impacts) (list all that apply, by resource type)	Types of Impacts (list all that apply, by resource type)	SF Impacted (by resource type)	Linear FT of Streams Impacted (for Corps use)
PFO	FF,NR,WH,STR	Filling	26,853	
				<u> </u>

Resource Types: Wetlands by NWI Type (PEM, PFO, PSS, PUB, M1, M2, E1, E2, etc), significant vernal pool depression (SVP), significant vernal pool critical terrestrial habitat (VPCTH), shorebird feeding & staging habitat (shorebird), inland waterfowl & wading bird habitat (IWWH), Tidal waterfowl & wading bird habitat (TWWH), lake or pond (L1, L2), river/stream/brook (RSB) Wetland Functions & Values: Groundwater recharge/discharge (GWR); floodflow alteration (FF);

Total impacts:

fish & shellfish habitat (FSH); sediment toxicant retention (STR); nutrient removal (NR); production export (PE); sediment/shoreline stabilization (SS); recreation (R); education/scientific value (ESV); uniqueness/heritage (UH); and visual quality/aesthetics (VQ); wildlife habitat (WH)

Types of Impacts: May include: filling, dredging, vegetation conversion (e.g. forested to shrub/scrub), excavation with associated discharge, etc.

26,853



WORK-START NOTIFICATION FORM

(Minimum Notice: Two weeks before work begins)

* MAIL TO:	U.S. Army Corps of Engineers, Nev	w England District	39
*	Policy Analysis/Technical Support	Branch	*
*	Regulatory Division		*
*	696 Virginia Road		*
*	Concord, Massachusetts 01742-275	51	*
*******	**********	**********	*****
permit authorized t the expansion of the Auburn/Poland, Mai	he permittee to place fill in 26,853 Sl Morse Brothers solid waste processing	issued to Morse Brothers Bark, LLC F (0.61 acres) of wetland in conjunction g facility off W. Hardscrabble Road ted below will do the work, and they	n with
PLEASE PRINT	OR TYPE		
Name of Person/F	irm:		
Business Address:			
Telephone Numbe			
		Finish	
Permittee's Signat	ure:	Date:	
Printed Name:		Title:	
******	**************************************	**************************************	*****
PM: Howe	Submitta	ls Required:	
Inspection Recomi	nendation:		·



Telephone Number

(Minimum Notice: Permittee must sign and return notification within one month of the completion of work.)

COMPLIANCE CERTIFICATION FORM

USACE Project Nu	mber: <u>NAE-2018-00135</u>		
Name of Permittee:	Morse Brothers Bark, LLC	<u> </u>	
Permit Issuance Da	te:		
mitigation required by		ollowing address upon completion of mit this after the mitigation is comple	
******	********	**********	****
* MAIL TO:	U.S. Army Corps of Engin	eers, New England District	*
*	Policy Analysis/Technical	-	*
*	Regulatory Division	•	*
*	696 Virginia Road		*
*	Concord, Massachusetts 0	1742-2751 **************	*
Diagon moto that wown	nomistad activity is subject	to a commission increasion by an IIS	Ammy Come of
	ive. If you fail to comply w	to a compliance inspection by an U.S with this permit you are subject to permit you are subject	
with the terms and c		ne above referenced permit was comerenced permit, and any required nations.	
Signature of Permitte	e	Date	
Printed Name		Date of Work Completion	
)		()	

Telephone Number



DEPARTMENT OF THE ARMY

NEW ENGLAND DISTRICT, CORPS OF ENGINEERS 896 VIRGINIA ROAD CONCORD, MASSACHUSETTS 01742-2751

MAINE GENERAL PERMIT (GP) AUTHORIZATION LETTER AND SCREENING SUMMARY

Jim Hiltner

CORPS PERMIT #_	NAE-2018-00135
CORPS GP ID#_	18-177
STATE ID#	L -19467-TG-C-N

Morse Brothers Bark, LLC 100 Bark Mulch Drive	CORPS GP ID#	18-177
Auburn, Maine 04210	STATE ID#	L -19467-TG-C-N
DESCRIPTION OF WORK:		
place fill in 26,853 SF (0.61 acres) of wetland in conjunction with the expansion Hardscrabble Road Auburn/Poland. Maine as shown on the attached plans entit	led MR Bark Recycling Facility	Expansion 100 Doels Madala
Drive Augurn/Poland, Maine by Stantec Consulting Services, Inc. in 27 sheets d	lated 2/6/2018.	andra attention of the contract of the contrac
ADDITIONAL CONDITIONS: SEE ATTACHED SHEET		, , , , , , , , , , , , , , , , , , ,
LAT/LONG COORDINATES : 44.05525° N 70.30294	° USGS QUAD	:ME- Minot
I. CORPS DETERMINATION:		
Based on our review of the information you provided, we have determined that your project wetlands of the United States. Your work is therefore authorized by the U.S. Army Corpermit (GP). Accordingly, we do not plan to take any further action on this project.	will have only minimal individual a ps of Engineers under the enclosed	and cumulative impacts on waters and defended Federal Permit, the Maine General
You must perform the activity authorized herein in compliance with all the terms and conditions along the first 101 March 2011 in Conditions along the first	ions of the GP [including any attach	ed Additional Conditions and any
beginning on page 5, to familiarize yourself with its contents. You are responsible for complete	ation]. Please review the enclosed G	P carefully, including the GP conditions
whoever does the work fully understands all of the conditions. You may wish to discuss the contractor can accomplish the work in a manner that conforms to all requirements.	conditions of this authorization with	h your contractor to ensure the
If you change the plans or construction methods for work within our jurisdiction, please cont	taat us immediataly to discuss dis	Section - Satisfact of the section
office must approve any changes before you undertake them.	act as immediately to discuss modif	ication of this authorization. This
Condition 38 of the GP (page 16) provides one year for completion of work that has commen October 13, 2020. You will need to apply for reauthorization for any work within Corps juris	nced or is under contract to commens sdiction that is not completed by Oc	ce prior to the expiration of the GP on tober 13, 2021.
This authorization presumes the work shown on your plans noted above is in waters of the U. an approved jurisdictional determination in writing to the undersigned.	.S. Should you desire to appeal our	jurisdiction, please submit a request for
No work may be started unless and until all other required local, State and Federal licenses an Hazard Development Permit issued by the town if necessary.	nd permits have been obtained. This	s includes but is not limited to a Flood
II. STATE ACTIONS: PENDING [X], ISSUED[], DENIED [] DATE	£	
APPLICATION TYPE: PBR; TIER 1; TIER 2; X TIER 3;	LURC: DMR LEASE: _	NA:
III. FEDERAL ACTIONS:		
JOINT PROCESSING MEETING: 3/15/18 LEVEL OF REVIEW: CATE	GGORY I: CATEG	ORY 2 <u>: X</u>
AUTHORITY (Based on a review of plans and/or State/Federal applications): SEC 10	, 404X10/404	_, 103
EXCLUSIONS: The exclusionary criteria identified in the general permit do not apply to the	nis project.	
FEDERAL RESOURCE AGENCY OBJECTIONS: EPA_NO_, USF&WS_NO_	, NMFS_NO_	
If you have any questions on this matter, please contact my staff at 207-623-8367 at our Augus appreciate your completing our Customer Service Survey located at http://per2.nwp.usacc.arm	sta, Maine Project Office. In order : <u>vy.mil/survey.html</u>	for us to better serve you, we would
12	(,50,10	

RODNEY A. HOWE SENIOR PROJECT MANAGER MAINE PROJECT OFFICE

FANK J. DEL GIUDICE CHIEF, PERMITS & ENFORCEMENT BRANCH REGULATORY DIVISION



PLEASE NOTE THE FOLLOWING ADDITIONAL CONDITIONS FOR DEPARTMENT OF THE ARMY GENERAL PERMIT NO. NAE-2018-00135

- 1. The permittee shall assure that a copy of this permit is at the work site whenever work is being performed and that all personnel performing work at the site of the work authorized by this permit are fully aware of the terms and conditions of the permit. This permit, including its drawings and any appendices and other attachments, shall be made a part of any and all contracts and sub-contracts for work which affects areas of Corps of Engineers' jurisdiction at the site of the work authorized by this permit. This shall be done by including the entire permit in the specifications for the work. If the permit is issued after construction specifications but before receipt of bids or quotes, the entire permit shall be included as an addendum to the specifications. The term "entire permit" includes permit amendments. Although the permittee may assign various aspects of the work to different contractors or sub-contractors, all contractors and sub-contractors shall be obligated by contract to comply with all environmental protection provisions of the entire permit, and no contract or sub-contract shall require or allow unauthorized work in areas of Corps of Engineers jurisdiction.
- 2. The permittee must still obtain any other Federal, State, or local permits as required by law before beginning work. This includes but is not limited to a Flood Hazard Development Permit issued by the town if necessary.
- 3. This authorization requires you to 1) notify us before beginning work so we may inspect the project, and 2) submit a Compliance Certification Form. You must complete and return the enclosed Work Start Notification Form(s) to this office at least two weeks before the anticipated starting date. You must complete and return the enclosed Compliance Certification Form within one month following the completion of the authorized work and any required mitigation (but not mitigation monitoring, which requires separate submittals).
- 4. Please note General Condition 21. Sedimentation and Erosion Control on pages 11 and 12 of the attached General Permit.
- 5. No tree clearing activities shall occur from June 1- July 31 of any given year to minimize the potential effect to the Northern Long eared bat.
- 6. Mitigation shall consist of payment of \$101,505.00 to the Natural Resource Mitigation Fund. The Corps will provide a completed ILF Project Data Worksheet which must be mailed with a cashier's check or bank draft, made out to "Treasurer, State of Maine", with the permit number noted on the check. The check and worksheet should be mailed to: MEDEP, Attn: ILF PROGRAM ADMINISTRATOR, State House Station 17, Augusta, ME 04333. This authorization (permit) is not valid until the permittee provides the Corps with a copy of the check, with the permit number noted on the check. The ILF amount is only valid for a period of one year from the date on the authorization letter. After that time, the project would need to be reevaluated and a new amount determined.
- 7. The permittee and their contractors will minimize the potential for impacts to aquatic habitat by conducting all instream work (which includes the installation and removal of cofferdams, as well as other activities) within the specified low flows work window of July 15th to September 30th.

Town of Poland

Planning Board DEPARTMENTAL REVIEW OF PROPOSED SITE APPLICATION

01

Tom Printup

Date:

,04

, 2019

To:	Tom Printup		Use Code for the Tov is required to ask that capacity of capital fac the Planning Board, b	napter 606, Site Review, of the Comprehensive Land yn of Poland, an applicant for development approval Municipal Departments to comment on their ilities to serve a proposed development. Therefore, by way of the applicant, is notifying you of the oject and requests your comments
Applicant: Address: Location:	100 Bark Mulch Drive. Auburn, Maine Map # 0004	Lot #	0015	Sublot #_0002
Road Local Project ove	tion: Hardscrabble Rd. Additional material	handling areas	s for processing	recyclable materials.
Scheduled	Planning Board Meeting Date	02 /12	/2019	
Mail thi to the s	cheduled meeting. (See reverse for n with the department heads that t	e application so the list of Department	hat each departme nt Heads)	nt head <u>receives</u> it at least fourteen days prior to the Planning Board Office in time for the
Signed:	reasons on department letterhead I need more information on the ap	Ilowing: Department. sting capital facilitie equate existing cap	pital facilities to serve	the project for the reasons listed. (Please submit
Please retu	urn by: Date: 02 / 07/ 2019		Planning Board Town of Poland 1231 Maine Stre Poland, Maine 0	et

Town of Poland

In accordance with Chapter 606, Site Review, of the Comprehensive Land Use Code for the Town of Poland, an applicant for development approval

Planning Board DEPARTMENTAL REVIEW OF PROPOSED SITE APPLICATION

, 2019

Date:

To:

_	-			is required to ask tha capacity of capital fac the Planning Board, I	t Municipal Departments to commen cilities to serve a proposed developm by way of the applicant, is notifying y roject and requests your comments	t on their nent. Therefore
Applicant:	MB Bark LLC					
Address:	100 Bark Mulch Drive.	ål be		- 1		
	Auburn, Maine				X 10.200	-
Location:	Map # 0004		Lot#	0015	Sublot #_0002	
Road Locatio	n: Hardscrabble Rd.		-			
Project overv		al handling a	areas	for processing	recyclable materials.	
3	- 1-5-1					
Scheduled Pla	anning Board Meeting Date	02 /	12	/2019		
to the sche	orm letter along with a copy of the duled meeting. (See reverse for with the department heads that	or list of Depa	rtmen	t Heads)		
Signed: Head of Depar	ed this application and provide the factor of the project has no impact on the project has no impact on the project has adequate expenses on department does not have an reasons on department letterhead in need more information on the appropriate of the project has been provided in the project has a project ha	ollowing: Department. cisting capital fadequate existir d)	acilities ng capit	al facilities to serve	the project for the reasons listed	
				1231 Maine Stre Poland, Maine 0		

Town of Poland

Planning Board

DEPARTMENTAL REVIEW OF PROPOSED SITE APPLICATION

Date:	01 / 04 / 2019			
To:	Chief Deputy William Gagne	Use Code for the T is required to ask the capacity of capital the Planning Board	Chapter 606, Site Review, of the Comprehensive Land own of Poland, an applicant for development approval nat Municipal Departments to comment on their acilities to serve a proposed development. Therefore, , by way of the applicant, is notifying you of the project and requests your comments	
Applicant: Address: Location: Road Locat Project ove	MB Bark LLC 100 Bark Mulch Drive. Auburn, Maine Map # 0004 ion: Hardscrabble Rd. rview: Additional material handling	Lot #_0015 ng areas for processino	Sublot #_0002 g recyclable materials.	
Scheduled	Planning Board Meeting Date 02	,12 ,2019		
 Applicants: Should attach all relevant sections of their plans to prevent delays. Mail this form letter along with a copy of the application so that each department head <u>receives</u> it at least fourteen days prior to the scheduled meeting. (See reverse for list of Department Heads) Confirm with the department heads that they have <u>delivered</u> their response to the Planning Board Office in time for the meeting. 				
	wed this application and provide the following: The project has no impact on the Departmer The Department has adequate existing capit The Department does not have adequate ex reasons on department letterhead) I need more information on the application.	tal facilities to serve the proje	re the project for the reasons listed. (Please submit	

Please return by: Date: 02 / 07/ 2019

Planning Board Office

Poland, Maine 04274-7328

Town of Poland 1231 Maine Street

CEO Office Tel: 207-998-4604

E-mail: planningadmin@polandtownoffice.org



Planning Board Office

1231 Maine Street, Poland, Maine 04274-7328

Findings of Fact & Conclusion of Law

Date: October 9, 2018

Application Type: Formal Shoreland Zoning Application

Owners Name: Thomas and Abbie Mannett

Located at: 41 Black Duck Lane **Parcel ID:** 0036-0007-0008

Zoning Districts: Rural Residential 2 and Limited Residential

Project Description:

On October 9, 2018 the Mannett's submitted a Formal Shoreland Zoning Application to remove the existing camp along with three storage sheds and construct a new 1,822 square foot three bedroom, three bathroom year round home. The new home will be placed behind the 100 foot normal high water mark.

303.2.G. In addition to the standards contained elsewhere in Comprehensive Land Use Code (CLUC), the Planning Board shall consider the following in the Shoreland Area as defined:

1. Will maintain safe and healthful conditions

The proposed building will not interfere with the general health or safety of any neighbors. Based on this information the Planning Board (Board) finds that this criterion will be met.

2. Will not result in water pollution, erosion, or sedimentation to surface waters

Based on the plan submitted, the Board finds that the issues of water pollution, erosion, or sedimentation to surface waters have been properly addressed. Based on this information the Board finds that this criterion will be met.

3. Will adequately provide for disposal of all wastewater

The application includes a new HHE-200 for a new three bedroom Subsurface Wastewater Disposal System. Based on this information the Board finds that this criterion will be met.

4. Will not have an adverse impact on spawning grounds, fish, aquatic life, birds, or other wildlife habitat

The structure is located completely on land and will not have an impact on the spawning grounds, fish, aquatic life, birds, or other wildlife habitat. Based on this information the Board finds this criterion will be met.

E-mail: planningadmin@polandtownoffice.org

5. Will conserve shore cover and visual, as well as actual, points of access to inland waters

The applicant is proposing to revegetate all disturbed areas. Based on this information above and in the record the Board finds that this criterion will be met.

6. Will protect archaeological and historic resources as designated in the Town of Poland Comprehensive Plan

The parcel and abutting parcels do not appear to be associated with any archaeological or historic resources as designated in the Comprehensive Plan. Therefore, the Board finds that this section is not applicable.

7. Will avoid problems associated with floodplain development and use

The structure associated with this application and the proposed activity does not appear to generate any problems in relation to floodplain development. Based on this information above and in the record the Board finds that this criterion will be met.

504.3 Non-Conforming Structures

504.3. A. Expansions of Non-Conforming Structures

The applicant is removing the existing non-conforming structure; therefore, the Board finds that section is not applicable.

504.3. B. Relocation of Non-Conforming Structures

The applicant is building a new structure behind the 100 foot normal high water mark; therefore, the Board finds that this section is not applicable.

507.3.C. Reconstruction or Replacement of Non-Conforming Structures

The Board must determine if the proposed structure meets the setbacks to the greatest practical extent. The Board considered the size of the lot, the slope of the land, the potential for soil erosion, the location of other similar structures on the adjacent property, the location of the existing rain gardens and underdrainage, the location of the existing septic system, and the type and amount of vegetation that may need to be removed if the structure would be relocated. The applicant has proposed to locate the new structure behind the 100 foot normal high water mark. Based on this information the Board finds that this criterion will be met.

504.3. D. Change of Use of a Nonconforming Structure

This application is not for a change of use of the existing non-conforming structure; therefore, the Board finds that this section is not applicable.

504.3. E. Planning Board Special Review for a Legal Non-Conforming Single-Family Dwelling Located in a Shoreland Zoning District

The applicant has asked for setback reductions to thirty feet on the west sideline and to 40' on the front setback; therefore, based on this information the Board finds that this criterion will be met.

E-mail: planningadmin@polandtownoffice.org

508.27 Shoreland Zoning Standards

508.27.B. Principal and Accessory Structures

Chapter 5 § 504.3 provides the performance standards for relocation and/or reconstruction of non-conforming structures. The applicant has proposed to move the new structure behind the 100 foot normal high water mark. Based on this information the Board finds that this criterion will be met.

508.27.C. Multiple Principal Structures

This application does not include multiple principal structures; therefore, the Board finds that this section is not applicable

508.27.D. Piers, Docks, Wharves, Bridges, and Other Structures and Uses

This application does not include any Piers, Docks, Wharves, Bridges or similar structures; therefore, the Board finds that this section is not applicable.

508.27.E. Individual Private Campsites

This application does not include any individual private campsites; therefore, the Board finds that this section is not applicable.

508.27.F. Parking Areas

There are no proposed parking areas with this application nor is the parcel located in the Resource Protection Shoreland Zoning District therefore; the Board finds that this section is not applicable

508.27.G. Roads and Driveways

The proposed driveway will be located behind the home 140 feet from the normal high water mark and is not located in a Resource Protection District. Based on this information the Board finds that this criterion will be met.

508.27.H. Storm Water Runoff

The applicant will incorporate installation of rock lined drip edges, a mitigation plan for pre and post construction erosion, and phosphorus control. Based on this information and in the record the Planning Board finds that this criterion will be met.

508.27.I. Essential Services

The applicant is not proposing to install any new electrical poles, transmission lines, satellite dishes, generators, hydrants etc.; therefore, the Board finds that this section is not applicable.

508.27.J. Mineral Exploration and Excavation Permits

The application is not for mineral exploration or any other mining or gravel pit operations; therefore, the Board finds that this section is not applicable.

508.27.K. Agriculture

The applicant is not proposing any livestock grazing areas, manure stockpiles, or any agriculture activates within the parcel; therefore, the Board finds that this section is not applicable.

E-mail: planningadmin@polandtownoffice.org

508.27.M. Clearing or Removal of Vegetation for Development Other Than Timber Harvesting or Individual Private Campsites

The applicant has proposed to remove nine trees, only two of them are located inside the 100 foot normal high water mark. Based on this information the Board finds that this criterion will be met.

Conclusion

- The application checklist was approved as complete on October 9, 2018, at which time the Board voted to waive the requirement for a site walk and public hearing.
- The applicant has provided the Board with a Deed (Book 9101, Page 54) showing reasonable right, title, or interest in the property.
- The Board has concluded that they have the jurisdiction to review the application under section Chapter 5 § 504.3 (Non-Conforming Structures).

Therefore, the Town of Poland Planning Board hereby approves (4-0) with the following conditions, the application for Thomas and Abbie Mannett, to replace a structure, as described in the application dated October 9, 2018 and the approved site plan dated September 20, 2018 and the above findings of facts.

Conditions of Approval

- Soil Erosion Control and Stormwater Management Measures shall be in place prior to construction. The Code Enforcement Officer may require additional measures be taken.
- Soil disturbance during the period March 1st to May 1st is prohibited.
- Any disturbed soils shall be revegetated immediately upon completion of construction and any disturbed soils within 100-ft. of the high-water mark shall be revegetated per the approved plan.
- This approval will expire twelve (12) months from the date of Planning Board approval if the project or the use has not been started within this allotted time.
- Building/use permits shall be obtained prior to the start of construction/use.
- A certified person in erosion control practices by the Maine Department of Environmental Protection must be present at the site each day earthmoving activity occurs for a duration that is sufficient to ensure that proper erosion and sedimentation control practices are followed. This is required until erosion and sedimentation control measures have been installed, which will either stay in place permanently or stay in place until the area is sufficiently covered with vegetation necessary to prevent soil erosion.
- The applicant has agreed to follow the recommendations by Androscoggin County Soil and Water Conservation District regarding pre and post storm water, erosion, and phosphorus issues within the parcel.
- Plan approval is also conditioned upon compliance by the Applicant with the Plans and specifications which have been received by the Planning Board in connection with the development proposal as well as with any oral or written commitments regarding the project which were specifically made by the Applicant to the Board in the course of its deliberations.
- The applicant must apply for and obtain all applicable permits for the proposed development under the Natural Resources Protection Act, Title 38 M.R.S.A. section 480-C, the Site Location of Development Act, the Erosion and Sedimentation Control law, Title 38 M.R.S.A. section 420-C, the Stormwater Management Law, the Federal Clean Waters Act as delegated to the State of Maine, and all other applicable state and federal laws regulating the use or development of land.

CEO Office Tel: 207-998-4604

E-mail: planningadmin@polandtownoffice.org

• The new site plan must be recorded with the Androscoggin County Registry of Deeds within 90 days of approval.

Pursuant to Section 304.5.B of the CLUC anyone aggrieved of this decision may file a written appeal within thirty (30) days of date of this decision in accordance with Rule 80-B of the Maine Rules of Civil Procedure.

Date Approved: October 9, 2018 Poland Planning Board

, Chairperson	James Porter, Vice -Chairperson		
George Greenwood, Secretary	Mark Weinberg		
Stephanie Floyd			

CEO Office Tel: 207-998-4604

E-mail: planningadmin@polandtownoffice.org



Planning Board Office

1231 Maine Street, Poland, Maine 04274-7328

Findings of Fact & Conclusion of Law

Date: January 22, 2019

Application Type: Formal Shoreland Zoning Application

Owners Name: Leonard and Patsy Adams Located at: 283 Jordan Shore Drive

Parcel ID: 0029-0003

Zoning Districts: Rural Residential 2 and Limited Residential

Project Description:

On January 22, 2019 the Adams' submitted a Formal Shoreland Zoning Application for an 8' x 15'6" addition with a new deck and steps (169.7 square feet). The addition will be placed on piers on the northeast corner of the existing camp which is also on piers. The existing finished floor elevation sits at 310.2 feet which is one foot above Base Flood Level and the floor in the addition will be the same height. The existing structures and the new addition fall within a Federally Designated Flood Hazard Zone.

303.2.G. In addition to the standards contained elsewhere in Comprehensive Land Use Code (CLUC), the Planning Board shall consider the following in the Shoreland Area as defined:

1. Will maintain safe and healthful conditions

The proposed building will not interfere with the general health or safety of any neighbors. Based on this information the Planning Board (Board) finds that this criterion will be met.

2. Will not result in water pollution, erosion, or sedimentation to surface waters

Based on the plan submitted, the Board finds that the issues of water pollution, erosion, or sedimentation to surface waters have been properly addressed. Based on this information the Board finds that this criterion will be met.

3. Will adequately provide for disposal of all wastewater

The application includes no changes to the existing Subsurface Wastewater Disposal system; therefore, the Board finds that this section is not applicable.

4. Will not have an adverse impact on spawning grounds, fish, aquatic life, birds, or other wildlife habitat

The structure is located completely on land and will not have an impact on the spawning grounds, fish, aquatic life, birds, or other wildlife habitat. Based on this information the Board finds this criterion will be met.

5. Will conserve shore cover and visual, as well as actual, points of access to inland waters

The applicant is proposing to revegetate all disturbed areas. Based on this information above and in the record the Board finds that this criterion will be met.

6. Will protect archaeological and historic resources as designated in the Town of Poland Comprehensive Plan

The parcel and abutting parcels do not appear to be associated with any archaeological or historic resources as designated in the Comprehensive Plan. Therefore, the Board finds that this section is not applicable.

7. Will avoid problems associated with floodplain development and use

The structure associated with this application has a finished floor level that is one foot above the base flood elevation and the Code Enforcement Officer will require a Flood Hazard Permit. Based on the information above and in the record the Board finds that this criterion will be met.

504.3 Non-Conforming Structures

504.3. A. Expansions of Non-Conforming Structures

The applicant is proposing a 100.4 square foot expansion which does not exceed their allowed 378.6 square feet minus the 84.4 square foot shed. Based on this information the Board finds that this criterion will be met.

504.3. B. Relocation of Non-Conforming Structures

The applicant is putting on a 100.4 square foot addition; based on this information the Board finds that this criterion will be met.

507.3.C. Reconstruction or Replacement of Non-Conforming Structures

The Board must determine if the proposed structure meets the setbacks to the greatest practical extent. The Board considered the size of the lot, the slope of the land, the potential for soil erosion, the location of other similar structures on the adjacent property, the location of the existing rain gardens and underdrainage, the location of the existing septic system, and the type and amount of vegetation that may need to be removed if the structure would be relocated. The applicant has not proposed to reconstruct or replace the structure only construct a 100.4 square foot addition. Based on this information the Board finds that this criterion will be met.

504.3. D. Change of Use of a Nonconforming Structure

This application is not for a change of use of the existing non-conforming structure; therefore, the Board finds that this section is not applicable.

504.3. E. Planning Board Special Review for a Legal Non-Conforming Single-Family Dwelling Located in a Shoreland Zoning District

The application does not include the need for setback reductions; therefore, the Board finds that this section is not applicable.

508.27 Shoreland Zoning Standards

508.27.B. Principal and Accessory Structures

Chapter 5 §504.3 provides the performance standards for relocation and/or reconstruction of non-conforming structures. The applicant has proposed an addition only; therefore, the Board finds that this section is not applicable.

508.27.C. Multiple Principal Structures

This application does not include multiple principal structures; therefore, the Board finds that this section is not applicable.

508.27.D. Piers, Docks, Wharves, Bridges, and Other Structures and Uses

This application does not include any Piers, Docks, Wharves, Bridges or similar structures; therefore, the Board finds that this section is not applicable.

508.27.E. Individual Private Campsites

This application does not include any individual private campsites; therefore, the Board finds that this section is not applicable.

508.27.F. Parking Areas

There are no proposed parking areas with this application nor is the parcel located in the Resource Protection Shoreland Zoning District therefore; the Board finds that this section is not applicable.

508.27.G. Roads and Driveways

There are no proposed driveway changes with this application nor is the parcel located in the Resource Protection Shoreland Zoning District therefore; the Board finds that this section is not applicable.

508.27.H. Storm Water Runoff

The applicant will incorporate installation of rock lined drip edges, a mitigation plan for pre and post construction erosion, and phosphorus control. Based on this information and in the record the Planning Board finds that this criterion will be met.

508.27.I. Essential Services

The applicant is not proposing to install any new electrical poles, transmission lines, satellite dishes, generators, hydrants etc.; therefore, the Board finds that this section is not applicable.

508.27.J. Mineral Exploration and Excavation Permits

The application is not for mineral exploration or any other mining or gravel pit operations; therefore, the Board finds that this section is not applicable.

508.27.K. Agriculture

The applicant is not proposing any livestock grazing areas, manure stockpiles, or any agriculture activates within the parcel; therefore, the Board finds that this section is not applicable.

508.27.M. Clearing or Removal of Vegetation for Development Other Than Timber Harvesting or Individual Private Campsites

There is no proposed vegetation removal with this application: therefore, the Board finds that this section is not applicable.

Conclusion

- The application checklist was approved as complete on January 22, 2019 at which time the Board voted to waive the requirement for a site walk and public hearing.
- The applicant has provided the Board with a Deed (Book 5449, Page 3) showing reasonable right, title, or interest in the property.
- The Board has concluded that they have the jurisdiction to review the application under section Chapter 5 §504.3 (Non-Conforming Structures).

Therefore, the Town of Poland Planning Board hereby approves (4-0) with the following conditions, the application for Leonard and Patsy Adams, to construct an addition, as described in the application dated January 22, 2019 and the approved site plan dated January 22, 2019 and the above findings of facts.

Conditions of Approval

- Soil Erosion Control and Stormwater Management Measures shall be in place prior to construction. The Code Enforcement Officer may require additional measures be taken.
- Soil disturbance during the period March 1st to May 1st is prohibited.
- Any disturbed soils shall be revegetated immediately upon completion of construction and any disturbed soils within 100-ft. of the high-water mark shall be revegetated per the approved plan.
- This approval will expire twelve (12) months from the date of Planning Board approval if the project or the use has not been started within this allotted time.
- Building/use permits shall be obtained prior to the start of construction/use.
- A certified person in erosion control practices by the Maine Department of Environmental Protection must be present at the site each day earthmoving activity occurs for a duration that is sufficient to ensure that proper erosion and sedimentation control practices are followed. This is required until erosion and sedimentation control measures have been installed, which will either stay in place permanently or stay in place until the area is sufficiently covered with vegetation necessary to prevent soil erosion.
- The applicant has agreed to follow the recommendations by Androscoggin County Soil and Water Conservation District regarding pre and post storm water, erosion, and phosphorus issues within the parcel.
- Plan approval is also conditioned upon compliance by the Applicant with the Plans and specifications which have been received by the Planning Board in connection with the development proposal as well as with any oral or written commitments regarding the project which were specifically made by the Applicant to the Board in the course of its deliberations.
- The applicant must apply for and obtain all applicable permits for the proposed development under the Natural Resources Protection Act, Title 38 M.R.S.A. section 480-C, the Site Location of Development Act, the Erosion and Sedimentation Control law, Title 38 M.R.S.A. section 420-C, the Stormwater Management Law, the Federal Clean Waters Act as delegated to the State of Maine, and all other applicable state and federal laws regulating the use or development of land.

CEO Office Tel: 207-998-4604 E-mail: planningadmin@polandtownoffice.org

- The new site plan must be recorded with the Androscoggin County Registry of Deeds within 90 days of approval.
- The new shed must be permitted before any new building permits are approved.

Pursuant to Section 304.5.B of the CLUC anyone aggrieved of this decision may file a written appeal within thirty (30) days of date of this decision in accordance with Rule 80-B of the Maine Rules of Civil Procedure.

Date Approved: January 22, 2019 Poland Planning Board

, Chairperson	James Porter, Vice -Chairperson
George Greenwood, Secretary	Mark Weinberg
Stephanie Floyd	



Planning Board Office

1231 Maine Street, Poland, Maine 04274-7328

Findings of Fact & Conclusion of Law

Application Type: Sketch Plan Review

Owners Name: Dawn Miller Located at: 1199 Maine St. Parcel ID: 0012-0044

Zoning District: Downtown Village, Aquifer Protection-1 Overlay

509.8 SUBMISSIONS

The Planning Board (Board) voted on November 13, 2018 that the application included all the mandatory submissions requirements for the site plan application except for sections; 509.8.A.3.h, 509.8.A.5.o.(1-5), 509.8.A.5.p.(1-5) and 509.8.A.5.q, that the Board has decided to waive, the Board finds that granting the waivers will not minimize public health, safety or welfare, or nullify the intent of the CLUC, or the Comprehensive Plan. Based on this information and in the record the Planning Board finds that this criterion will be met.

509.9 SITE PLAN REVIEW STANDARDS

A. Preservation of Landscape:

The applicant is not proposing any exterior changes to the parcel except the new stockade fence between the buildings. Based on this information above and in the record the Planning Board finds that this criterion will be met.

B. Relation of Proposed Buildings to Environment:

The existing building has wood siding, is of neutral colors, and has a pitched asphalt roof, all of which are consistent with the surrounding properties. Based on this information above and in the record the Planning Board finds that this criterion will be met.

C. Compatibility with Residential Areas:

The property has been commercial for several years and the change in use will not alter the existing character of the lot. There is ample parking located on the lot for the proposed use and the proposal will not create any unsightly views, noise, odor, or lighting pollution. Based on this information above and in the record the Planning Board finds that this criterion will be met.

D. Vehicular Access:

This parcel does not appear to contain more than five hundred (500') feet of street frontage on a single street and does not consist of more than ten (10) acres; therefore, a conceptual access master plan is not required. Furthermore, the applicant is not proposing any changes to the existing Maine Department of Transportation (MDOT) approved curb cuts. Based on this information and in the record the Planning Board finds that this criterion will be met.

E. Access to Route:

The Maine Department of Transportation (MDOT) has approved the existing location of the driveway entrance. Based on this information and in the record the Planning Board finds that this criterion will be met.

F. Surface Water:

The applicant has requested a waiver from this section due to the fact that there is no proposed development. The Board waives this section since there is no proposed development or soil disturbance and there are no known drainage issues at this time. The Board finds that granting the waivers will not minimize public health, safety or welfare, or nullify the intent of the CLUC, or the Comprehensive Plan. Based on this information and in the record the Planning Board finds that this criterion will be met.

G. Conservation, Erosion, and Sediment Control:

This application does not include any proposed development or soil disturbances; therefore, the Board finds that this section is not applicable.

H. Phosphorus Export:

The parcel is located in the Range Pond watershed, a great pond watershed. No changes to the site are proposed; therefore, the Board finds that this section is not applicable.

I. Site Conditions:

This application does not include any proposed development or soil disturbances; therefore, the Board finds that this section is not applicable.

J. Signs:

The applicant has proposed to use the existing sign on the property. The only change will be the graphics. Based on this information and in the record the Planning Board finds that this criterion will be met.

K. Special Features:

The applicant is not proposing to install any new mechanical equipment. Based on this information and in the record the Planning Board finds that this criterion will be met.

L. Exterior Lighting:

The applicant is not proposing to install any new exterior lighting. Based on this information and in the record the Planning Board finds that this criterion will be met.

M. Emergency Vehicle Access:

The property already has emergency access to three sides of the building, the Fire/Rescue Chief has stated that he is all set with the access at this time. Based on this information and in the record the Planning Board finds that this criterion will be met.

N. Municipal Services:

All Town departments have not disclosed any concerns with the application as it stands. Based on this information and in the record the Planning Board finds that this criterion will be met.

O. Water Supply:

The building has a drilled well which is located on the property, the existing well meets the minimum plumbing requirements for the proposed dog grooming and pet boarding. Based on this information and in the record the Planning Board finds that this criterion will be met.

P. Ground Water:

The parcel is located in an aquifer overlay district however, the application does not include any activities that are prohibited within the overlay district therefore, the existing and proposed development shall not result in undue effect of the quality or quantity of ground water. Based on this information and in the record the Planning Board finds that this criterion will be met.

Q. Air Emissions:

The proposed dog grooming and pet boarding will not create any dust, ash, smoke, or other particulate matter and will meet or exceed the standards set by the MDEP. Based on this information and in the record the Planning Board finds that this criterion will be met.

R. Odor Control:

The proposed dog grooming and pet boarding will not produce and offensive or harmful odors. Based on this information and in the record the Planning Board finds that this criterion will be met.

S. Noise:

The applicant has stated that the proposed dog grooming and pet boarding will meet the Town and MDEP'S minimum noise standards. Based on this information and in the record the Planning Board finds that this criterion will be met.

T. Sewage Disposal

The existing Subsurface wastewater system was approved for the two previous business's that were on the site. Based on this information and in the record the Planning Board finds that this criterion will be met.

U. Waste Disposal

The applicant is proposing no visible on-site waste disposal. Based on this information and in the record the Planning Board finds that this criterion will be met.

V. Buffer Areas

There are no existing buffer areas on the lot except at the rear. However, the proposed dog grooming and pet boarding will have little effect on the buffer. Based on this information above and in the record the Planning Board finds that this criterion will be met.

W. Adequate Financial and Technical Capacity

The applicant has stated that she will be running the dog grooming and pet boarding and that cost of work to complete the change of use will be minimal. Based on this information and in the record the Planning Board finds that this criterion will be met.

X. Conformance with the Comprehensive Plan

The proposed dog grooming and pet boarding which would be defined as a "business primary function" is a permitted use in the Downtown Village zoning district and will be in conformance with the Comprehensive Plan. Based on this information and in the record the Planning Board finds that this criterion will be met.

Conclusion

- The Board reviewed the Site Plan application on November 13, 2018 at which time the Board deemed the application as completed and decided to not hold a public hearing or site walk for the application.
- The applicant has provided the Board with a Deed (Book 9972, Page 73, 74) showing reasonable right, title, or interest in the property.
- The Board has concluded that they have the jurisdiction to review the application under Ch. 5 §509.2.A.3, (New uses of existing structures or land or existing uses that require Site Plan Review,

Therefore, the Town of Poland Planning Board hereby approves with the following conditions the application for Dawn Miller, "K9's and Kitty's 2" for the change of use from a gym/office space to a dog grooming and pet boarding business as described in the application letter dated November 01, 2018 and the above findings of facts.

Conditions of Approval:

- Plan approval is also conditioned upon compliance by the Applicant with the Plans and specifications which
 have been received by the Planning Board in connection with the development proposal as well as with any oral
 or written commitments regarding the project which were specifically made by the Applicant to the Board in
 the course of its deliberations.
- This approval will expire twelve (12) months from the date of Planning Board approval if the project or the use has not been started within this allotted time.
- Building/use permits shall be obtained prior to the start of construction/use.
- The applicant must apply for and obtain all applicable permits for the proposed development under the Natural Resources Protection Act, Title 38 M.R.S.A. section 480-C, the Site Location of Development Act, the Erosion and Sedimentation Control law, Title 38 M.R.S.A. section 420-C, the Stormwater Management Law, the Federal Clean Waters Act as delegated to the State of Maine, and all other applicable state and federal laws regulating the use or development of land.

Pursuant to Section 304.5.B of the CLUC anyone aggrieved of this decision may file a written appeal within Thirty (30) Days of date of this decision in accordance with Rule 80-B of the Maine Rules of Civil Procedure.

CEO Office Tel: 207-998-4604 Main Office Tel: 207-998-4601

E-mail: planningadmin@polandtownoffice.org

Date Approved: November 13, 2018 Poland Planning Board

, Chairperson	James Porter, Vice Chairperson
George Greenwood, Secretary	Mark Weinberg, Member
Stephanie Floyd, Member	<u> </u>

E-mail: planningadmin@polandtownoffice.org



Planning Board Office

1231 Maine Street, Poland, Maine 04274-7328

Findings of Fact & Conclusion of Law

Date: January 29, 2019

Application Type: Formal Shoreland Zoning Application

Owners Name: Arthur and Rebecca Weissman

Located at: 37 Mountain View Drive

Parcel ID: 0049-0035

Zoning Districts: Rural Residential 2 and Limited Residential

Project Description:

On September 25, 2018 the Weissman's submitted an application to remove a set of spiral stairs on the back (water side) of the garage. The applicant has requested approval to replace the spiral stairs with a seventy two (72) square foot balcony with no stairs. The historical property card shows there was an eighty (80) square foot balcony prior to the spiral stairs being installed.

303.2.G. In addition to the standards contained elsewhere in Comprehensive Land Use Code (CLUC), the Planning Board shall consider the following in the Shoreland Area as defined:

1. Will maintain safe and healthful conditions

The proposed building will not interfere with the general health or safety of any neighbors. Based on this information the Planning Board (Board) finds that this criterion will be met.

2. Will not result in water pollution, erosion, or sedimentation to surface waters

Based on the plan submitted, the Board finds that the issues of water pollution, erosion, or sedimentation to surface waters have been properly addressed. Based on this information the Board finds that this criterion will be met.

3. Will adequately provide for disposal of all wastewater

The application includes no changes to the existing Subsurface Wastewater Disposal system; therefore, the Board finds that this section is not applicable.

4. Will not have an adverse impact on spawning grounds, fish, aquatic life, birds, or other wildlife habitat

The structure is located completely on land and will not have an impact on the spawning grounds, fish, aquatic life, birds, or other wildlife habitat. Based on this information the Board finds this criterion will be met.

5. Will conserve shore cover and visual, as well as actual, points of access to inland waters

The applicant is proposing to revegetate all disturbed areas. Based on this information above and in the record the Board finds that this criterion will be met.

6. Will protect archaeological and historic resources as designated in the Town of Poland Comprehensive Plan

The parcel and abutting parcels do not appear to be associated with any archaeological or historic resources as designated in the Comprehensive Plan. Therefore, the Board finds that this section is not applicable.

7. Will avoid problems associated with floodplain development and use

The structure associated with this application has a finished floor level is more than one foot above base flood elevation. Based on this information above and in the record the Board finds that this criterion will be met.

504.3 Non-Conforming Structures

504.3. A. Expansions of Non-Conforming Structures

The applicant is proposing a seventy two (72) square foot balcony that does not exceed the historical square footage of eighty (80) square feet. Based on this information the Board finds that this criterion will be met.

504.3. B. Relocation of Non-Conforming Structures

The applicant is only proposing to construct a seventy two (72) square foot balcony. Based on this information the Board finds that this criterion will be met.

507.3.C. Reconstruction or Replacement of Non-Conforming Structures

The Board must determine if the proposed structure meets the setbacks to the greatest practical extent. The Board considered the size of the lot, the slope of the land, the potential for soil erosion, the location of other similar structures on the adjacent property, the location of the existing rain gardens and underdrainage, the location of the existing septic system, and the type and amount of vegetation that may need to be removed if the structure would be relocated. The applicant has not proposed to reconstruct or replace the structure only construct a seventy two (72) square foot Balcony. Based on this information the Board finds that this criterion will be met.

504.3. D. Change of Use of a Nonconforming Structure

This application is not for a change of use of the existing non-conforming structure; therefore, the Board finds that this section is not applicable.

504.3. E. Planning Board Special Review for a Legal Non-Conforming Single-Family Dwelling Located in a Shoreland Zoning District

The application does not include the need for setback reductions; therefore, the Board finds that this section is not applicable.

508.27 Shoreland Zoning Standards

508.27.B. Principal and Accessory Structures

Chapter 5 §504.3 provides the performance standards for relocation and/or reconstruction of non-conforming structures. The applicant has proposed an addition only therefore, the Board finds that this section is not applicable.

508.27.C. Multiple Principal Structures

This application does not include multiple principal structures; therefore, the Board finds that this section is not applicable.

508.27.D. Piers, Docks, Wharves, Bridges, and Other Structures and Uses

This application does not include any Piers, Docks, Wharves, Bridges, or similar structures; therefore, the Board finds that this section is not applicable.

508.27.E. Individual Private Campsites

This application does not include any individual private campsites; therefore, the Board finds that this section is not applicable.

508.27.F. Parking Areas

There are no proposed parking areas with this application nor is the parcel located in the Resource Protection Shoreland Zoning District. The Board finds that this section is not applicable.

508.27.G. Roads and Driveways

There are no proposed driveway changes with this application nor is the parcel located in the Resource Protection Shoreland Zoning District therefore; the Board finds that this section is not applicable.

508.27.H. Storm Water Runoff

The applicant will incorporate installation of rock lined drip edges, a mitigation plan for pre and post construction erosion, and phosphorus control. Based on this information and in the record the Planning Board finds that this criterion will be met.

508.27.I. Essential Services

The applicant is not proposing to install any new electrical poles, transmission lines, satellite dishes, generators, hydrants etc.; therefore, the Board finds that this section is not applicable.

508.27.J. Mineral Exploration and Excavation Permits

The application is not for mineral exploration or any other mining or gravel pit operations; therefore, the Board finds that this section is not applicable.

508.27.K. Agriculture

The applicant is not proposing any livestock grazing areas, manure stockpiles, or any agriculture activates within the parcel; therefore, the Board finds that this section is not applicable.

508.27.M. Clearing or Removal of Vegetation for Development Other Than Timber Harvesting or Individual Private Campsites

There is no proposed vegetation removal with this application: therefore, the Board finds that this section is not applicable.

Conclusion

- The application checklist was approved as complete on September 25, 2018, at which time the Board voted to waive the requirement for a site walk and public hearing.
- The applicant has provided the Board with a Deed (Book 9498, Page 178) showing reasonable right, title, or interest in the property.
- The Board has concluded that they have the jurisdiction to review the application under section Chapter 5 § 504.3 (Non-Conforming Structures).

Therefore, the Town of Poland Planning Board hereby approves (4-0) with the following conditions, the application for Arthur and Rebecca Weissman, to construct a seventy two (72) square foot balcony, as described in the application dated September 25, 2019 and the approved site plan dated September 25, 2019 and the above findings of facts.

Conditions of Approval

- Soil Erosion Control and Stormwater Management Measures shall be in place prior to construction. The Code Enforcement Officer may require additional measures be taken.
- Soil disturbance during the period March 1st to May 1st is prohibited.
- Any disturbed soils shall be re-vegetated immediately upon completion of construction and any disturbed soils within 100-ft. of the high-water mark shall be revegetated per the approved plan.
- This approval will expire twelve (12) months from the date of Planning Board approval if the project or the use has not been started within this allotted time.
- Building/use permits shall be obtained prior to the start of construction/use.
- A certified person in erosion control practices by the Maine Department of Environmental Protection must be present at the site each day earthmoving activity occurs for a duration that is sufficient to ensure that proper erosion and sedimentation control practices are followed. This is required until erosion and sedimentation control measures have been installed, which will either stay in place permanently or stay in place until the area is sufficiently covered with vegetation necessary to prevent soil erosion.
- The applicant has agreed to follow the recommendations by Androscoggin County Soil and Water Conservation District regarding pre and post storm water, erosion, and phosphorus issues within the parcel.
- Plan approval is also conditioned upon compliance by the Applicant with the Plans and specifications which have been received by the Planning Board in connection with the development proposal as well as with any oral or written commitments regarding the project which were specifically made by the Applicant to the Board in the course of its deliberations.

CEO Office Tel: 207-998-4604 E-mail: planningadmin@polandtownoffice.org

• The applicant must apply for and obtain all applicable permits for the proposed development under the Natural Resources Protection Act, Title 38 M.R.S.A. section 480-C, the Site Location of Development Act, the Erosion and Sedimentation Control law, Title 38 M.R.S.A. section 420-C, the Stormwater Management Law, the Federal Clean Waters Act as delegated to the State of Maine, and all other applicable state and federal laws regulating the use or development of land.

- The new site plan must be recorded with the Androscoggin County Registry of Deeds within 90 days of approval.
- The applicant must provide the correct shoreland zoning application.

Pursuant to Section 304.5.B of the CLUC anyone aggrieved of this decision may file a written appeal within thirty (30) days of date of this decision in accordance with Rule 80-B of the Maine Rules of Civil Procedure.

Date Approved: September 25, 2018 Poland Planning Board

, Chairperson	James Porter, Vice -Chairperson
George Greenwood, Secretary	Mark Weinberg
Stephanie Flovd	

E-mail: planningadmin@polandtownoffice.org



Planning Board Office

1231 Maine Street, Poland, Maine 04274-7328

Findings of Fact & Conclusion of Law

Application Type: Minor Subdivision Application for Megquire Hill Overlook Subdivision **Owners Name:** Megquire Hill Holdings LLC 1532 Thames St. Baltimore, Maryland 21231

Located at: 260 Megquire Hill Road **Parcel ID:** 0014-0025 and 0025-B

Zoning Districts: Village 3

Project Description:

On May 8, 2018 Megquier Hill Holdings LLC and their agent JKL Land Surveying submitted an application for a Minor Subdivision. This subdivision will be served by the existing Town Road Megquier Hill Road for two added lots and retainage of the existing farm on the third lot. The subdivision will propose three residential lots total, with an open space area as shown on the Subdivision Plan, named Megquier Hill Overlook for the Megquier Hill Holdings LLC, by JKL Land surveying, dated through April 2018.

The project is located off the northerly side of Megquier Hill Road and near the intersection of Plummer's Lane. The property is located on 47 acres in the Village 3 Zoning District which typically requires 80,000 SF minimums for conventional lots. A cluster concept as provided to consider reduction in frontage requirements and area to allow for the layout as provided, but with a sizeable parcel of open space (40+ acres) the requirement for a cluster lot did not seem to warrant market demands or benefit the Town, and the conventional lot layout was selected.

The project application was reviewed to meet the performance standards in Section 612 of Poland Comprehensive Land Use Code (CLUC) and are intended to clarify and expand upon the criteria for approval found within the Subdivision Statute (Title 30-A, M.R.S.A. Section 4404) and carry out the purposes of the CLUC and the Town of Poland Comprehensive Plan. In reviewing a proposed Subdivision, the Board has reviewed the application for conformance with the following performance standards and make findings that each has been met as part of the approval of a Final Plan. Compliance with the design guidelines of Section 613 shall be considered to be evidence of meeting the appropriate performance standards.

612.1 Pollution: The proposed Subdivision will not discharge waste water into a water body and proposed subsurface wastewater systems have been considered for the proposed lots. Discharges of storm water shall be treated to remove contaminants to discharge into surface water bodies by use of retained tree buffers.

612.2 Sufficient Water: The proposed Subdivision is not within the area of a public water supply shall be from individual wells and such wells have been sited to prevent infiltration of surface water, and contamination from subsurface wastewater disposal systems and other sources of potential contamination. The design of and permit placement of wells, subsurface wastewater disposal areas, and reserve sites for subsurface wastewater disposal areas in compliance with the Maine Subsurface Wastewater Disposal Rules and the Well

E-mail: planningadmin@polandtownoffice.org

Drillers and Pump Installers Rules. In lieu of providing Public Water or private source of water supply for fire protection with fire hydrants the Fire Chief has accepted the use of life safety sprinkler systems to be added to each proposed home in the Subdivision.

- **612.3 Impact on Existing Water Supplies:** In meeting the standards of Section 612.2., the proposed Subdivision will not generate a demand on the source, treatment facilities or distribution system of the servicing water company or district beyond the capacity of those system components.
- **612.4 Soil Erosion:** The proposed Subdivision will prevent soil erosion from entering water bodies, and adjacent properties following the procedures outlined in submitted Erosion and Sedimentation Control Plan to be implemented during the site preparation, construction, and clean-up stages. Topsoil shall be considered part of the Subdivision and shall not be removed from the site except for surplus topsoil from roads, parking areas, and building excavations.
- **612.5 Traffic Conditions**: The proposed subdivision does not propose any roads and only driveways therefore, due to the proposed low daily trip volume from the lot development vehicular access to the Subdivision and within the Subdivision will be conducted in a manner as to: 1. Safeguard against hazards to traffic and pedestrians in existing streets and within the Subdivision; 2. Avoid traffic congestion on any street; and 3. Provide safe and convenient circulation on public streets and within the Subdivision. Adequate provisions for fire and emergency service could be provided from the existing public street, Megquire Hill Road.
- **612.6 Sewage Disposal**: The development will provide private subsurface wastewater disposal systems and has provided evidence of site suitability for subsurface sewage disposal prepared by a State of Maine Department of Human Services licensed Site Evaluator in full compliance with the requirements of the State of Maine Subsurface Wastewater Disposal Rules. With evidence to certify in writing that each lot has one passing test pit which meet the requirements for a new system represents an area large enough to a disposal area on soils which meet the State of Maine Subsurface Wastewater Disposal Rules.
- **612.7 Solid Waste**: If the additional solid waste from the proposed Subdivision does exceeds the capacity of the Municipal Solid Waste given the relative low output from the 3 single family lots.
- **612.8** Impact on Natural Beauty, Aesthetics, Historic Sites, Wildlife Habitat, Rare Natural Areas or Public Access to the Shoreline: The Plan shall, by conditions of approval on the Final Plan and deed restrictions, limit the clearing of trees to those areas designated on the Subdivision Plan. Preservation of the existing fam and tree clearing will promote the natural Beauty and vistas from the proposed property.
- **612.9 Conformance with Zoning and Other Land Use Standards:** All lots meet the minimum dimensional requirements of Chapter 5, Land Zoning Standards, for the Village 3 zoning district in which they are located. The proposed Subdivision meets all applicable performance standards or design criteria from Chapter 5.
- **612.10 Financial and Technical Capacity:** The applicant has adequate financial resources to construct the proposed improvements and meet the criteria of Title 30-A, M.R.S.A. Section 4404 and the standards of this Code. The applicant will qualified contractors and consultants to supervise, construct and inspect the required improvements in the proposed Subdivision.
- **612.11 Impact on Water Quality or Shoreline:** The property is not located along a shore so there is no cutting or removal of vegetation along water bodies that could increase water temperature, or result in shoreline erosion or sedimentation of water bodies.

E-mail: planningadmin@polandtownoffice.org

- **612.12 Impact on Ground Water Quality or Quantity:** The Subdivision will not adversely affect the quality or quantity of ground water. Based on the low number of single family dwellings lots proposed no hydrogeological study for nitrate plums or adverse groundwater impacts was required.
- **612.13 Floodplain Management**: No part of the proposed Subdivision is located in a Special Flood Hazard Area as identified by the Federal Emergency Management Agency.
- **612.14 Identification of Freshwater Wetlands:** No Freshwater wetlands were identified in accordance with the 1987 Corps of Engineers Wetland Delineation Manual, (published by the United States Army Corps of Engineers and as amended) on the site project limits.
- **612.15 River, Stream, or Brook**: There are no rivers, streams, or brooks within or abutting the Subdivision shall be identified on the Plan.
- **612.16 Storm Water Management:** Adequate provisions were made for the management of the quantity and quality of all storm water generated within the Subdivision, through incorporation of Best Management Practices equivalent to those described in the Stormwater Management for Maine: Best Management Practices, (published by the Maine Department of Environmental Protection, 2015 and as amended), and in conformance with the policies of the Town of Poland Comprehensive Plan. The stormwater design incorporates the use of buffers to mitigate downstream impacts to abutters and Tripp Lake.
- **612.17 Phosphorus Impacts on Great Ponds:** The Subdivision is within the watershed of Tripp Lake and did provide basic computations to comply with the required phosphorus export for the development. The subdivision qualifying for and using the Simplified Review Method in accordance with the Simplified Review Method for Minor Projects (Chapter 4) in Phosphorus Control in Lake Watersheds: A Technical Guide for Evaluating New Development, (published by the Maine Department of Environmental Protection, revised September 1992 and as amended). The buffer areas were located downslope of developed areas.
- 612.18 Impact on Adjoining Municipality: The proposed Subdivision does not cross the boundary into an adjacent Municipality, the Subdivision will not cause unreasonable traffic congestion or unsafe conditions with respect to the use of existing public roads in an adjacent Municipality in which part of the Subdivision is located.

613 DESIGN GUIDELINES

- **613.1 Sufficient Water A. Fire Protection:** No public roads were proposed, and no public water lines or existing fire protection services are nearby to the project. In lieu of provisions for fire protection apparatus and water supply the Fire Chief requested a condition of approval that the homes to be built be provided with a life sprinkler system to be installed during the home construction. The Board accepted the condition and no additional measures were required to meet the guideline.
- **613.2 Traffic Conditions:** The Subdivision does not abut an arterial street, nor has frontage on two (2) or more streets, the access to the lots shall be provided to the lot across the frontage and to the street where there is lesser potential for traffic congestion and for hazards to traffic and pedestrians unless other factors make it not practical.
- **613.3** Wildlife Habitat, Rare Natural Areas or Public Access to the Shoreline A. Preservation of Natural Beauty and Aesthetics: The project is located in the Village 3 District as defined in Chapter 5, Land Zoning Standards, but still intends to preserve the existing 40 acre farm as part of the land retained in its natural farm

E-mail: planningadmin@polandtownoffice.org

and maple syrup production. Although not a Historic Features – the proposed Subdivisions will include preservation measures and buffers to retain the character of the existing farm and maintain the vista and viewshed from the property.

- **613.4 Storm Water Management Design Guidelines:** Design of Best Management Practices are substantially equivalent to those described in the Storm Water Management for Maine: Best. Due to the limited soil disturbance for access no pipe or culverts will be required.
- **613.5 Impact on Water Quality or Shoreline** The project is not in close proximity nor abuts a great pond or has shoreline frontages. Thereby the guideline does not apply.
- **613.6 Lots**: The side lines of the lot are generally perpendicular to the street. The proposed parcels do not provide more than twice the required minimum lot size and are laid out in such a manner as to not provide for or preclude future division. Land retained is in an area approximately 40 acres and though placed in preservation potentially the lot could be further divided in the future. Deed restrictions and conditions of approval on the Plan shall either prohibit future divisions of the lots or specify that any future division shall constitute a revision to the Plan and shall require approval from the Board, subject to the criteria of the State Subdivision Statute, the standards of this Code and conditions placed on the original approval.
- **613.7 Utilities**: There are no proposed roads for the subdivision and the lots shall be serviced from existing public utilities available along their street frontage. The Planning Board determined that the homeowner shall have the option of overhead vs underground but advised to place underground from the last pole location.
- **613.8 Monuments** A. No road monuments are required and all other Subdivision boundary corners and angle points, as well as all lot boundary corners and angle points are to be marked by suitable monuments or use of natural stonewalls as required by the Maine Board of Registration of Land Surveyors.
- **613.9 Cluster Developments:** The project did provide a plan for considering Cluster development however the Planning Board following the conventional size and bulk layouts with retention of the existing farm, proposed the optimum benefit for the community and the developer for marketing. The Cluster Development design was not selected.
- **613.10 Reservation or Dedication and Maintenance of Open Space and Common Land, Facilities and Services**: The development retained tree buffers preservation and placed the remaining tract to remain in a 40 acre parcel to be retained as farming. Since the number of units was only two new lots being proposed the guideline does not apply, even with the concept of preserving the farm character.
- **613.11 Agricultural Land Buffers**: The proposed Subdivision will abut active commercial agricultural land, a buffer of unimproved natural vegetation of a minimum of one hundred (100) feet will be provided between dwellings and the active agricultural land.
- **613.12 Buffers for Non-residential Subdivisions**: The guideline does not apply.

E-mail: planningadmin@polandtownoffice.org

Conclusion

THEREFORE, the Town of Poland Planning Board hereby approves by a 5-0 vote held on May 22, 2018, with the following conditions, for the Minor Subdivision Plan application for Megquire Hill Holdings LLC, Megquire Hill Overlook Subdivision project consisting of construction of 3 residential lots, with 2 lots for out sale and one lot for the existing farm to remain. No private or public streets or infrastructure will be required as the lots have adequate frontage on the existing Megquire Hill Road. Proposed Minor Subdivision development is as described in the application approved and in this Findings of Fact.

Conditions of Approval

- Plan approval is also conditioned upon compliance by the Applicant with the Plans and specifications which have been
 received by the Planning Board in connection with the development proposal as well as with any oral or written
 commitments regarding the project which were specifically made by the Applicant to the Board in the course of its
 deliberations.
- Except in the case of a Phased Development Plan, failure to complete substantial construction of the Subdivision within five (5) years of the date of approval and signing of the Plan shall render the Plan null and void. Upon determining that a Subdivision's approval has expired under this Paragraph, the Board shall have a notice placed in the Androscoggin County Registry of Deeds to that effect.
- Any Subdivision not recorded in the Registry of Deeds within ninety (90) days of the date upon which the Plan is approved and signed by the Board shall become null and void.
- Building/use permits shall be obtained prior to the start of construction/use.
- The applicant must apply for and obtain all applicable permits for the proposed development under the Natural Resources Protection Act, Title 38 M.R.S.A. section 480-C, the Site Location of Development Act, the Erosion and Sedimentation Control law, Title 38 M.R.S.A. section 420-C, the Stormwater Management Law, the Federal Clean Waters Act as delegated to the State of Maine, and all other applicable state and federal laws regulating the use or development of land.
- The subdivision will require to provide an access easement over lot two for the benefit of the existing farm access and service road to the farming/ maple syrup operation. The easement shall be conveyed by deed, and proof of the execution shall be provided to the Town prior to issuance of a building permit for the lot.
- The two lots considered for sale and new home construction shall provide and install the required sprinkler life safety systems as outlined in the Fire Chiefs memo provided at the May 22, 2018 meeting. Systems shall be approved by the Fire Chief and the Code Enforcement Officer prior to building permits and no occupancy shall be granted until proof of their final inspection and full operation has been deemed acceptable by the Town.
- Pursuant to Section 304.5.B of the CLUC anyone aggrieved of this decision may file a written appeal within Thirty (30) Days of date of this decision in accordance with Rule 80-B of the Maine Rules of Civil Procedure.

E-mail: planningadmin@polandtownoffice.org

Date Approved:		
Poland Planning Board		
Dawn Dyer, Vice -Chairperson	Alex Duff	
James Porter, Chairperson	George Greenwood	
Mark Weinberg	Stephanie Floyd, Alternate	

E-mail: planningadmin@polandtownoffice.org



Planning Board Office

1231 Maine Street, Poland, Maine 04274-7328

Findings of Fact & Conclusion of Law

Application Type: Amended Minor Subdivision Application for Washburn Minor Subdivision

Owners Name: Greg Washburn

Located at: 535 Woodman Hill Rd. Minot, Maine 04250

Parcel ID: Tax Map 15 Lot 18B **Zoning Districts:** Downtown Village

Project Description:

The project is an Amended Minor Subdivision to be served by a private 60 foot wide Right of way off Route 26/Maine Street for it entails altering existing lot lines to consolidate 4 previous lots into 3 lots. The project scope is to develop the main larger lot of 8.2 acres which will incorporate one smaller existing lot into one self-storage facility for both indoor cold storage, and an outdoor storage area, with a small office building, while modifying the remaining lots to reduce to create just two lots remaining with slightly deeper layout. The site is accessed by an existing 60 foot right of way previously approved. The Washburn Property Subdivision plan was prepared by Survey Works and George Courbron, JR PLS #1126, dated through October 2018.

The project is located off the easterly side of Maine Street and near the intersection of Route 11. The property is located on 9+ acres in the Downtown Village Zoning District which typically requires 20,000 SF minimums for conventional lots. The project application was reviewed to meet the performance standards in Section 612 of Poland Comprehensive Land Use Code (CLUC) and are intended to clarify and expand upon the criteria for approval found within the Subdivision Statute (Title 30-A, M.R.S.A. Section 4404) and carry out the purposes of the CLUC and the Town of Poland Comprehensive Plan. In reviewing a proposed Subdivision, the Board has reviewed the application for conformance with the following performance standards and make findings that each has been met as part of the approval of a Final Plan. Compliance with the design guidelines of Section 613 shall be considered to be evidence of meeting the appropriate performance standards.

612.1 Pollution: The proposed Subdivision will not discharge waste water into a water body and proposed subsurface wastewater systems have been considered for the proposed lots. Discharges of storm water shall be treated to remove contaminants to discharge into surface water bodies while Lot 1 has been considered lots 2 and 3 have not. A note shall be added stating each lot will require Planning Board approval for future development and stormwater provisions for conveyance and treatment.

612.2 Sufficient Water: The proposed Subdivision is within the area of a public water supply, water supply is noted as providing Public Water and that the requirement for water supply for fire protection with fire hydrants shall be at the discretion of the Fire Department as it applies their interpretation of life safety sprinkler systems requirement within the Subdivision.

E-mail: planningadmin@polandtownoffice.org

- **612.3 Impact on Existing Water Supplies:** In meeting the standards of Section 612.2., the proposed Subdivision will not generate a large demand on the source, treatment facilities or distribution system of the servicing water company or district beyond the capacity of those system components.
- **612.4 Soil Erosion:** The proposed Subdivision will prevent soil erosion from entering water bodies, and adjacent properties following the procedures outlined in submitted Erosion and Sedimentation Control Plan to be implemented during the site preparation, construction, and clean-up stages. Top soil shall be considered part of the Subdivision and shall not be removed from the site except for surplus topsoil from roads, parking areas, and building excavations.
- **612.5 Traffic Conditions**: The proposed subdivision does not propose any roads and only access is a shared driveway. It is anticipated the proposed development will have low daily trip volume from the lot development, but vehicular access to the Subdivision and within the Subdivision will be conducted in a manner as to: 1. Safeguard against hazards to traffic and pedestrians in existing streets and within the Subdivision; 2. Avoid traffic congestion on any street; and 3. Provide safe and convenient circulation on public streets and within the Subdivision. A statement as to the site entrance meeting MDOT permits shall be obtained for the modification as well as a statement as to the trip generation anticipated from the subdivision, regardless of how driveways are interconnected or not. Provisions and turning space for fire and emergency service shall be provided from the existing public street, Route 26 for all the lots.
- **612.6 Sewage Disposal**: The development will provide private subsurface wastewater disposal systems and has provided evidence of site suitability for subsurface sewage disposal prepared by a State of Maine Department of Human Services licensed Site Evaluator in full compliance with the requirements of the State of Maine Subsurface Wastewater Disposal Rules. With evidence to certify in writing that each lot has one passing test pit which meet the requirements for a new system represents an area large enough to a disposal area on soils which meet the State of Maine Subsurface Wastewater Disposal Rules.
- **612.7 Solid Waste**: The additional solid waste from the proposed Subdivision does not exceed the capacity of the Municipal Solid Waste Facilities, given the relative low output from the relatively small commercial uses proposed. A conditional statement shall be added how waste will be handled on the plan.
- 612.8 Impact on Natural Beauty, Aesthetics, Historic Sites, Wildlife Habitat, Rare Natural Areas or Public Access to the Shoreline: The Plan shall, by conditions of approval on the Final Plan and with deed restrictions, include landscape buffers reserved for lot1 to those areas designated on the Subdivision/Site Plan. Given the openness of the lots there is no aesthetics to necessarily reserve, and the applicant has noted that no habitat or wildlife species will be disturbed as the remaining wetlands on the site will not be directly impacted...
- **612.9 Conformance with Zoning and Other Land Use Standards:** All lots meet the minimum dimensional requirements of Chapter 5, Land Zoning Standards, for the Downtown Village zoning district in which they are located. The proposed Subdivision meets all applicable performance standards or design criteria from Chapter 5.
- **612.10 Financial and Technical Capacity:** The applicant has adequate financial resources to construct the proposed improvements and meet the criteria of Title 30-A, M.R.S.A. Section 4404 and the standards of this Code. The applicant will use qualified contractors and consultants to supervise, construct and inspect the required improvements in the proposed Subdivision.

E-mail: planningadmin@polandtownoffice.org

- **612.11 Impact on Water Quality or Shoreline:** The property is not located along a shore so there is no cutting or removal of vegetation along water bodies that could increase water temperature, or result in shoreline erosion or sedimentation of water bodies.
- **612.12 Impact on Ground Water Quality or Quantity:** The Subdivision will not adversely affect the quality or quantity of ground water. Based on the low number of commercial or office employees proposed, no hydrogeological study for nitrate plums or adverse groundwater impacts should be required.
- **612.13 Floodplain Management**: No part of the proposed Subdivision is located in a Special Flood Hazard Area as identified by the Federal Emergency Management Agency.
- **612.14 Identification of Freshwater Wetlands:** Freshwater wetlands were identified in accordance with the 1987 Corps of Engineers Wetland Delineation Manual, (published by the United States Army Corps of Engineers and as amended) on the site project limits. No wetlands will be directly impacted by the subdivision.
- **612.15 River, Stream, or Brook**: There are no rivers, streams, or brooks within or abutting the Subdivision shall be identified on the Plan.
- 612.16 Storm Water Management: Adequate provisions were proposed to Lot subject to Maine DEP approval for the management of the quantity and quality of all storm water generated within the developed Lot1, through incorporation of Best Management Practices equivalent to those described in the Stormwater Management for Maine: Best Management Practices, (published by the Maine Department of Environmental Protection, 2015 and as amended), and in conformance with the policies of the Town of Poland Comprehensive Plan. The stormwater design does not incorporates the proposed future uses of Lots 2 &3 and as such the condition of approval is that the applicants shall return to the Planning Board for site development planned on those lots. The proposed lot 1 utilizes filtration underdrain measures to acquire runoff treatment and volume control.
- **612.17 Phosphorus Impacts on Great Ponds:** The Subdivision is not within the watershed of a great pond and is in the watershed to Little Androscoggin River. The applicant will be acquiring a stormwater management permit from the Maine DEP and subject to those approvals.
- 612.18 Impact on Adjoining Municipality: The proposed Subdivision does not cross the boundary into an adjacent Municipality, the Subdivision will not cause unreasonable traffic congestion or unsafe conditions with respect to the use of existing public roads in an adjacent Municipality in which part of the Subdivision is located.

613 DESIGN GUIDELINES

- **613.1 Sufficient Water A. Fire Protection:** No public roads were proposed and public water lines for fire protection services form nearby water mains adjacent to the project will serve the site. Given provisions for fire protection apparatus and water supply, the Fire Department may require sprinkler systems depending on the Building use or as the structure volume/size meets the fire code. The Board accepted the condition and no additional measures were required to meet the guideline.
- **613.2 Traffic Conditions:** The Subdivision does abut, an arterial street, and has frontage on one public street, however, the access to the lots shall be provided through the shared private right of way, where there is lesser potential for traffic congestion and for hazards to traffic and pedestrians unless other factors make it not practical.

E-mail: planningadmin@polandtownoffice.org

- **613.3** Wildlife Habitat, Rare Natural Areas or Public Access to the Shoreline A. Preservation of Natural Beauty and Aesthetics: The project is located in the Downtown Village District as defined in Chapter 5, Land Zoning Standards, and the location offers little opportunity to reserve natural areas. There are some wetland areas to rear of the property that are not to be disturbed which will offer some coincidental preservation. The area consists of low vegetation in the wetland and offers little aesthetic value, but can offer some buffering qualities.
- **613.4 Storm Water Management Design Guidelines:** Design of Best Management Practices are substantially equivalent to those described in the Storm Water Management for Maine: Best Management Practices. Each lot as they are developed will be required to assess the Stormwater Management computations and engineering on a lot by lot basis.
- **613.5 Impact on Water Quality or Shoreline** The project is not in close proximity nor abuts a great pond or has shoreline frontages. Thereby the guideline does not apply.
- **613.6 Lots**: The side lines of the lot are generally perpendicular to the street. The proposed parcels do provide more than the required minimum lot size and are laid out in such a manner as to not provide for or preclude future division. Deed restrictions and conditions of approval on the Plan shall either prohibit future divisions of the lots or specify that any future division shall constitute a revision to the Plan and shall require approval from the Board, subject to the criteria of the State Subdivision Statute, the standards of this Code and conditions placed on the original approval.
- **613.7 Utilities**: There are no proposed public roads for the subdivision and the lots shall be serviced from existing public utilities available along the Maine Street frontage or through the shared access right of way.
- **613.8 Monuments** A. No road monuments are required and all other Subdivision boundary corners and angle points, as well as all lot boundary corners and angle points are be marked by suitable monuments, as required by the Maine Board of Registration of Land Surveyors.
- **613.9 Cluster Developments:** The project did provide a plan for considering Cluster development and followed the conventional size and bulk layouts with the Planning Board satisfied the optimum benefit for the community and the developer for marketing was provided. Thereby no Cluster Development design was not selected.
- 613.10 Reservation or Dedication and Maintenance of Open Space and Common Land, Facilities and Services: No open space was required or provided for the minor subdivision.
- **613.11 Agricultural Land Buffers**: The proposed Subdivision does not abut any active commercial agricultural land.
- **613.12 Buffers for Non-residential Subdivisions**: Each lot will be reviewed for buffers and screening as well as meeting the Downtown Village Design Guidelines during the site plan review process.

E-mail: planningadmin@polandtownoffice.org

Conclusion

THEREFORE, the Town of Poland Planning Board hereby approves by a 4-0 vote held on November 13, 2018, with the following conditions, for the Amended Minor Subdivision Plan application for Washburn Property Minor Subdivision project consisting of construction of 3 lots, which were created by consolidation from a former approved 4 lot subdivision. No public streets or infrastructure will be required as the lots have adequate frontage on the existing Maine Street, but will access through a common 60 foot wide Right of Way. The Proposed Minor Subdivision development is as described in the application approved and in this Findings of Fact.

Conditions of Approval

- Plan approval is also conditioned upon compliance by the Applicant with the Plans and specifications which have been
 received by the Planning Board in connection with the development proposal as well as with any oral or written
 commitments regarding the project which were specifically made by the Applicant to the Board in the course of its
 deliberations.
- Except in the case of a Phased Development Plan, failure to complete substantial construction of the Subdivision within
 five (5) years of the date of approval and signing of the Plan shall render the Plan null and void. Upon determining that a
 Subdivision's approval has expired under this Paragraph, the Board shall have a notice placed in the Androscoggin
 County Registry of Deeds to that effect.
- Any Subdivision not recorded in the Registry of Deeds within ninety (90) days of the date upon which the Plan is approved and signed by the Board shall become null and void.
- Building/use permits shall be obtained prior to the start of construction/use.
- The applicant must apply for and obtain all applicable permits for the proposed development under the Natural Resources Protection Act, Title 38 M.R.S.A. section 480-C, the Site Location of Development Act, the Erosion and Sedimentation Control law, Title 38 M.R.S.A. section 420-C, the Stormwater Management Law, the Federal Clean Waters Act as delegated to the State of Maine, and all other applicable state and federal laws regulating the use or development of land.

Pursuant to Section 304.5.B of the CLUC anyone aggrieved of this decision may file a written appeal within Thirty (30) Days of date of this decision in accordance with Rule 80-B of the Maine Rules of Civil Procedure.

Date Approved:		
	Poland Planning Board	
James Porter, Vice -Chairperson	Chairperson	
George Greenwood	Mark Weinberg	
Stephanie Floyd, Alternate	,	